



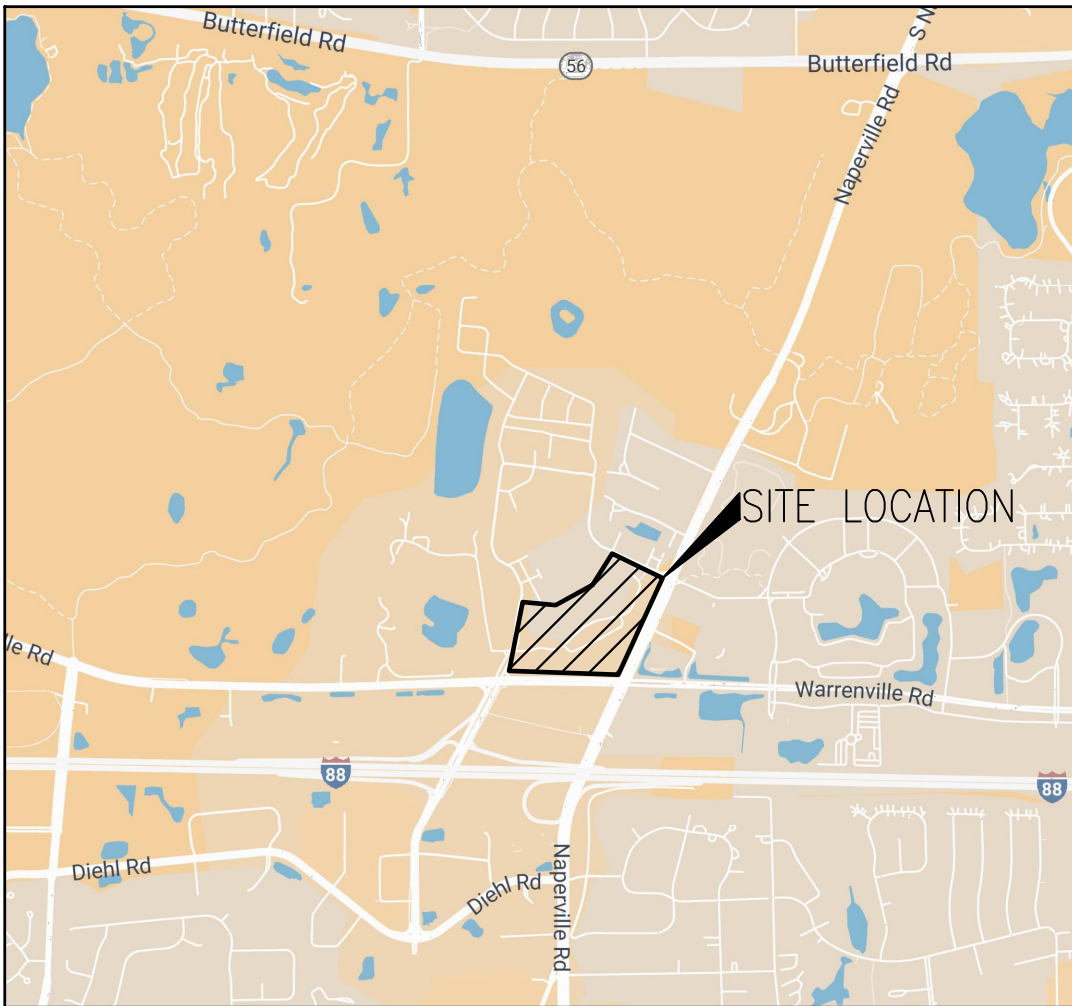
SITE IMPROVEMENT PLANS

FOR

1960 WEST LUCENT LANE

NAPERVILLE, ILLINOIS

KARIS CRITICAL



LOCATION MAP
NOT TO SCALE

DRAWING INDEX

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ABBREVIATION	LEGEND
ADA	AMERICANS WITH DISABILITIES ACT
B/	BOTTOM OF
BC	BACK OF CURB ELEVATION
C-C	CENTER TO CENTER SPACING
C&G	CURB AND GUTTER
CB	CATCH BASIN
DEP	DEPRESSED
DIP	DUCTILE IRON PIPE
DWS	DETECTABLE WARNING STRIP
EP	EDGE OF PAVEMENT ELEVATION
EX	EXISTING
FH	FIRE HYDRANT
FR	FUTURE
GR	GRADE RING ELEVATION
HC	HANDICAPPED
HWL	POND HIGH WATER LEVEL
I	UTILITY INVERT ELEVATION
MH	MANHOLE
NWL	POND NORMAL WATER LEVEL
PC	PRESSURE CONNECTION
PCC	PORTLAND CEMENT CONCRETE
PR	PROPOSED
PVC	POLYVINYL CHLORIDE PIPE
R	UTILITY STRUCTURE RIM ELEVATION
R&R	REMOVE AND REPLACE
RCF	REINFORCED CONCRETE PIPE
STM	STORM SEWER
T/	TOP OF
TC	TOP OF CURB ELEVATION
TDC	TOP OF DEPRESSED CURB ELEVATION
TF	TOP OF FOUNDATION ELEVATION
TW	TOP OF RETAINING WALL ELEVATION
VCP	VITRIFIED CLAY PIPE
VF	VERIFY IN FIELD
Vv	VALVE VAULT
W/	WITH
WM or WTR	WATER MAIN

PROPOSED	DESCRIPTION	EXISTING
	STORM SEWER	
	WATER MAIN WITH SIZE	
	SANITARY SEWER	
	RIGHT-OF-WAY/PROPERTY LINE	
	CONTOUR	
	SPOT GRADE	
	SANITARY MANHOLE	
	STORM MANHOLE	
	STORM INLET	
	STORM CATCH BASIN	
	FIRE HYDRANT	
	PRESSURE CONNECTION	
	GATE VALVE W/VAULT	
	LIGHT POLE	
	STREET LIGHT W/MAST	
	UTILITY POLE	
	SILT FENCE	
	OVERFLOW DIRECTION	
	SURFACE/DITCH FLOW DIRECTION	
	ROAD SIGN	
	CURB & GUTTER	
	DEPRESSED CURB FOR RAMP/DRIVEWAY	
	TOP OF FOUNDATION	
	GARAGE FLOOR, AT REAR OF GARAGE	
	TOP OF CURB, DEPRESSED	
	TOP OF RETAINING WALL	
	RIM FOR STRUCTURES	
	HIGH/NORMAL WATER LEVEL	
	TRANSFORMER	
	FENCE LINE	
	GUARD RAIL	
	FORCE MAIN	
	UNDERGROUND TELEPHONE	
	UNDERGROUND ELECTRIC	
	OVERHEAD ELECTRIC	
	UNDERGROUND GAS LINE	

BENCHMARK & LOCATIONS:

REFERENCE BENCHMARK(S):
CITY OF NAPERVILLE SURVEY MONUMENT STATION NO. 703

BERNSTEIN 3D TOP SECURITY MONUMENT, CONSISTING OF A 9/16" DIAM. STAINLESS STEEL DATUM POINT ON THREADED 9/16" X 4" LONG ROD TOTALING (8") IN LENGTH WITH GREASED TOP SECURITY SLEEVE ENCLOSED IN SAND AND 6" PVC PIPE WITH BMAC 6 ALUMINUM ACCESS COVER.

ELEVATION = 780.31 (NAVD 1988)

SITE BENCHMARK(S):

SITE BENCHMARK 1 (SBM 1) (JHA CP 2) - CROSS NOTCH CUT IN TOP OF CURB

SBM 1 IS A CROSS NOTCH CUT IN THE TOP OF CONC. CURB ON THE W. SIDE OF W. LUCENT LN./FREEDOM DR. +-610' N. OF BARKEI LN. NEAR THE NW. COR. OF OUR SITE (W. SIDE OF STREET). "X" IS OPP. THE NW'LY MOST ENTRANCE NEAR THE NW. COR. OF OUR SITE @ 1960 NAPERVILLE WHEATON RD. "X" IS +-0.20'E. OF THE W. BACK OF CURB OF W. LUCENT LN. (NEAR NW. COR. OF SITE)

ELEVATION = 739.86

SITE BENCHMARK 2 (SBM 2) (JHA CP 3) - MAG NAIL SET IN ASPH.

SITE BENCHMARK 2 IS A MAG NAIL SET IN THE ASPH. PATH RUNNING E-W IN THE N'LY. R.O.W. OF WEATHERBEE LN.(AKA LUCENT LN.) NEAR THE NW. COR. OF THE INTERSECTION OF WEATHERBEE LN. & NAPERVILLE RD. NAIL IS +-210' NW. OF THE CENTERLINE OF NAPERVILLE RD. NAIL IS +-7.30' NW. OF THE W. SIDE OF THE DRIVEWAY ENT. 2104-2114 WEATHERBEE LN. (MULTI FAMILY UNITS). NAIL IS +-0.50' NE. OF THE S'LY. EDGE OF PAVE OF THE ASPH. PATH. (NEAR NE. COR. OF SITE)

ELEVATION = 738.01

SITE BENCHMARK 3 (SBM 3) (JHA CP 4) - MAG NAIL SET IN ASPH.

SITE BENCHMARK 3 IS A MAG NAIL SET IN THE ASPH. PATH RUNNING N-S IN THE W'LY. R.O.W. OF NAPERVILLE RD. NEAR THE NW. COR. OF THE INTERSECTION OF NAPERVILLE RD. & WARRENVILLE RD. NAIL IS +-150' NE. OF THE CENTERLINE OF WARRENVILLE RD. NAIL IS +-26' NE. OF A TRAFFIC SIGNAL POST ON A PIECE OF CONC. WALK CONNECTING THE ASPH PATH TO THE CROSSWALK AT THE BACK OF CURB AND NAIL IS +-0.40' W. OF THE E'LY. EDGE OF PAVEMENT OF THE ASPH. PATH. (NEAR THE SE. COR. OF SITE)

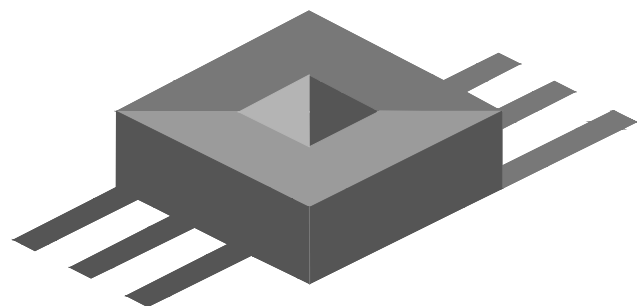
ELEVATION = 736.11

SITE BENCHMARK 4 (SBM 4) (JHA CP 1) - MAG NAIL SET IN ASPH.

SBM 4 IS A MAG NAIL SET IN THE ASPH. PATH RUNNING E-W ALONG THE N. R.O.W. OF WARRENVILLE RD. NEAR THE SW. COR. OF OUR SITE, ALSO BEING NEAR THE NE. COR. OF THE INTERSECTION OF WARRENVILLE RD. & LUCENT LN./FREEDOM DR. NAIL IS +-57' E. OF THE POINT WHERE THE W. END OF THE ASPH. PATH CONNECTS TO THE CONC. WALK AT THAT INTERSECTION. NAIL IS +-41' NE. OF A STORM MANHOLE IN THE RIGHT TURN LANE TO TURN FROM WARRENVILLE RD. ON TO LUCENT LN./FREEDOM DR. (NEAR SW. COR. OF SITE)

ELEVATION = 755.74

No.	Description	Date
4	REVISED PER CITY COMMENTS	09/26/25
3	REVISED PER CITY COMMENTS	07/31/25
2	REVISED PER CITY COMMENTS	06/12/25
1	ORIGINAL PLAN DATE	04/07/25



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ASSOCIATES

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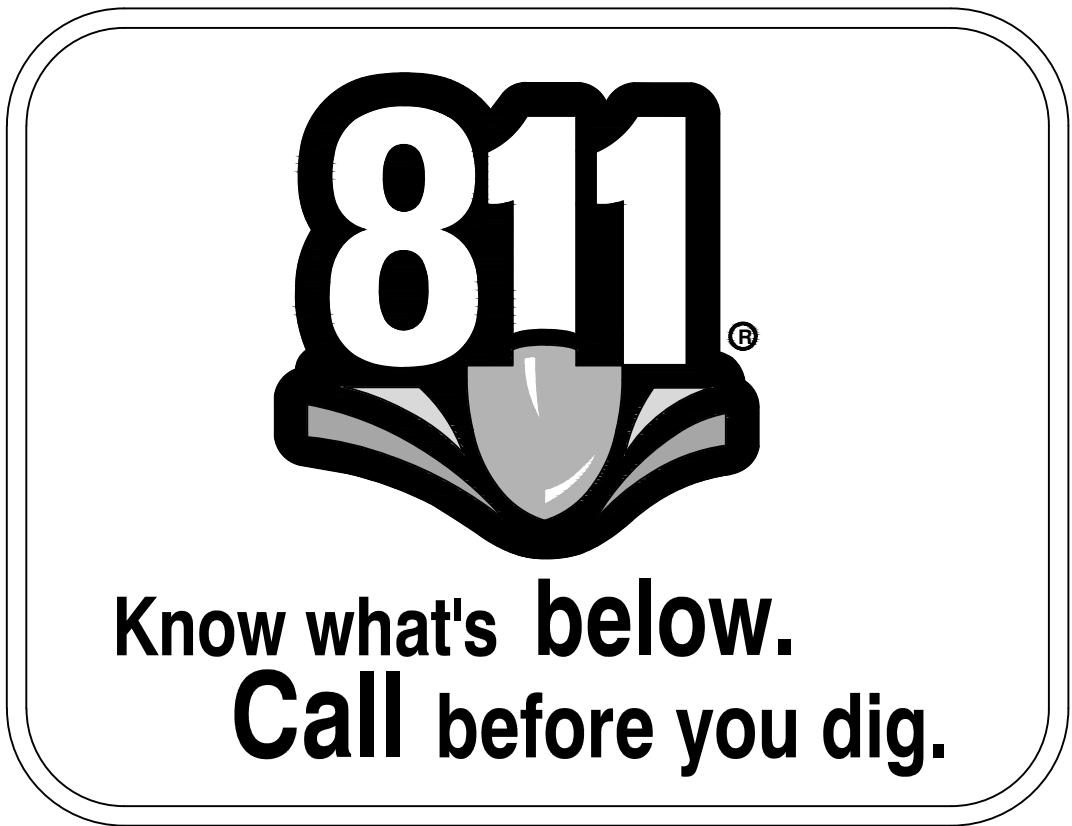
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www.jacobandhefner.com

FOR REVIEW PURPOSES ONLY



Municipality: Naperville
County: DuPage
Township: 38N
Range: 10E
Section: 5



H:\V\14771\EDwg\Pre\Sheets\H477a-Pre\2-General Notes and Typical Sections.dwg
H: 1/4/77 E.Dwg\Pre\Sheets\H477a-Pre\2-General Notes and Typical Sections.dwg

GENERAL NOTES		EARTHWORK		SANITARY SEWER		PAVEMENT		CONTACTS																																																			
1. ALL PAVING AND RELATED CONSTRUCTION SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, LATEST EDITION, AS ADOPTED BY THE ILLINOIS DEPARTMENT OF TRANSPORTATION, ALL AMENDMENTS THERETO AND IN ACCORDANCE WITH THE LATEST EDITION OF THE SUBDIVISION REGULATIONS OF THE MUNICIPALITY. IN CASE OF CONFLICT, THE MORE STRINGENT CODE SHALL TAKE PRECEDENCE. 2. ALL STORM SEWER, SANITARY SEWER AND WATER MAIN CONSTRUCTION SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR WATER AND SEWER MAIN CONSTRUCTION IN ILLINOIS, LATEST EDITION AND IN ACCORDANCE WITH THE CURRENT SUBDIVISION REGULATIONS OF THE MUNICIPALITY UNLESS OTHERWISE NOTED ON THE PLANS. 3. STANDARD SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND REQUIRING SPECIAL PROVISIONS, CONSTRUCTION PLANS, AND SUBSEQUENT DETAILS ARE ALL TO BE CONSIDERED AS PART OF THE CONTRACT. INCIDENTAL ITEMS OR ACCESSORIES NECESSARY TO COMPLETE THIS WORK MAY NOT BE SPECIFICALLY NOTED BUT ARE CONSIDERED A PART OF THIS CONTRACT. 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR HAVING THE UTILITY COMPANIES LOCATE THEIR FACILITIES IN THE FIELD PRIOR TO CONSTRUCTION AND SHALL ALSO BE RESPONSIBLE FOR THE MAINTENANCE AND PRESERVATION OF THESE FACILITIES. THE ENGINEER DOES NOT WARRANT THE LOCATION OF ANY EXISTING UTILITIES SHOWN ON THE PLANS. THE CONTRACTOR SHALL CALL J.U.L.I.E. AT 800-892-0123 AND THE MUNICIPALITY FOR UTILITY LOCATIONS. 5. NO CONSTRUCTION PLANS SHALL BE USED FOR CONSTRUCTION UNLESS SPECIFICALLY MARKED "FOR CONSTRUCTION." PRIOR TO COMMENCEMENT OF CONSTRUCTION, THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AFFECTING THE WORK WITH THE ACTUAL CONDITIONS AT THE JOB SITE. IN ADDITION, THE CONTRACTOR MUST VERIFY THE ENGINEER'S LINE AND GRADE STAKES. IF THERE ARE ANY DISCREPANCIES WITH WHAT IS SHOWN ON THE CONSTRUCTION PLANS, HE MUST IMMEDIATELY REPORT SAME TO ENGINEER. BEFORE COMMENCING ANY WORK, OTHERWISE THE CONTRACTOR ASSUMES FULL RESPONSIBILITY. IN THE EVENT OF DISAGREEMENT BETWEEN THE CONSTRUCTION PLANS, SPECIFICATIONS AND/OR SPECIAL DETAILS, THE CONTRACTOR SHALL SECURE WRITTEN INSTRUCTION FROM THE ENGINEER PRIOR TO PROCEEDING WITH ANY PART OF THE WORK AFFECTED BY OMISSIONS OR DISCREPANCIES. FAILING TO SECURE SUCH INSTRUCTION, THE CONTRACTOR WILL BE CONSIDERED TO HAVE PROCEEDED AT HIS OWN RISK AND EXPENSE. IN THE EVENT OF ANY DOUBT OR QUESTIONS ARISING WITH RESPECT TO THE TRUE MEANING OF THE CONSTRUCTION PLANS OR SPECIFICATIONS, THE DECISION OF THE ENGINEER SHALL BE FINAL AND CONCLUSIVE. 6. NOTIFICATION OF COMMENCING CONSTRUCTION A. THE CONTRACTOR SHALL NOTIFY THE OWNER AND/OR HIS REPRESENTATIVE AND THE AFFECTED GOVERNMENTAL AGENCIES IN WRITING AT LEAST THREE FULL WORKING DAYS PRIOR TO COMMENCEMENT OF CONSTRUCTION. IN ADDITION, THE CONTRACTOR SHALL NOTIFY AS NECESSARY, ALL TESTING AGENCIES, EITHER MUNICIPALITY'S OR THE OWNER'S, SUFFICIENTLY IN ADVANCE OF CONSTRUCTION. B. FAILURE OF THE CONTRACTOR TO ALLOW PROPER NOTIFICATION TIME, WHICH RESULTS IN TESTING COMPANIES TO BE UNABLE TO VISIT THE SITE AND PERFORM TESTING, WILL CAUSE THE CONTRACTOR TO SUSPEND OPERATION (PERTAINING TO TESTING) UNTIL THE TESTING AGENCY CAN SCHEDULE TESTING OPERATIONS. THE COST OF SUSPENSION OF WORK TO BE BORNE BY THE CONTRACTOR. 7. ALL CONTRACTORS SHALL KEEP ACCESS AVAILABLE AT ALL TIMES, FOR ALL TYPES OF TRAFFIC. AT NO TIME SHALL ACCESS BE DENIED TO PROPERTIES SURROUNDING THE SITE. 8. ALL PROPOSED ELEVATIONS SHOWN ON THE PLANS ARE FINISHED SURFACE ELEVATIONS, UNLESS OTHERWISE SPECIFIED. 9. THE CONTRACTOR SHALL PRESERVE ALL CONSTRUCTION STAKES UNTIL THEY ARE NO LONGER NEEDED, ANY STAKES DESTROYED OR DISTURBED BY THE CONTRACTOR PRIOR TO THEIR USE SHALL BE RESET AT THE CONTRACTOR'S COST. 10. ALL FRAMES AND LIDS FOR STORM AND SANITARY SEWER STRUCTURES ARE TO BE ADJUSTED TO MEET FINAL FINISHED GRADE. THIS ADJUSTMENT IS TO BE MADE BY THE SEWER CONTRACTOR AND THE COST IS TO BE CONSIDERED INCIDENTAL. THESE ADJUSTMENTS TO FINISHED GRADE WILL NOT ALLOCATE THE CONTRACTOR FROM ANY ADDITIONAL ADJUSTMENTS AS REQUIRED BY THE MUNICIPALITY UPON FINAL INSPECTION OF THE PROJECT. FINAL GRADES ARE TO BE DETERMINED BY THE MUNICIPALITY AT THE TIME OF FINAL INSPECTION AND MAY VARY FROM PLAN GRADE. 11. ANY EXISTING SIGNS, LIGHT STANDARDS AND UTILITY POLES WHICH INTERFERE WITH CONSTRUCTION OPERATIONS AND NOT NOTED FOR DISPOSAL, SHALL BE REMOVED AND RESET BY THE CONTRACTOR AT HIS OWN EXPENSE, AS DIRECTED BY THE ENGINEER. ANY DAMAGE TO THESE ITEMS SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR, AT HIS OWN EXPENSE, TO THE SATISFACTION OF THE OWNER. ANY SIGNS NOT REQUIRED TO BE RESET SHALL BE DELIVERED TO THE RESPECTIVE OWNERS. 12. REMOVAL OF SPECIFIED ITEMS, INCLUDING BUT NOT LIMITED TO, PAVEMENT, SIDEWALK, CURB AND GUTTER, CULVERTS, ETC., SHALL BE DISPOSED OF OFF-SITE BY THE CONTRACTOR AT HIS OWN EXPENSE. THE CONTRACTOR IS RESPONSIBLE FOR ANY PERMIT REQUIRED FOR SUCH DISPOSAL. 13. ALL FIELD LIEU ENCOUNTERED DURING CONSTRUCTION OPERATIONS SHALL BE CONNECTED TO THE PROPOSED STORM SEWER SYSTEM OR SHALL BE RESTORED TO ORIGINAL CONDITION. A RECORD OF THE LOCATION OF ALL FIELD LIEUS OR DRAIN PIPES UNLOCATED SHALL BE KEPT BY THE CONTRACTOR AND TURNED OVER TO THE ENGINEER UPON COMPLETION OF THE PROJECT. THE COST OF THIS WORK SHALL BE CONSIDERED AS INCIDENTAL TO THE CONTRACT AND NO ADDITIONAL COMPENSATION WILL BE ALLOWED BY OWNER AND MUNICIPALITY. 14. ALL WORK PERFORMED UNDER THIS CONTRACT SHALL BE GUARANTEED BY THE CONTRACTOR AND HIS SURETY FOR A PERIOD OF 12 MONTHS FROM THE DATE OF FINAL ACCEPTANCE OF THE PROJECT. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR ALL DEFECTS IN MATERIALS AND WORKMANSHIP OF WHATEVER NATURE DURING THAT PERIOD. 15. BEFORE ACCEPTANCE BY THE OWNER AND FINAL PAYMENT, ALL WORK SHALL BE INSPECTED BY THE OWNER OR HIS REPRESENTATIVE. FINAL PAYMENT WILL BE MADE AFTER ALL THE CONTRACTOR'S WORK HAS BEEN APPROVED AND ACCEPTED BY THE OWNER AND MUNICIPALITY. 16. UPON AWARDING OF THE CONTRACT AND WHEN REQUIRED BY THE MUNICIPALITY, THE CONTRACTOR SHALL FURNISH A LABOR, MATERIAL AND PERFORMANCE BOND WITH INSURANCE, IN THE AMOUNT REQUIRED BY THE MUNICIPALITY, GUARANTEEING COMPLETION OF THE WORK. THE UNDERWRITER SHALL BE ACCEPTABLE TO THE MUNICIPALITY. 17. EASEMENTS FOR THE EXISTING UTILITIES, BOTH PUBLIC AND PRIVATE, AND UTILITIES WITHIN PUBLIC RIGHTS-OF-WAY ARE SHOWN ON THE PLANS ACCORDING TO KNOWN AVAILABLE RECORDS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION IN THE FIELD OF THESE UTILITY LINES AND THEIR PROTECTION FROM DAMAGE DUE TO CONSTRUCTION OPERATIONS. IF EXISTING UTILITY LINES OF ANY NATURE ARE ENCOUNTERED WHICH CONFLICT WITH THE LOCATION OF THE NEW CONSTRUCTION, JACOB AND HEFNER ASSOCIATES, INC. IS RESPONSIBLE TO RESOLVE THE CONFLICT. JACOB AND HEFNER ASSOCIATES, INC. IS NOT RESPONSIBLE FOR THE COST OF CONSTRUCTION. 18. THE OWNER SHALL OBTAIN EASEMENTS AND PERMITS NECESSARY TO FACILITATE CONSTRUCTION OF THE PROPOSED UTILITIES. THE CONTRACTOR, SHALL FURNISH ALL REQUIRED BONDS AND EVIDENCE OF INSURANCE NECESSARY TO SECURE THESE PERMITS. 19. THE CONTRACTORS SHALL PLAN THEIR WORK BASED ON THEIR OWN BORINGS, EXPLORATIONS AND OBSERVATIONS TO DETERMINE SOIL CONDITIONS AT THE LOCATION OF THE PROPOSED WORK. 20. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR SAFETY ON THE JOB SITE IN ACCORDANCE WITH OSHA REGULATIONS. 21. AT NO ADDITIONAL EXPENSE TO THE OWNER, THE CONTRACTOR SHALL COLLECT AND REMOVE ALL CONSTRUCTION DEBRIS, EXCESS MATERIALS, TRASH, OIL AND GREASE RESIDUE, MACHINERY, TOOLS, STAKES OR LATH SET BY SURVEYORS FOR CONSTRUCTION AND OTHER MISCELLANEOUS ITEMS NOT PRESENT PRIOR TO PROJECT COMMENCEMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ACQUIRING ANY AND ALL PERMITS NECESSARY FOR THE HAULING AND DISPOSAL REQUIRED FOR CLEAN-UP AS DIRECTED BY THE ENGINEER OR OWNER. BURNING ON THE SITE IS NOT PERMITTED. 22. IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO PROVIDE PROPER BARRICADING, WARNING DEVICES AND THE SAFE MANAGEMENT OF TRAFFIC AND PEDESTRIANS WITHIN THE AREA OF CONSTRUCTION. ALL SUCH DEVICES AND THEIR INSTALLATION SHALL CONFORM TO THE ILLINOIS "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREET AND HIGHWAYS," LATEST EDITION AND IN CONFORMANCE WITH REGULATIONS OF THE MUNICIPALITY OR D.O.T. 23. NO UNDERGROUND WORK SHALL BE COVERED UNTIL IT HAS BEEN APPROVED BY THE MUNICIPALITY. APPROVAL TO PROCEED MUST BE OBTAINED FROM THE MUNICIPALITY PRIOR TO INSTALLING PAVEMENT BASE, BINDER COURSE OR SURFACE COURSE AND PRIOR TO POURING ANY CONCRETE AFTER FORMS HAVE BEEN SET. 24. ALL EXISTING UTILITIES OR IMPROVEMENTS, INCLUDING WALKS, CURBS, PAVEMENT AND PARKWAYS DAMAGED OR REMOVED DURING CONSTRUCTION SHALL BE PROMPTLY RESTORED TO THEIR RESPECTIVE ORIGINAL CONDITION. 25. AT THE CLOSE OF EACH WORKING DAY AND AT THE CONCLUSION OF CONSTRUCTION OPERATIONS, ALL DRAINAGE STRUCTURES AND FLOW LINES SHALL BE FREE FROM DIRT AND DEBRIS. 26. TREES NOT MARKED FOR REMOVAL SHALL BE CONSIDERED AS DESIGNATED TO BE SAVED AND SHALL BE PROTECTED UNDER THE PROVISIONS OF ARTICLE 201.03 OF THE STANDARD SPECIFICATIONS. 27. LIMB PRUNING SHALL BE PERFORMED UNDER THE SUPERVISION OF AN APPROVED LANDSCAPE ARCHITECT AND SHALL BE UNDERTAKEN IN A TIMELY FASHION SO AS NOT TO INTERFERE WITH CONSTRUCTION. 28. ALL LIMBS, BRANCHES AND OTHER DEBRIS RESULTING FROM THIS WORK SHALL BE DISPOSED OF OFF-SITE BY THE CONTRACTOR AT HIS OWN EXPENSE. 29. ALL CUTS OVER ONE INCH IN DIAMETER SHALL BE MADE FLUSH WITH THE NEXT LARGE BRANCH. WOUNDS OVER ONE INCH IN DIAMETER SHALL BE PAINTED WITH AN APPROVED TREE PAINT. 30. ANY DEMATERING OF SEWER AND WATER TRENCHES, AS WELL AS TEMPORARY SHEETING OR BRACING THAT MAY BE REQUIRED, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL NOT BE CONSIDERED EXTRA WORK. IN THE EVENT THAT SOFT MATERIALS WITH UNCONFINED COMPRESSIVE STRENGTH LESS THAN 0.5 TSP ARE ENCOUNTERED DURING SEWER AND WATER MAIN CONSTRUCTION, THE CONTRACTOR SHALL (UPON APPROVAL OF THE OWNER AND/OR ENGINEER) OVER-EXCAVATE TO A DEPTH OF AT LEAST ONE FOOT BELOW THE BOTTOM OF THE PIPE. THE TRENCH SHALL BE BACKFILLED WITH COMPACTED CRUSHED STONE, PROPERLY FORMED TO FIT THE BOTTOM OF THE PIPE. 31. THE CONTRACTOR SHALL RECORD VIDEO OF THE WORK AREA PRIOR TO CONSTRUCTION FOR THE PURPOSE OF DOCUMENTING EXISTING CONDITIONS. 32. AGGREGATE TRENCH BACKFILL WILL BE REQUIRED TO FULL DEPTH ABOVE ALL UNDERGROUND UTILITIES WITHIN TWO FEET OF PROPOSED OR EXISTING PAVEMENT, UTILITIES, BUILDINGS AND SIDEWALKS. AGGREGATE TRENCH BACKFILL SHALL BE INSTALLED IN ACCORDANCE WITH CURRENT IDOT STANDARD SPECIFICATIONS. TRENCH BACKFILL AND BEDDING MATERIAL SHALL CONSIST OF CRUSHED GRAVEL CONFORMING TO DOT GRADATIONS CA-6 OR CA-7. AGGREGATE TRENCH BACKFILL SHALL BE PLACED IN CONTROLLED LIFTS (12 INCH LOOSE MEASURE TYPICAL FOR GRANULAR MATERIAL) TO 95 PERCENT OF THE MAXIMUM DRY DENSITY OF THE MATERIAL. THE MATERIAL SHALL BE PLACED IN LIFTS OF 6 INCHES OR LESS. IF THE MATERIAL IS NOT AVAILABLE, THE MATERIAL MAY BE REPLACED WITH A MODIFIED PROCTOR TEST (ASTM D 1557), OR IN THE CASE OF OPEN GRADED STONE, TO A FIRM, DENSE STATE AS OBSERVED BY THE GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE PRIOR TO PLACEMENT OF THE NEXT LIFT. LIFT THICKNESS ADJUSTMENTS CAN ONLY BE DETERMINED/APPROVED BY THE GEOTECHNICAL ENGINEER (OR HIS REPRESENTATIVE) BASED ON THE SIZE/TYPE OF COMPACTING EQUIPMENT USED. 33. WHERE SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER, EXISTING DRAINAGE STRUCTURES AND SYSTEMS SHALL BE CLEANED OF DEBRIS AND PATCHED AS NECESSARY TO ASSURE INTEGRITY OF THE STRUCTURE. 34. HYDRANTS SHALL NOT BE FLUSHED DIRECTLY ONTO THE ROAD SUBGRADE. WHEREVER POSSIBLE, HOSES SHALL BE USED TO DIRECT THE WATER INTO STORM SEWERS. DAMAGE TO THE ROAD SUBGRADE OR LOT AREAS DUE TO EXCESSIVE WATER SATURATION AND/OR EROSION FROM HYDRANT FLUSHING OR LEAKS IN THE WATER DISTRIBUTION SYSTEM, WILL BE THE RESPONSIBILITY OF THE CONTRACTOR. FLUSHING OR USING THE HYDRANT TO MAKE ALL NECESSARY REPAIRS WILL BE AT HIS EXPENSE. THE CONTRACTOR SHALL PROVIDE ALL CONSTRUCTION WATER AT HIS EXPENSE. 35. AFTER THE STORM SEWER SYSTEM HAS BEEN CONSTRUCTED, THE CONTRACTOR SHALL PLACE EROSION CONTROL AT LOCATIONS SHOWN ON THE PLANS OR AS SELECTED IN THE FIELD BY THE ENGINEER. INLET PROTECTION (INLET BASKETS) SHALL BE INSTALLED IN EACH STRUCTURE AS SOON AS THE STRUCTURE INSTALLATION IS COMPLETE. THE PURPOSE OF THE EROSION CONTROL WILL BE TO MINIMIZE THE AMOUNT OF SILTATION, WHICH NORMALLY ENTERS THE STORM SEWER SYSTEM FROM ADJACENT AND/OR UPSTREAM DRAINAGE AREAS. 36. EROSION CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH ILLINOIS URBAN MANUAL AND SHALL BE MAINTAINED BY THE CONTRACTOR. EROSION CONTROL MEASURES SHALL REMAIN IN PLACE UNTIL A SUITABLE GROWTH OF GRASS, ACCEPTABLE TO THE ENGINEER, HAS BEEN DEVELOPED. 37. UPON PROJECT COMPLETION, THE CONTRACTOR SHALL PROVIDE FINAL RECORD DRAWINGS TO THE OWNER AND ENGINEER FOR REVIEW PRIOR TO ANY REQUEST FOR FINAL INSPECTION. AT A MINIMUM, THE RECORD DRAWINGS SHALL INDICATE THE FINAL LOCATION AND LAYOUT OF ALL SITE IMPROVEMENTS, INCLUDING BUT NOT LIMITED TO, VERIFICATION OF ALL BUILDING FOOTPRINTS, LOCATIONS OF ALL UTILITIES, UTILITY ROW AND INVERT ELEVATIONS, KEY PAVEMENT SPOT GRADE ELEVATIONS, LOCATIONS OF ALL WATER SERVICE B-BOXES, SANITARY SEWER SERVICES, AND STORM SEWER SERVICES AND SHALL INCORPORATE ALL FIELD DESIGN CHANGES INCLUDING THOSE APPROVED BY THE OWNER, ENGINEER AND/OR PROJECT GOVERNING AUTHORITY. THE RECORD DRAWINGS SHALL ALSO INCORPORATE ANY FIELD DESIGN CHANGES THAT WERE NOT APPROVED BY OWNER, ENGINEER AND/OR PROJECT GOVERNING AUTHORITY. RECORD DRAWINGS SHALL BE PREPARED BY OR UNDER THE DIRECT SUPERVISION OF A LICENSED PROFESSIONAL LAND SURVEYOR.		1. TOPSOIL EXCAVATION A. TOPSOIL, ORGANIC MATERIAL, OR ANY OTHER UNSUITABLE MATERIALS SHALL BE REMOVED FROM AREAS REQUIRING STRUCTURAL FILL. B. PLACEMENT OF EXCAVATED MATERIAL SHALL BE DESIGNATED BY THE OWNER FOR FUTURE USE WITHIN AREAS TO BE LANDSCAPED OR AS FILL IN THE AREAS NOT REQUIRING STRUCTURAL FILL MATERIAL. C. EXCESS MATERIALS NOT UTILIZED AS FILL OR NOT STOCKPILED FOR FUTURE LANDSCAPING, SHALL BE COMPLETELY REMOVED FROM THE CONSTRUCTION SITE AND DISPOSED OF OFF-SITE BY THE CONTRACTOR. 2. EARTH EXCAVATION A. EXCAVATION OF EARTH AND OTHER MATERIALS, WHICH ARE SUITABLE FOR USE AS STRUCTURAL FILL, SHALL BE WITHIN A TOLERANCE OF 0.05 FEET FOR PADS AND PAVEMENT, AT 0.1 FEET +/- OF THE PLAN SUBGRADE ELEVATIONS. THE +/- TOLERANCE WITHIN PAVEMENT AREAS UTILIZING EARTH MATERIALS SHALL "BALANCE" AS PART OF THE FINE GRADING OPERATION. B. PLACEMENT OF EARTH AND OTHER SUITABLE MATERIALS SHALL BE PLACED WITHIN THOSE AREAS REQUIRING STRUCTURAL FILL TO ACHIEVE THE PLAN SUBGRADE ELEVATIONS WITHIN A TOLERANCE OF 0.1 FEET +/- OF THE FILL MATERIAL. SHALL BE PLACED IN LOOSE LIFTS, NOT EXCEED EIGHT INCHES IN THICKNESS. THE WATER CONTENT SHALL BE ADJUSTED IN ORDER TO ACHIEVE THE REQUIRED COMPACTION. EARTH MATERIAL MAY BE PLACED WITHIN THOSE AREAS OF THE SITE NOT REQUIRING STRUCTURAL FILL, WITHIN THE PLAN SUBGRADE ELEVATION. IN AREAS REQUIRING STRUCTURAL FILL, THE EARTH MATERIAL SHALL NOT BE PLACED OVER TOPSOIL OR OTHER UNSUITABLE MATERIALS. THE STRUCTURAL SUBGRADE AREA SHALL EXTEND TO THE ZONE OF INFLUENCE IN ALL FILL AREAS. C. COMPACTION OF THE EARTH AND OTHER SUITABLE MATERIALS SHALL BE TO A MINIMUM OF 95% OF THE MODIFIED PROCTOR DRY DENSITY. THIS INCLUDES AREAS WITHIN PROPOSED PAVEMENT AND BUILDING PAD LOCATIONS, SIDEWALKS, ETC. IN NON-STRUCTURAL FILL AREAS, 90% TO 95% OF THE MODIFIED PROCTOR DRY DENSITY IS REQUIRED. 3. UNSUITABLE MATERIAL A. UNSUITABLE MATERIAL SHALL BE CONSIDERED AS MATERIAL THAT IS NOT SUITABLE FOR THE SUPPORT OF PAVEMENT AND BUILDING CONSTRUCTION. IF IT IS ENCOUNTERED BELOW NORMAL TOPSOIL DEPTHS AND THE PROPOSED SUBGRADE ELEVATION, IT SHALL BE REMOVED AND REPLACED WITH SELECT GRANULAR MATERIAL APPROVED BY THE SOILS ENGINEER. THE DECISION TO REMOVE SAID MATERIAL AND TO WHAT EXTENT, SHALL BE MADE BY A SOILS ENGINEER WITH THE CONCURRENCE OF THE OWNER. 4. THE GRADING CONTRACTORS RESPONSIBILITIES A. MAINTAIN POSITIVE DRAINAGE AT ALL TIMES DURING THE COURSE OF CONSTRUCTION AND PREVENT STORM WATER FROM RUNNING INTO OR STANDING IN EXCAVATED AREAS. B. SPREAD AND COMPACT UNIFORMLY ALL EXCESS TRENCH SPOIL, AS SPECIFIED, AFTER COMPLETION OF THE UNDERGROUND IMPROVEMENTS. C. SCARIFY AND COMPACT THE UPPER 12 INCHES OF THE SUITABLE SUBGRADE MATERIAL, AS SPECIFIED, IN ALL AREAS THAT MAY BE SOFT DUE TO EXCESS MOISTURE CONTENT. THIS APPLIES TO CUT AREAS AS WELL AS FILL AREAS. D. PROVIDE ADDITIONAL WATER TO DRY MATERIAL TO ADJUST THE MOISTURE CONTENT FOR THE PURPOSE OF ACHIEVING THE SPECIFIED COMPACTION. E. BACKFILL THE CURB AND GUTTER AFTER CONSTRUCTION AND PRIOR TO THE PLACEMENT OF THE BASE COURSE MATERIAL. F. ACCOUNTABLE FOR IMPLEMENTATION OF THE "SOIL EROSION AND SEDIMENTATION CONTROL MEASURES" AS DESCRIBED ON THE PLANS. ALL CONTRACTORS SHALL COMPLY WITH THE SWPPP AND NPDES REQUIREMENTS. G. PERFORM LIME STABILIZATION OF THE SUBGRADE MATERIAL, IF REQUIRED BY THE SOILS ENGINEER. 5. TESTING AND FINAL ACCEPTANCE A. THE CONTRACTOR SHALL PROVIDE, AS A MINIMUM, A TANDDEM AXLE TRUCK LOADED TO 14 TONS FOR PROOF ROLLING THE PAVEMENT SUBGRADE. PROOF ROLLING SHALL BE PERFORMED PRIOR TO THE PLACEMENT OF THE CURB AND GUTTER AND BASE MATERIAL. THIS SHALL BE WITNESSED AND APPROVED BY THE MUNICIPAL ENGINEER AND OWNER. B. ANY UNSUITABLE AREA ENCOUNTERED AS A RESULT OF PROOF ROLLING, SHALL BE REMOVED AND REPLACED WITH SUITABLE MATERIAL APPROVED BY THE SOILS CONSULTANT. PROOF ROLLING SHALL BE PERFORMED UNTIL THE SUBGRADE IS APPROVED BY THE MUNICIPAL ENGINEER, OWNER AND SOILS ENGINEER. C. THE WORK AREAS SHALL BE POSITIVELY DRAINED DURING CONSTRUCTION. FINAL GRADES SHALL BE PROTECTED AGAINST DAMAGE FROM EROSION, SEDIMENTATION AND TRAFFIC.		1. UNLESS NOTED OTHERWISE, ALL SANITARY SEWER SHALL BE RING-TITE PVC (POLYVINYL CHLORIDE) PIPES CONFORMING TO ASTM D-3034 WITH ELASTOMERIC RUBBER RING GASKET JOINTS CONFORMING TO ASTM D3212. THE STANDARD DIMENSION RATIO (SDR) FOR SANITARY PIPE SHALL BE 26, WHERE SPECIFIED. PVC SDR 21 SANITARY SEWER SHALL BE RING-TITE PVC (POLYVINYL CHLORIDE) PLASTIC PIPE, CONFORMING TO ASTM D-2241 WITH ELASTOMERIC RUBBER RING GASKET JOINTS CONFORMING TO ASTM D3139. THE PVC D1818 SANITARY SEWER SHALL CONFORM TO ANMA C900/C905, WITH RUBBER GASKET JOINTS CONFORMING TO ANMA C900/C905. 2. CONNECTING SEWER PIPE OF DISSIMILAR MATERIAL IS NOT PERMITTED. 3. ALL FLOOR DRAINS SHALL CONNECT TO THE SANITARY SEWER. CONNECTIONS TO EXISTING SANITARY SEWER SYSTEM SHALL NOT BE COMPLETED UNTIL AUTHORIZED BY THE MUNICIPALITY. 4. ALL UNSUITABLE MATERIAL SHALL BE REMOVED BELOW THE PROPOSED SANITARY SEWER AND REPLACED WITH COMPACTED CRUSHED GRAVEL, CONFORMING TO ASTM D-2321, CL 1. 5. COST FOR PIPE BEDDING SHALL BE MERGED WITH THE UNIT PRICE BID FOR THE SEWER. 6. PIPE BEDDING SHALL CONSIST OF A MINIMUM OF FOUR INCHES OF COMPACTED CRUSHED GRAVEL OR STONE FOR ALL SANITARY SEWERS. SANITARY SEWER PIPE SHALL HAVE TAMPED CRUSHED GRAVEL OR STONE COVER ABOVE THE TOP OF PIPE, TO A MINIMUM OF 12 INCHES. THE BEDDING AND TRENCH BACKFILL MATERIAL SHALL CONFORM TO IDOT GRADATION CA-6 AND INSTALLED PER ASTM D-2321 CLASS 1. 7. WATER MAINS SHALL BE SEPARATED FROM SANITARY SEWERS AND STORM SEWERS IN ACCORDANCE WITH IEPA REQUIREMENTS AS SPECIFIED IN THE "WATER MAIN" SECTION. 8. NO WATER LINE SHALL BE PLACED IN THE SAME TRENCH AS A SEWER LINE. 9. THE TESTING OF THE PIPE STRAIGHTNESS AND FIELD TESTING, SHALL BE IN ACCORDANCE WITH THE DEVELOPMENT CODE OF THE MUNICIPALITY. 10. SANITARY SEWER MANHOLES SHALL BE FOUR FOOT INNER DIAMETER MINIMUM, PRECAST REINFORCE CONCRETE RINGS OR MONOLITHIC CONCRETE, CONFORMING TO ASTM C 478. THE STRUCTURE SHALL HAVE AN ECCENTRIC COLE INSTALLED, LINING UP WITH THE MANHOLE STEPS. ALL MANHOLE STEPS SHALL BE NENAH R-1981-I AT 16 INCHES ON-CENTER. 11. ALL SANITARY SEWER MANHOLE LIDS SHALL BE NENAH R-1713-B OR APPROVED EQUAL. THE LIDS SHALL HAVE RECESSED (CONCEALED) PICK HOLE AND BE SELF SEALING WITH "OT" RING GASKET. THE LIDS SHALL HAVE THE WORD "SANITARY" AND THE MUNICIPALITY NAME EMBOSSED ON THE LID. 12. ALL MANHOLE JOINTS SHALL BE SEALED WITH "OT" RING JOINTS. NO MORE THAN EIGHT INCHES OF PRECAST CONCRETE ADJUSTING RINGS SHALL BE USED TO ADJUST FRAME ELEVATIONS. ALL MANHOLES SHALL HAVE MISSION COUPLING TYPE FITTINGS FOR PIPE CONNECTIONS. 13. DROP MANHOLE ASSEMBLIES SHALL BE PROVIDED AT THE JUNCTION OF SANITARY SEWERS WHERE THE DIFFERENCE IN INVERT GRADES EXCEEDS TWO FEET OR AT LOCATIONS SHOWN ON THE PLANS. THE ENTIRE DROP ASSEMBLY SHALL BE CAST IN CONCRETE MONOLITHICALLY WITH THE MANHOLE BARREL. 14. INSPECTION OF MANHOLES: A. ALL MANHOLES SHALL BE THOROUGHLY CLEANED OF DIRT AND DEBRIS. ALL VISIBLE LEAKAGE SHALL BE ELIMINATED BEFORE FINAL INSPECTION AND ACCEPTANCE. B. TESTING FOR ACCEPTANCE OF SANITARY SEWERS AND MANHOLES, INCLUDING SERVICE LINES, SHALL INCLUDE LOAD, LOW PRESSURE AIR TEST AND DEFLECTION TEST PER STANDARD SPECIFICATIONS AND SUBDIVISION REGULATIONS OF THE MUNICIPALITY. THE TESTS SHALL BE APPROVED BY THE MUNICIPALITY BEFORE ACCEPTANCE. ALL SANITARY MANHOLES SHALL BE VACUUM TESTED FOR LEAKAGE IN ACCORDANCE WITH MUNICIPAL STANDARDS AND ASTM C 1244-02. C. TELEVISION TESTING OF ALL SANITARY SEWERS SHALL BE TELEVISED. A COPY OF THE RECORDING AND A WRITTEN REPORT SHALL BE SUBMITTED TO THE MUNICIPALITY FOR APPROVAL. THE REPORT SHALL INCLUDE STUB LOCATIONS, A DESCRIPTION OF ALL DEFECTS, WATER LEVEL, LEAKS AND IDENTIFICATION OF LENGTH FROM THE MANHOLE NUMBER SPECIFIED ON THE APPROVED PLANS. ALL COSTS SHALL BE INCIDENTAL TO THE WORK. TESTING SHALL BE WITNESSED AND APPROVED BY THE MUNICIPALITY BEFORE FINAL ACCEPTANCE. THE LOCATION OF TELEVISION INSPECTION SHALL BE DESIGNATED BY THE MUNICIPAL ENGINEER. 17. IF THE SANITARY SEWER INSTALLATION FAILS TO MEET THE TEST REQUIREMENTS SPECIFIED, THE CONTRACTOR SHALL REPAIR OR REPLACE ALL MATERIALS AND WORKMANSHIP AS NECESSARY TO COMPLY WITH THE TEST REQUIREMENTS. 18. THE CERTIFICATION CONTRACTOR SHALL SUBMIT CERTIFIED COPIES OF ALL REPORTS OF THE TESTS CONDUCTED BY AN INDEPENDENT LABORATORY BEFORE INSTALLATION OF PVC PLASTIC PIPE. TESTS SHALL BE CONDUCTED IN ACCORDANCE WITH THE STANDARD METHOD OF TESTING FOR "EXTERNAL LOADING PROPERTIES OF PLASTIC PIPE BY PARALLEL PLATE LOADING," ASTM STANDARD D-2412. TESTS SHALL ALSO BE CONDUCTED IN ACCORDANCE WITH ASTM D-3212, TO DEMONSTRATE JOINT PERFORMANCE. AT 5% MAXIMUM DIAMETRIC DEFLECTION OF THE SPIGOT, AS SPECIFIED IN ASTM D-3212 SPECIFICATIONS.		1. FINE GRADING A. PRIOR TO THE CONSTRUCTION OF CURB AND GUTTER AND PLACEMENT OF THE BASE MATERIAL, THE STREETS SHALL BE FINE GRADED TO WITHIN 0.05 FEET OF FINAL SUBGRADE ELEVATION, TO A POINT TWO FEET BEYOND THE BACK OF CURB. 2. CURB AND GUTTER A. THE TYPE OF THE CURB AND GUTTER SHALL BE AS DETAILED ON THE ENGINEERING PLANS. B. THE CURBS SHALL BE BACKFILLED AFTER CONSTRUCTION AND PRIOR TO THE PLACEMENT OF THE BASE COURSE. C. THE STONE COST UNDER THE CURB AND GUTTER SHALL BE CONSIDERED INCIDENTAL. D. DEPRESSED CURBS FOR DRIVEWAYS AND HANDICAPPED RAMPS SHALL BE INSTALLED PER THE PLANS AND IDOT STANDARDS. 3. CONCRETE PAVEMENT A. CONSTRUCTION JOINTS SHALL INCLUDE DOWELS PER "CONSTRUCTION JOINT TABLE" ALL DOWELS SHALL BE SMOOTH EPOXY COATED; GREASE ONE END. B. CONTRACTOR SHALL PROVIDE CONTRACTION JOINT 5' OFF OF FACE OF BUILDING. C. EXCEPT AT FIRST PANEL FROM THE FACE OF BUILDING, CONCRETE PANEL LENGTH SHALL NOT EXCEED ITS WIDTH BY MORE THAN 25%. D. CONTRACTION JOINTS SHALL BE CUT TO A DEPTH D/4 FOR CONVENTIONAL CONCRETE, OR D/3 FOR FIBER REINFORCED CONCRETE. WIDTH OF SAW-CUT SHALL BE PER JOINT SEALANT MATERIAL MANUFACTURER'S RECOMMENDATIONS. E. ALL JOINTS SHALL BE CAULKED -SWAGE-7285 (GROUT JOINTS) -SIXFLEX-16/35 (EXPANSION JOINTS) -OR APPROVED EQUAL F. CURB & GUTTER WITH CONCRETE PAVEMENT AREAS SHALL BE MONOLITHICALLY POURED OR SHALL BE TIED TO THE FIRST PANEL. <table><thead><tr><th colspan="3">DOWEL SIZE AND SPACING AT CONSTRUCTION JOINTS</th></tr><tr><th>PAVEMENT DEPTH (INCH)</th><th>CONSTRUCTION JOINT DOWEL DIMENSIONS (INCH)</th><th>DOWEL SPACING C-C (INCH)</th></tr></thead><tbody><tr><td>5 to <6</td><td>3/4 x 10</td><td>12</td></tr><tr><td>6 to <8</td><td>1 x 13</td><td>12</td></tr><tr><td>8 to <10</td><td>1-1/4 x 15</td><td>12</td></tr><tr><td>10 to 12</td><td>1-1/2 x 18</td><td>12</td></tr></tbody></table> 3. PAVEMENT A. THE PAVEMENT MATERIALS SHALL BE AS DETAILED ON THE ENGINEERING PLANS. DEPTHS SPECIFIED SHALL BE CONSIDERED THE MINIMUM COMPACTED THICKNESS. 4. GENERAL A. THE PAVING CONTRACTOR SHALL: 1. REPAIR ANY BASE COURSE AND BINDER COURSE FAILURES PRIOR TO THE INSTALLATION OF THE FINAL BITUMINOUS CONCRETE SURFACE COURSE. 2. SWEEP CLEAN THE BINDER COURSE PRIOR TO THE INSTALLATION OF THE FINAL BITUMINOUS CONCRETE SURFACE COURSE. ANY DAMAGE TO THE BINDER COURSE SHALL BE REPAIRED BY THE CONTRACTORS AT NO ADDITIONAL COST TO THE OWNER. 3. PROVIDE CONSTRUCTION, EXPANSION AND CONTRACTION JOINTS FOR CURB AND GUTTER AND P.C.C. SIDEWALK PER DOT STANDARDS AND MUNICIPAL STANDARDS. 4. REMOVE ALL EXCESS MATERIALS AND DEBRIS. DISPOSE OF MATERIALS OFF-SITE AT NO ADDITIONAL COST TO THE OWNER. 5. TESTING AND FINAL ACCEPTANCE A. PRIOR TO THE PLACEMENT OF THE BASE COURSE, THE SUBGRADE MUST BE PROOF ROLLED AND INSPECTED FOR UNSUITABLE LOCATIONS. IF UNSUITABLE SUBGRADE IS ENCOUNTERED, IT SHALL BE REMOVED AND REPLACED WITH GRANULAR MATERIAL, APPROVED BY THE OWNER OR HIS REPRESENTATIVE. THE SUBGRADE SHALL HAVE A MINIMUM IIR VALUE OF 3.0. B. PRIOR TO PLACEMENT OF THE BITUMINOUS CONCRETE SURFACE COURSE, THE CONTRACTOR SHALL OBTAIN SPECIMENS OF THE BINDER COURSE WITH A CORE DRILL WHERE DIRECTED AND AS REQUIRED. THICKNESS VERIFICATION WILL BE IN ACCORDANCE WITH THE "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION." C. FINAL ACCEPTANCE OF THE TOTAL PAVEMENT INSTALLATION SHALL BE SUBJECT TO THE TESTING AND VERIFICATION REQUIREMENTS CITED ABOVE. 6. METHOD OF MEASUREMENT A. CURB AND GUTTER AND BASE COURSE SHALL BE MEASURED IN THE FIELD BY THE CONTRACTOR. THE QUANTITIES SHALL BE SUBMITTED TO THE ENGINEER FOR VERIFICATION WHEN REQUESTED BY THE OWNER. B. WHEN REQUESTED BY THE OWNER, DOCUMENTATION FOR THE INSTALLED BASE COURSE, BITUMINOUS CONCRETE BINDER AND SURFACE COURSE, SHALL BE SUBMITTED TO THE ENGINEER FOR VERIFICATION AND REQUIRED BY THE MUNICIPALITY WHERE DIRECTED. THE CONTRACTOR SHALL OBTAIN SPECIMENS OF THE BITUMINOUS CONCRETE WITH A CORE DRILL TO CONFIRM THE PLAN THICKNESS. DEFICIENCIES IN THICKNESS SHALL BE ADJUSTED BY THE METHOD DESCRIBED IN THE "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION." 7. IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO PROVIDE PROPER BARRICADING, WARNING DEVICES AND TRAFFIC MANAGEMENT WITHIN THE AREA OF CONSTRUCTION. ALL SUCH DEVICES AND THEIR INSTALLATION SHALL CONFORM TO THE ILLINOIS "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS," LATEST EDITION. THIS SHALL BE IN ACCORDANCE WITH THE SUBDIVISION CONTROL, ORDINANCE OF THE MUNICIPALITY. 8. LONGITUDINAL JOINT CONSTRUCTION A. AS MANY LONGITUDINAL JOINTS AS PRACTICAL SHALL BE CLOSED AT THE END OF EACH DAY OF PAVING. A TACK COAT SHALL BE APPLIED TO THE "COLD" SIDE OF THE LONGITUDINAL JOINT PRIOR TO THE PLACEMENT OF THE HOT SIDE MAT. B. LONGITUDINAL JOINT CONSTRUCTION SHALL BE COMPLETED BEFORE THE "COLD" SIDE OF THE JOINT FALLS BELOW 200°F. C. IN THE EVENT THE TEMPERATURE OF THE "COLD" SIDE OF A JOINT FALLS BELOW 200°F PRIOR TO JOINT CONSTRUCTION COMPLETION, THE CONTRACTOR SHALL PERFORM THE FOLLOWING: 1. HEAT THE COLD SIDE JOINT TO 200°F EITHER BY MEANS OF A HAND TORCH OR AN INDEPENDENT HEATING DEVICE. 2. APPLY TACK COAT TO THE REHEATED JOINT PRIOR TO ASPHALT PLACEMENT. D. THE CONTRACTOR SHALL OFFSET SURFACE COURSE JOINTS FROM BINDER COURSE JOINTS, WHEREVER PRACTICABLE. 9. LONGITUDINAL JOINT DENSITY SPECIFICATIONS A. COMPLETED LONGITUDINAL JOINTS SHALL BE ASSESSED BASED ON SECTION 1030 OF THE STANDARD SPECIFICATIONS AND THE "HOT MIX ASPHALT - DENSITY TESTING OF LONGITUDINAL JOINTS" (BDE) AS FOLLOWS: LONGITUDINAL JOINT DENSITY TESTING SHALL BE PERFORMED AT EACH RANDOM DENSITY TEST LOCATION. LONGITUDINAL JOINT TESTING SHALL BE LOCATED AT A DISTANCE EQUAL TO THE LIFT THICKNESS OR A MINIMUM OF TWO INCHES FROM EACH PAVEMENT EDGE, I.E. FOR A FOUR INCH LIFT, THE NEAR EDGE OF THE DENSITY GAUGE OR CORE BARREL SHALL BE WITHIN FOUR INCHES FROM THE EDGE OF PAVEMENT. LONGITUDINAL JOINT DENSITY TESTING SHALL BE PERFORMED USING EITHER A CORE OR A TACK COATED "WHEEL" DENSITY GAUGE OR CORES. 1. CONFIRMED EDGE: EACH CONFIRMED EDGE DENSITY TEST SHALL BE REPRESENTED BY A ONE MINUTE AVERAGE DENSITY READING OR A CORE DENSITY. THE TESTING SHALL BE INCLUDED IN THE AVERAGE OF DENSITY READINGS OR CORE DENSITIES TAKEN ACROSS THE MAT, WHICH REPRESENTS THE INDIVIDUAL TEST. 2. UNCONFIRMED EDGE: EACH UNCONFIRMED EDGE JOINT DENSITY TEST SHALL BE REPRESENTED BY AN AVERAGE OF ONE MINUTE DENSITY ONE MINUTE READING OR A SINGLE CORE AT THE GIVEN DENSITY TEST LOCATION. THE TESTING SHALL MEET THE DENSITY REQUIREMENTS SPECIFIED HEREIN. THE THREE, ONE MINUTE READINGS SHALL BE SPACED TEN FEET APART LONGITUDINALLY ALONG THE UNCONFIRMED PAVEMENT EDGE AND CENTERED AT THE RANDOM DENSITY TEST LOCATION. <table><thead><tr><th colspan="4">DENSITY CONTROL LIMITS TABLE</th></tr><tr><th>MIXTURE COMPOSITION</th><th>PARAMETER</th><th>INDIVIDUAL TEST (INCLUDES CONFIRMED EDGES)</th><th>UNCONFIRMED EDGE JOINT DENSITY MINIMUM</th></tr></thead><tbody><tr><td rowspan="3">IL-9.5, IL12.5</td><td>Ndesign >= 90</td><td>92.0-96.0%</td><td>90%</td></tr><tr><td>IL-9.5, IL9.5L, IL-12.5</td><td>Ndesign < 90</td><td>92.5-97.4%</td><td>90%</td></tr><tr><td>IL-19.0, IL-25.0</td><td>Ndesign >= 90</td><td>93.0-96.0%</td><td>90%</td></tr><tr><td rowspan="3">IL-19.0, IL-19.0L, IL-25.0</td><td>Ndesign >= 90</td><td>93.0-97.4%</td><td>91%</td></tr><tr><td>SMA</td><td>Ndesign = 50 & 80</td><td>93.5-97.4%</td><td>91%</td></tr><tr><td>ALL OTHER</td><td>Ndesign = 90</td><td>93.0-97.4%</td><td>90%</td></tr></tbody></table>		DOWEL SIZE AND SPACING AT CONSTRUCTION JOINTS			PAVEMENT DEPTH (INCH)	CONSTRUCTION JOINT DOWEL DIMENSIONS (INCH)	DOWEL SPACING C-C (INCH)	5 to <6	3/4 x 10	12	6 to <8	1 x 13	12	8 to <10	1-1/4 x 15	12	10 to 12	1-1/2 x 18	12	DENSITY CONTROL LIMITS TABLE				MIXTURE COMPOSITION	PARAMETER	INDIVIDUAL TEST (INCLUDES CONFIRMED EDGES)	UNCONFIRMED EDGE JOINT DENSITY MINIMUM	IL-9.5, IL12.5	Ndesign >= 90	92.0-96.0%	90%	IL-9.5, IL9.5L, IL-12.5	Ndesign < 90	92.5-97.4%	90%	IL-19.0, IL-25.0	Ndesign >= 90	93.0-96.0%	90%	IL-19.0, IL-19.0L, IL-25.0	Ndesign >= 90	93.0-97.4%	91%	SMA	Ndesign = 50 & 80	93.5-97.4%	91%	ALL OTHER	Ndesign = 90	93.0-97.4%	90%	DEVELOPER: KARIS CRITICAL CONTACT: DAVID DIX 2150 GOODLETTE--FRANK ROAD, SUITE 700 NAPLES, FLORIDA 34102 P: (407) 616-0398 GENERAL CONTRACTOR: DEVELOPMENT SOLUTIONS, INC. CONTACT: MICHAEL LONG, VP, INDUSTRIAL SERVICES 121 N. JEFFERSON, 1ST FLOOR CHICAGO, IL 60661 P: (312) 629-2800 ARCHITECT: CARLSON DESIGN GROUP CONTACT: GUY C. TORO R.A. 5550 PEACHTREE PARKWAY, SUITE 125 PEACHTREE CORNERS, GA 30092 P: (770) 217-7129 F: (313) 234-0800 CIVIL ENGINEER: JACOB AND HEFNER ASSOCIATES, INC. CONTACT: KRISTEN BRUNS, P.E. 1333 BUTTERFIELD ROAD, SUITE 300 DOWNERS GROVE, ILLINOIS 60515 P: (630) 652-4600 F: (630) 652-4601 CITY OF NAPERVILLE: CONTACT: ERIN VENARD 402 S. AGLE STREET NAPERVILLE, ILLINOIS 60540 P: (630) 420-4101	
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1960 WEST LUCENT LANE NAPERVILLE, ILLINOIS		KARIS CRITICAL		JACOB & HEFNER ASSOCIATES		1333 Butterfield Rd, Suite 300, Downers Grove, IL 60515 PHONE: (630) 652-4600, FAX: (630) 652-4601 www.jacobandhefner.com		H477a N.T.S. C2.0																																																			

1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH ALL APPLICABLE PROVISIONS OF THE SUBDIVISION CONTROL ORDINANCE OF THE MUNICIPALITY AND THE ILLINOIS URBAN MANUAL.

2. BEFORE STARTING SITE CLEARING AND GRADING WORK, A CONSTRUCTION ENTRANCE AND SILT FENCE SHALL BE INSTALLED AS SHOWN ON THE PLANS.

3. THE CONSTRUCTION ENTRANCE TO THE SITE SHALL BE STABILIZED WITH GRAVEL PRIOR TO BEGINNING ANY WORK ON THE SITE. THE ENTRANCE SHALL BE MAINTAINED PERIODICALLY FOR ITS EFFECTIVENESS TO REMOVE DIRT FROM CONSTRUCTION VEHICLES LEAVING THE SITE.

4. SILT FILTER FENCE SHALL BE PLACED AS SHOWN ON THE PLANS AND AS DIRECTED BY THE MUNICIPALITY'S ENGINEERING INSPECTOR, TO PREVENT SEDIMENT FROM LEAVING THE SITE.

5. STAKED SILT FENCE SHALL BE INSTALLED AND MAINTAINED AROUND THE INLETS AND CATCH BASINS AS SHOWN ON THE PLANS.

6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER INSTALLATION AND MAINTENANCE OF ALL TEMPORARY AND PERMANENT EROSION CONTROL MEASURES.

7. THE CONTRACTOR SHALL INSPECT EROSION CONTROL MEASURES WEEKLY AND AFTER ANY STORM EVENT IN EXCESS OF HALF AN INCH. AN INSPECTION REPORT SHALL BE COMPLETED EACH TIME AND KEPT WITHIN A BINDER ON THE JOB SITE AT ALL TIMES. THE NOI, NPDES PERMIT AND SWPP PLAN SHALL ALSO BE KEPT ON-SITE.

8. AT THE COMPLETION OF THE PROJECT, ALL STORM SEWER PIPES AND STRUCTURES SHALL BE CLEANED AND FREE OF DIRT AND DEBRIS. SEDIMENTATION SHALL BE REMOVED FROM THE STORM SEWER SYSTEM AND SHALL NOT BE WASHED OUT INTO THE EXISTING STORM SEWER SYSTEM.

9. THE TEMPORARY EROSION CONTROL MEASURES SHALL BE EFFECTIVELY IN PLACE UNTIL ALL THE PERMANENT EROSION CONTROL MEASURES ARE FULLY FUNCTIONAL.

10. THE GUARANTEE PERIOD SHALL BEGIN AFTER ALL THE PERMANENT EROSION CONTROL MEASURES ARE FULLY FUNCTIONAL AND ACCEPTABLE TO OWNER OR HIS REPRESENTATIVE.

11. STOCKPILES OF ANY KIND SHALL NOT BE PLACED IN SPECIAL MANAGEMENT AREAS. IF A STOCKPILE IS TO REMAIN IN PLACE FOR MORE THAN SEVEN DAYS, THEN SEDIMENT AND EROSION CONTROL MEASURES SHALL BE PROVIDED FOR THE STOCKPILE.

12. IF THE VOLUME, VELOCITY, SEDIMENT LOAD OR PEAK FLOW RATES OF STORM WATER RUNOFF ARE TEMPORARILY INCREASED DURING CONSTRUCTION, THEN PROPERTIES AND SPECIAL MANAGEMENT AREAS DOWNSTREAM FROM SUCH DEVELOPMENT SITES SHALL BE PROTECTED FROM EROSION AND SEDIMENTATION.

13. STORM SEWER INLETS SHALL BE PROTECTED WITH SEDIMENT TRAPPING OR FILTER CONTROL DEVICES DURING CONSTRUCTION.

14. THE SURFACE OF STRIPPED AREAS SHALL BE PERMANENTLY OR TEMPORARILY PROTECTED FROM SOIL EROSION WITHIN SEVEN DAYS AFTER FINAL GRADE IS REACHED. UNTIL A HALF INCH OR GREATER RAINFALL EVENT IS FORECAST PRIOR TO SEVEN DAYS, IN THIS OCCURRENCE, THE PROPER SOIL PROTECTION SHALL BE INSTALLED IMMEDIATELY. STRIPPED AREAS NOT AT FINAL GRADE THAT REMAIN UNDISTURBED FOR MORE THAN SEVEN DAYS AFTER INITIAL DISTURBANCE, SHALL BE PROTECTED FROM EROSION. UNLESS GRADING ACTIVITIES ARE RESUMED WITHIN 14 DAYS FROM WHEN ACTIVITIES CEASED, TEMPORARY COVER SHALL BE MAINTAINED CONTINUOUSLY UNTIL PERMANENT COVER IS ESTABLISHED.

1. WATER PUMPED OR OTHERWISE DISCHARGED FROM THE SITE DURING CONSTRUCTION DENEATURING, IRRIGATION OR FIRE HYDRANT FLUSHING, SHALL BE FILTERED PRIOR TO LEAVING THE PROJECT SITE.

2. IF NECESSARY, GRAVELED ROADS, ACCESS DRIVES, PARKING AREAS OF SUFFICIENT WIDTH AND LENGTH AND VEHICLE WASHDOWN FACILITIES, SHALL BE PROVIDED TO PREVENT THE DEPOSIT AND TRACKING OF SOIL ONTO PUBLIC OR PRIVATE ROADWAYS. ANY SOIL REACHING A PUBLIC OR PRIVATE ROADWAY SHALL BE REMOVED CONTINUOUSLY.

3. ALL CONTRACTORS SHALL COMPLY WITH SWPP PLAN AND NPDES REQUIREMENTS. A SIGNED SWPP PLAN SHALL BE KEPT ON FILE WITH OWNER OR GENERAL CONTRACTOR.

1. **FINE GRADING**

A. PRIOR TO THE CONSTRUCTION OF CURB AND GUTTER AND PLACEMENT OF THE BASE MATERIAL, THE STREETS SHALL BE FINE GRADED TO WITHIN 0.05 FEET OF FINAL SUBGRADE ELEVATION, TO A POINT TWO FEET BEYOND THE BACK OF CURB.

2. **CURB AND GUTTER**

A. THE TYPE OF THE CURB AND GUTTER SHALL BE AS DETAILED ON THE ENGINEERING PLANS.

B. THE CURBS SHALL BE BACKFILLED AFTER CONSTRUCTION AND PRIOR TO THE PLACEMENT OF THE BASE COURSE.

C. THE STONE COST UNDER THE CURB AND GUTTER SHALL BE CONSIDERED INCIDENTAL.

D. DEPRESSED CURBS FOR DRIVEWAYS AND HANDICAPPED RAMPS SHALL BE INSTALLED PER THE PLANS AND IDOT STANDARDS.

3. **CONCRETE PAVEMENT**

A. CONSTRUCTION JOINTS SHALL INCLUDE DOWELS PER "CONSTRUCTION JOINT TABLE" ALL DOWELS SHALL BE SMOOTH EPOXY COATED, GREASE, ONE END.

B. CONTRACTOR SHALL PROVIDE CONTRACTION JOINT 5' OFF OF FACE OF BUILDING.

C. EXCEPT AT FIRST PANEL FROM THE FACE OF BUILDING, CONCRETE PANEL LENGTH SHALL NOT EXCEED ITS WIDTH BY MORE THAN 25%.

D. CONTRACTION JOINTS SHALL BE CUT TO A DEPTH D/4 FOR CONVENTIONAL CONCRETE, OR D/3 FOR FIBER REINFORCED CONCRETE. WIDTH OF SAW-CUT SHALL BE PER JOINT SEALANT MANUFACTURER'S RECOMMENDATIONS.

E. ALL JOINTS SHALL BE CALKED

-SIKASIL-728SL (SAWCUT JOINTS)

-SIKAFLEX-1C/SL (EXPANSION JOINTS)

-OR APPROVED EQUAL.

F. CURB & GUTTER WITHIN CONCRETE PAVEMENT AREAS SHALL BE MONOLITHICALLY POURED OR SHALL BE TIED TO THE FIRST PANEL.

DOWEL SIZE AND SPACING AT CONSTRUCTION JOINTS			
PAVEMENT DEPTH (INCH)	CONSTRUCTION JOINT DOWEL DIMENSIONS (INCH)	DOWEL SPACING C-C (INCH)	
5 TO <6	3/4 x 10	12	
6 TO <8	1 x 13	12	
8 TO <10	1-1/4 x 15	12	
10 TO 12	1-1/2 x 18	12	

3. **PAVEMENT**

A. THE PAVEMENT MATERIALS SHALL BE AS DETAILED ON THE ENGINEERING PLANS. DEPTHS SPECIFIED SHALL BE CONSIDERED THE MINIMUM COMPACTED THICKNESS.

4. **GENERAL**

A. THE PAVING CONTRACTOR SHALL:

A. REPAIR ANY BASE COURSE AND BINDER COURSE FAILURES PRIOR TO THE INSTALLATION OF THE FINAL BITUMINOUS CONCRETE SURFACE COURSE.

B. SWEEP CLEAN THE BINDER COURSE PRIOR TO THE INSTALLATION OF THE FINAL BITUMINOUS CONCRETE SURFACE COURSE. ANY DAMAGE TO THE BINDER COURSE SHALL BE REPAIRED BY THE CONTRACTORS AT NO ADDITIONAL COST TO THE OWNER.

C. PROVIDE CONSTRUCTION, EXPANSION AND CONTRACTION JOINTS FOR CURB AND GUTTER AND P.C.C. SIDEWALK PER IDOT STANDARDS AND MUNICIPAL STANDARDS.

D. REMOVE ALL EXCESS MATERIALS AND DEBRIS. DISPOSE OF MATERIALS OFF-SITE AT NO ADDITIONAL COST TO THE OWNER.

5. **TESTING AND FINAL ACCEPTANCE**

A. PRIOR TO THE PLACEMENT OF THE BASE COURSE, THE SUBGRADE MUST BE PROOF ROLLED AND INSPECTED FOR UNSUITABLE LOCATIONS. IF UNSUITABLE SUBGRADE IS ENCOUNTERED, IT SHALL BE REMOVED AND REPLACED WITH GRANULAR MATERIAL APPROVED BY THE OWNER OR HIS REPRESENTATIVE. THE SUBGRADE SHALL HAVE A MINIMUM IIR VALUE OF 3.0.

B. PRIOR TO PLACEMENT OF THE BITUMINOUS CONCRETE SURFACE COURSE, THE CONTRACTOR SHALL OBTAIN SPECIMENS OF THE BINDER COURSE WITH A CORE DRILL WHERE DIRECTED AND AS REQUIRED. THE THICKNESS VERIFICATION BEZEL BE IN ACCORDANCE WITH THE "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION."

C. FINAL ACCEPTANCE OF THE TOTAL PAVEMENT INSTALLATION SHALL BE SUBJECT TO THE TESTING AND VERIFICATION REQUIREMENTS CITED ABOVE.

6. **METHOD OF MEASUREMENT**

A. CURB AND GUTTER AND BASE COURSE SHALL BE MEASURED IN THE FIELD BY THE CONTRACTOR. THE QUANTITIES SHALL BE SUBMITTED TO THE ENGINEER FOR VERIFICATION WHEN REQUESTED BY THE OWNER.

B. WHEN REQUESTED BY THE OWNER, DOCUMENTATION FOR THE INSTALLED BASE COURSE, BITUMINOUS CONCRETE BINDER AND SURFACE COURSE, SHALL BE SUBMITTED TO THE ENGINEER FOR VERIFICATION AS REQUIRED BY THE MUNICIPALITY. WHERE DIRECTED, THE CONTRACTOR SHALL OBTAIN SPECIMENS OF THE BITUMINOUS CONCRETE WITH A CORE DRILL TO CONFIRM THE PLAN THICKNESS. DEFICIENCIES IN THICKNESS SHALL BE ADJUSTED BY THE METHOD DESCRIBED IN THE "STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION."

7. IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO PROVIDE PROPER BARRICADING, WARNING DEVICES AND SAFE TRAFFIC MANAGEMENT WITHIN THE AREA OF CONSTRUCTION. ALL SUCH DEVICES AND THEIR INSTALLATION SHALL CONFORM TO THE ILLINOIS "MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES FOR STREETS AND HIGHWAYS," LATEST EDITION. THIS SHALL BE IN ACCORDANCE WITH THE SUBDIVISION CONTROL ORDINANCE OF THE MUNICIPALITY.

8. **LONGITUDINAL JOINT CONSTRUCTION**

A. AS MANY LONGITUDINAL JOINTS AS PRACTICAL SHALL BE CLOSED AT THE END OF EACH DAY OF PAVING. A JACK COAT SHALL BE APPLIED TO THE "COLD" SIDE OF THE LONGITUDINAL JOINT PRIOR TO THE PLACEMENT OF THE HOT SIDE MAT.

B. LONGITUDINAL JOINT CONSTRUCTION SHALL BE COMPLETED BEFORE THE "COLD" SIDE OF THE JOINT FALLS BELOW 200°F.

C. IN THE EVENT THE TEMPERATURE OF THE "COLD" SIDE OF A JOINT FALLS BELOW 200°F PRIOR TO JOINT CONSTRUCTION COMPLETION, THE CONTRACTOR SHALL PERFORM THE FOLLOWING:

1. HEAT THE COLD SIDE JOINT TO 200°F EITHER BY MEANS OF A HAND TORCH OR AN INCREASED HEATER. THE CONTRACTOR SHALL AVOID BURNING THE ASPHALT DURING HEATING.

2. APPLY TACK COAT TO THE REHEATED JOINT PRIOR TO ASPHALT PLACEMENT.

D. THE CONTRACTOR SHALL OFFSET SURFACE COURSE JOINTS FROM BINDER COURSE JOINTS, WHEREVER PRACTICABLE.

9. **LONGITUDINAL JOINT DENSITY SPECIFICATIONS**

A. COMPLETED LONGITUDINAL JOINTS SHALL BE ASSESSED BASED ON SECTION 1030 OF THE STANDARD SPECIFICATIONS AND THE "HOT MIX ASPHALT - DENSITY TESTING OF LONGITUDINAL JOINTS" (BDE) AS FOLLOWS:

LONGITUDINAL JOINT DENSITY TESTING SHALL BE PERFORMED AT EACH RANDOM DENSITY TEST LOCATION. LONGITUDINAL JOINT TESTING SHALL BE LOCATED AT A DISTANCE EQUAL TO THE LIFT THICKNESS OR A MINIMUM OF TWO INCHES FROM EACH PAVEMENT EDGE. IF, FOR A FOUR INCH LIFT, THE NEAR EDGE OF THE DENSITY GAUGE OR CORE BARREL SHALL BE WITHIN FOUR INCHES FROM THE EDGE OF PAVEMENT, LONGITUDINAL JOINT DENSITY TESTING SHALL BE PERFORMED USING EITHER A CORRELATED NUCLEAR GAUGE OR CORES.

1. CONFIRMED EDGE: EACH UNCONFIRMED EDGE DENSITY TEST SHALL BE REPRESENTED BY A ONE MINUTE NUCLEAR DENSITY READING OR A CORE DENSITY. THE TESTING SHALL BE INCLUDED IN THE AVERAGE OF DENSITY READINGS OR CORE DENSITIES TAKEN ACROSS THE MAT, WHICH REPRESENTS THE INDIVIDUAL TEST.

2. UNCONFIRMED EDGE: EACH UNCONFIRMED EDGE JOINT DENSITY TEST SHALL BE REPRESENTED BY AN AVERAGE OF TWO ONE MINUTE NUCLEAR DENSITY READINGS OR A SINGLE CORE DENSITY AT THE GIVEN DENSITY TEST LOCATION. THE TESTING SHALL MEET THE DENSITY REQUIREMENTS SPECIFIED HEREIN. THE THREE, ONE MINUTE READINGS SHALL BE SPACED TEN FEET APART LONGITUDINALLY ALONG THE UNCONFIRMED PAVEMENT EDGE AND CENTERED AT THE RANDOM DENSITY TEST LOCATION.

1) THE STORM DRAIN SHALL BE CONSTRUCTED OF "OT" RING JOINTS AND SANITARY SEWER SHALL BE CONSTRUCTED WITH PIPE EQUIVALENT TO WATER MAIN STANDARDS.

2) THE WATER MAIN MAY BE ENCASED IN A WATERLOUT CARRIER PIPE AS INDICATED ON THE PLANS AND AS PER THE SPECIAL CROSSING DETAIL SHOWN ON THE DETAIL SHEET.

3) A VERTICAL SEPARATION OF 18 INCHES BETWEEN THE INVERT OF THE SEWER OR DRAIN AND THE CROWN OF THE WATER MAIN SHALL BE MAINTAINED WHERE A WATER MAIN CROSSES UNDER SEWER. SUPPORT THE SEWER OR DRAIN LINES TO PREVENT SETTLING AND BREAKING OF THE WATER MAIN.

4) CONSTRUCTION SHALL EXTEND ON EACH SIDE OF THE CROSSING UNTIL THE NORMAL DISTANCE FROM THE WATER MAIN TO THE SEWER OR DRAIN LINE IS AT LEAST 10 FEET.

1. **WATER MAINS**

1. HORIZONTAL SEPARATION:

a) WATER MAINS SHALL BE LAID AT LEAST 10 FEET HORIZONTALLY FROM ANY EXISTING OR PROPOSED DRAIN, STORM SEWER, SANITARY SEWER OR SEWER SERVICES CONNECTION.

b) WATER MAINS MAY BE LAID CLOSER THAN 10 FEET TO A SEWER LINE WHEN:

1) LOCAL CONDITIONS PREVENT A LATERAL SEPARATION OF 10 FEET

2) THE WATER MAIN INVERT IS AT LEAST 18 INCHES ABOVE THE CROWN OF THE SEWER

3) THE WATER MAIN IS IN A SEPARATE TRENCH.

2) BOTH THE WATER MAIN AND DRAIN OR SEWER SHALL BE CONSTRUCTED OF PIPE EQUIVALENT TO WATER MAIN STANDARDS OF CONSTRUCTION WHEN IT IS IMPOSSIBLE TO MEET CONDITION (a) OR (b) ABOVE. THE DRAIN OR SEWER SHALL BE PRESSURE TESTED TO THE MAXIMUM EXPECTED SURCHARGE HEAD BEFORE BACKFILLING.

2. VERTICAL SEPARATION:

a) A WATER MAIN SHALL BE LAID SO THAT ITS INVERT IS 18 INCHES ABOVE THE CROWN OF THE DRAIN OR SEWER, WHENEVER WATER MAINS CROSS STORM SEWERS, SANITARY SEWERS OR SEWER SERVICES CONNECTIONS. THE VERTICAL SEPARATION SHALL BE MAINTAINED FOR THE PORTION OF THE WATER MAIN LOCATED WITHIN 10 FEET HORIZONTALLY OF ANY SEWER OR DRAIN CROSSED. A LENGTH OF WATER MAIN PIPE SHALL BE CENTERED OVER THE SEWER TO BE CROSSED WITH JOINTS EQUIDISTANT FROM THE SEWER OR DRAIN.

b) IF IT IS IMPOSSIBLE TO OBTAIN THE PROPER VERTICAL SEPARATION AS DESCRIBED IN (a) OR (b) ABOVE, THE WATER MAIN SHALL PASS UNDER A SEWER OR DRAIN, THEN ONE OF THE FOLLOWING METHODS SHALL BE FOLLOWED:

1) THE STORM DRAIN SHALL BE CONSTRUCTED OF "OT" RING JOINTS AND SANITARY SEWER SHALL BE CONSTRUCTED WITH PIPE EQUIVALENT TO WATER MAIN STANDARDS.

2) THE WATER MAIN MAY BE ENCASED IN A WATERLOUT CARRIER PIPE AS INDICATED ON THE PLANS AND AS PER THE SPECIAL CROSSING DETAIL SHOWN ON THE DETAIL SHEET.

c) A VERTICAL SEPARATION OF 18 INCHES BETWEEN THE INVERT OF THE SEWER OR DRAIN AND THE CROWN OF THE WATER MAIN SHALL BE MAINTAINED WHERE A WATER MAIN CROSSES UNDER SEWER. SUPPORT THE SEWER OR DRAIN LINES TO PREVENT SETTLING AND BREAKING OF THE WATER MAIN.

d) CONSTRUCTION SHALL EXTEND ON EACH SIDE OF THE CROSSING UNTIL THE NORMAL DISTANCE FROM THE WATER MAIN TO THE SEWER OR DRAIN LINE IS AT LEAST 10 FEET.

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CHICAGO, IL 60661
P: (312) 629-2800

ARCHITECT:
CARLSON DESIGN GROUP
CONTACT: GUY C. TORO R.A.
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PEACHTREE CORNERS, GA 30092
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F: (313) 234-0800

CIVIL ENGINEER:
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CONTACT: KRISTEN BRUNS, P.E.
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F: (630) 652-4601

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NAPERVILLE, ILLINOIS 60540
P: (630) 420-4101

4 REVISED PER CITY COMMENTS 09/26/25

3 REVISED PER CITY COMMENTS 07/31/25

2 REVISED PER CITY COMMENTS 6/12/25

1 ORIGINAL PLAN DATE 4/7/25

No. Description

Date

GENERAL NOTES AND SPECIFICATIONS

1960 WEST LUCENT LANE

NAPERVILLE, ILLINOIS

KARIS CRITICAL

JACOB & HEFNER ASSOCIATES

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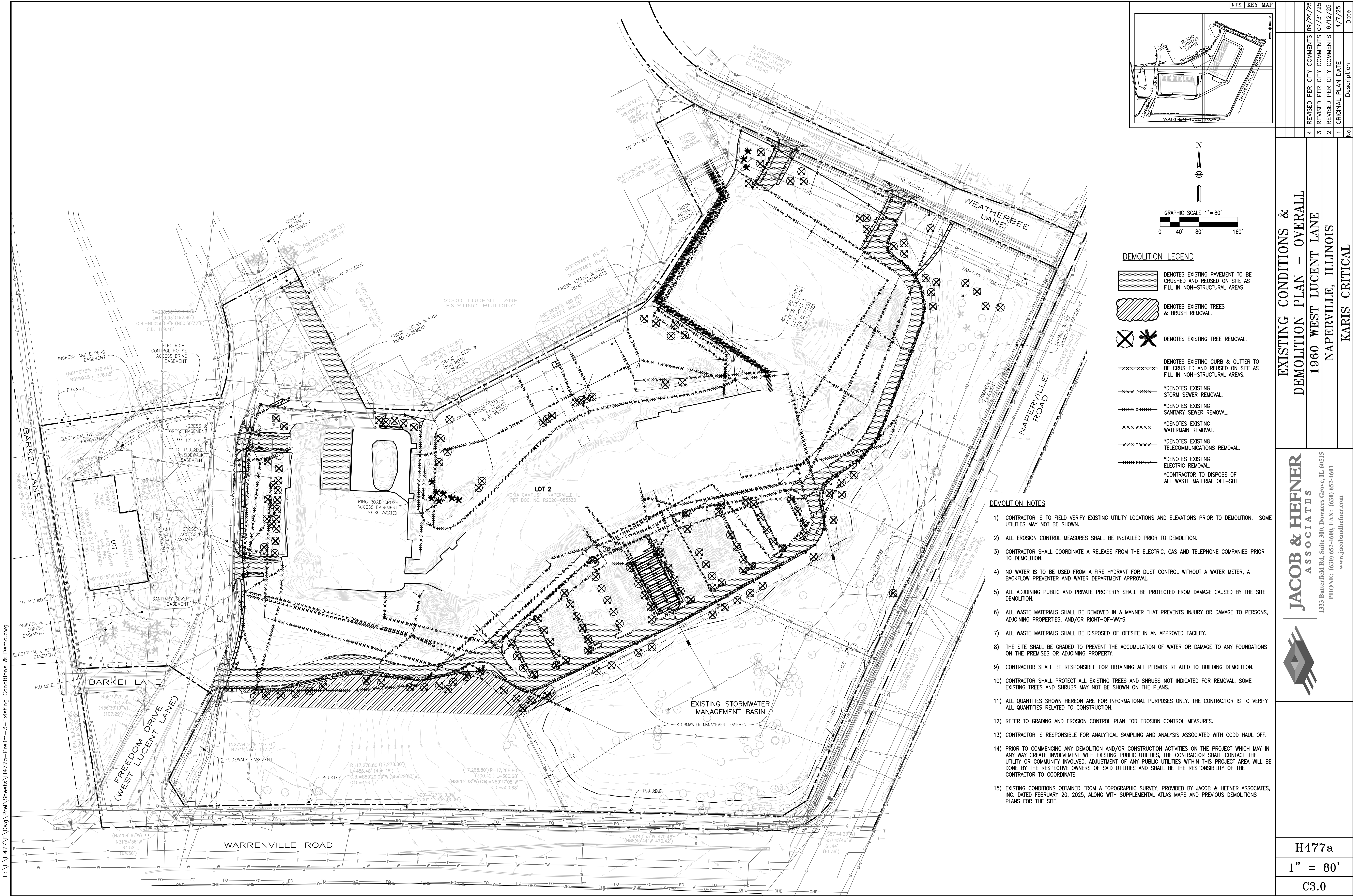
N.T.S.

C2.0

NOTE: IF SPECIFICATIONS AND/OR DETAILS PROVIDED IN THESE PLANS CONFLICT WITH THE CITY OF NAPERVILLE SPECIFICATIONS AND/OR DETAILS, THE CITY OF NAPERVILLE SHALL SUPERCEDE.

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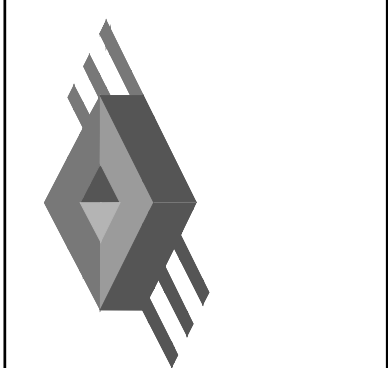
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EXISTING CONDITIONS & DEMOLITION PLAN - OVERALL		1960 WEST LUCENT LANE		NAPERVILLE, ILLINOIS		KARIS CRITICAL	
No.	Description	Original Plan Date	Revised Per City Comments	Revised Per City Comments	Revised Per City Comments	Date	Description
1	ORIGINAL PLAN DATE	4/7/25					
2	REVISD PER CITY COMMENTS	6/12/25					
3	REVISD PER CITY COMMENTS	07/31/25					
4	REVISD PER CITY COMMENTS	09/26/25					

JACOB & HEFNER ASSOCIATES

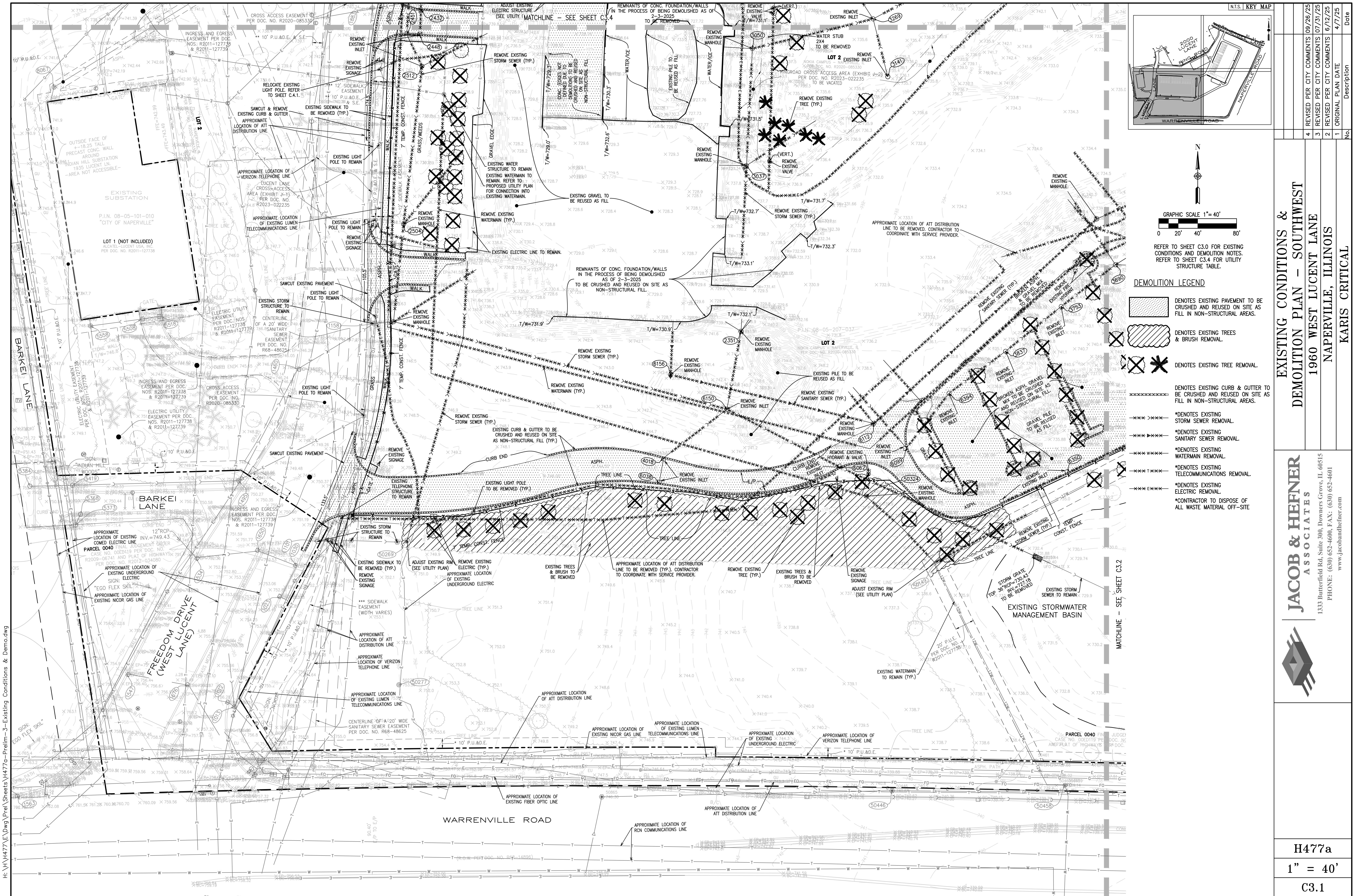
1333 Butterfield Rd, Suite 300, Downers Grove, IL 60515
PHONE: (630) 652-4600, FAX: (630) 652-4601
www.jacobandhefner.com

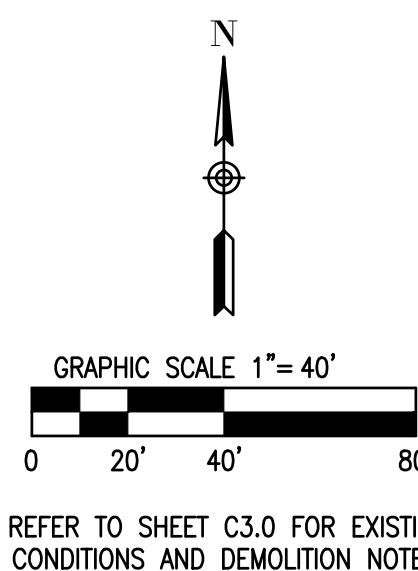
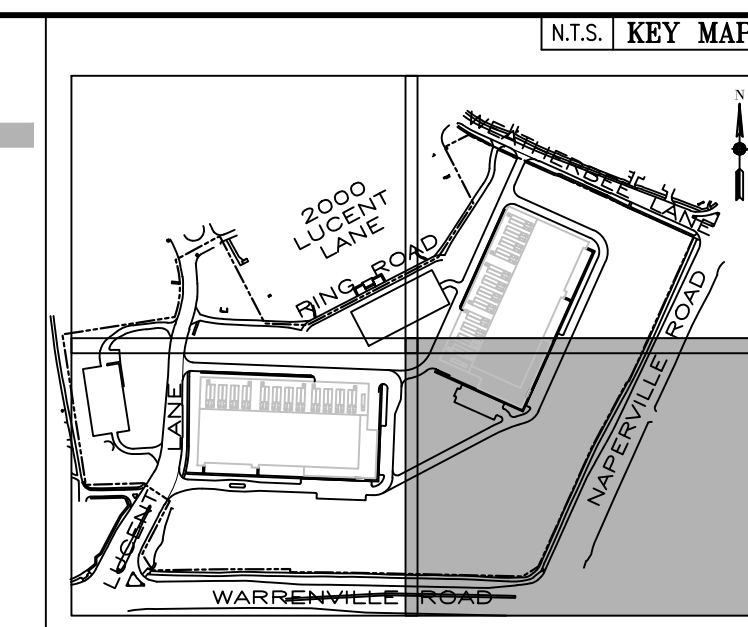
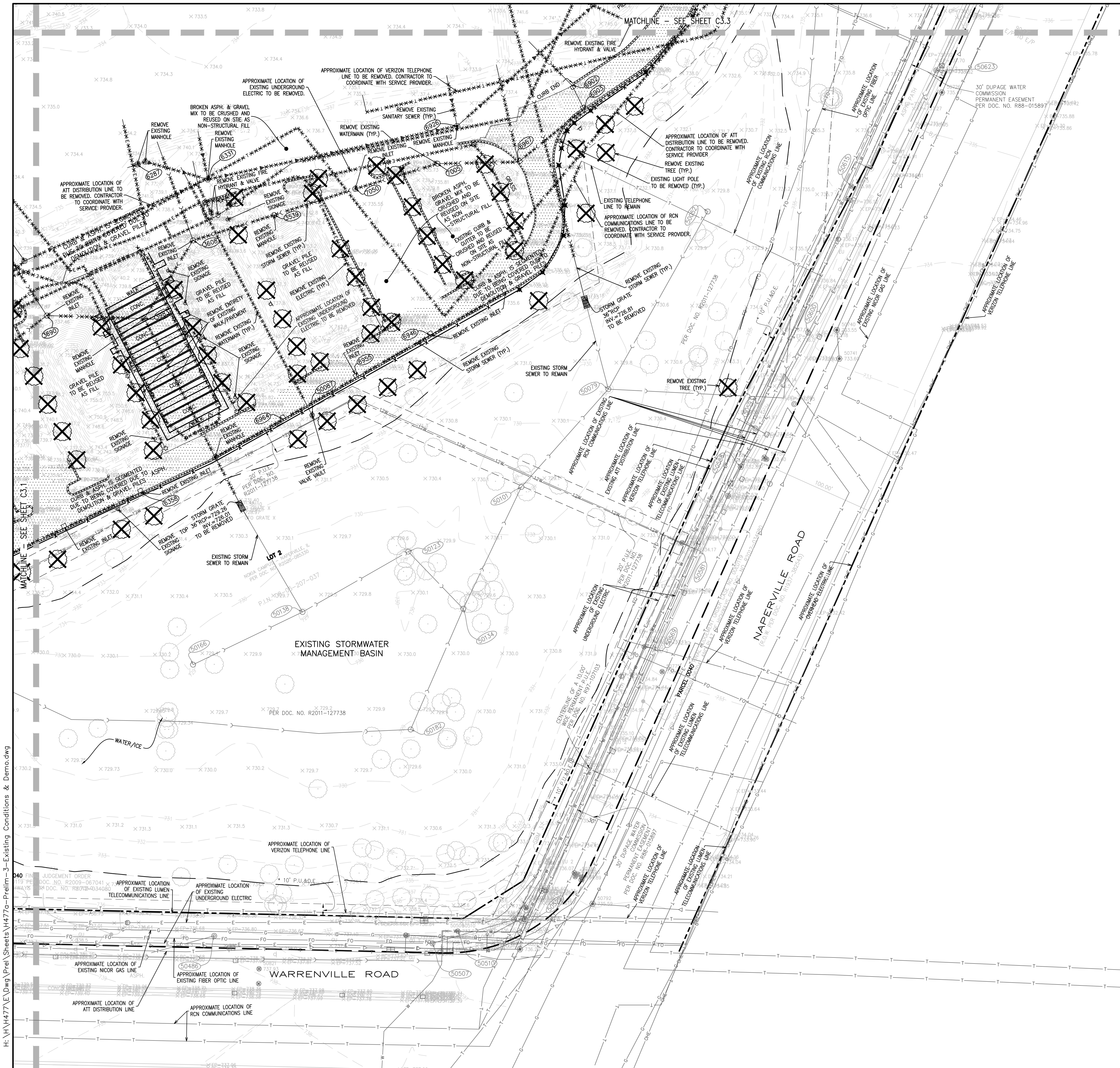


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1" = 80'

C3.0





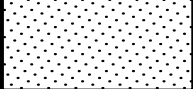
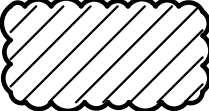
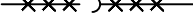
GRAPHIC SCALE 1" = 40'



0 20' 40' 80'

REFER TO SHEET C3.0 FOR EXISTING CONDITIONS AND DEMOLITION NOTATION

DEMOLITION LEGEND

- | | |
|---|---|
|  | <p>DENOTES EXISTING PAVEMENT TO BE CRUSHED AND REUSED ON SITE AS FILL IN NON-STRUCTURAL AREAS.</p> |
|  | <p>DENOTES EXISTING TREES & BRUSH REMOVAL.</p> |
|   | <p>DENOTES EXISTING TREE REMOVAL.</p> |
|  | <p>DENOTES EXISTING CURB & GUTTER TO BE CRUSHED AND REUSED ON SITE AS FILL IN NON-STRUCTURAL AREAS.</p> |
|  | <p>*DENOTES EXISTING STORM SEWER REMOVAL.</p> |
|  | <p>*DENOTES EXISTING SANITARY SEWER REMOVAL.</p> |
|  | <p>*DENOTES EXISTING WATERMAIN REMOVAL.</p> |
|  | <p>*DENOTES EXISTING TELECOMMUNICATIONS REMOVAL.</p> |
|  | <p>*DENOTES EXISTING ELECTRIC REMOVAL.</p> |
| | <p>*CONTRACTOR TO DISPOSE OF ALL WASTE MATERIAL OFF-SITE</p> |

WATER STRUCTURE TABLE - SHEET C3.2			
STRUCTURE#	TYPE	RIM ELEVATION	INVERT
50510	WATER VALVE VAULT	737.04	TOP OF WATER = 728.49 WATER FILLED-NO ACCESSIBLE PIPES
50569	WATER VALVE VAULT	734.22	TOP +18" DIP SW-NE=727.37
50615	WATER VALVE VAULT	735.98	TOP 18" DIP E-W=730.88
50581	WATER VALVE VAULT	734.32	TOP +18" DIP N-S=728.32
50087	WATER VALVE VAULT	736.24	TOP DIP NW-SE=727.94 WATER LINE=730.04

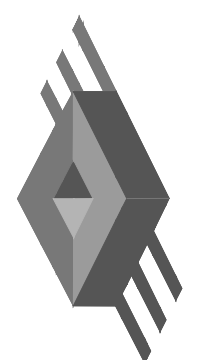
STRUCTURE#	TYPE	RIM ELEVATION	INVERT
6331	SANITARY MANHOLE	737.12	LID FROZEN-NOT ACCESSIBLE
50588	SANITARY MANHOLE	732.85	I(E-W)12'DIP=724.70
6926	SANITARY MANHOLE	735.95	I(N)8'PVC=726.90 I(NW-SE)12'PVC=725.10

STORM STRUCTURE TABLE - SHEET C3.2			
STRUCTURE#	TYPE	RIM ELEVATION	INVERT
6358	STORM INLET	734.85	LID FROZEN--NOT ACCESSIBLE
6287	STORM MANHOLE	739.33	(W)6"VERT. PIPE=727.53 TOP OF 6" PIPE=732.18
50507	STORM MANHOLE	737.23	(E-W)36"RCP=728.93
50486	STORM MANHOLE	737.13	(SW)4+-36"RCP--OFFSET--NOT ACCESSIBLE NO OTHER ACCESSIBLE PIPES
50522	STORM MANHOLE	736.52	(SW-NE)36"RCP=728.17
50593	STORM MANHOLE	734.10	(E)18"RCP--OFFSET--NOT ACCESSIBLE (W)18"RCP=726.80 NO OTHER VISIBLE PIPES
50600	STORM MANHOLE	735.26	(E)18"RCP--OFFSET--NOT ACCESSIBLE (W)18"RCP=726.36
50623	STORM MANHOLE	737.28	(N,E)12"RCP=729.28 (S)18"RCP=729.28
7005	STORM INLET	735.11	SILT TRAP FULL--NOT ACCESSIBLE
7050	STORM INLET	735.14	(SW-NE)18"RCP=731.04
6967	STORM INLET	735.33	LID FROZEN--NOT ACCESSIBLE
6903	STORM INLET	737.36	LID FROZEN--NOT ACCESSIBLE
6964	STORM INLET	734.83	LID FROZEN--NOT ACCESSIBLE
6902	STORM INLET	737.23	LID FROZEN--NOT ACCESSIBLE
3539	STORM INLET	735.09	(E-W)18"RCP=730.44
3608	STORM INLET	737.31	(N)12"PVC=731.81 (E)18"RCP=729.41 (S)W24"RCP=729.41
3983	STORM MANHOLE	739.11	(N,NW)24"RCP=728.86 (S,E)36"RCP=728.86
3690	STORM INLET	737.41	SILT TRAP FROZEN IN STRUCTURE--NOT ACCESSIBLE
6955	STORM INLET	734.90	LID FROZEN--NOT ACCESSIBLE
6946	STORM INLET	734.79	LID FROZEN--NOT ACCESSIBLE
50166	STORM INLET--BEEHIVE	728.93	(N,E)6"PVC=726.48
50138	STORM INLET--BEEHIVE	728.89	(N,E)10"PVC=726.24 (S)W6"PVC=726.24
50123	STORM INLET--BEEHIVE	728.83	(S,E)5W10"PVC=725.35
50134	STORM INLET--BEEHIVE	728.94	(N,NW)10"PVC=+-725.64 (S)W10"PVC--NOT ACCESSIBLE
50182	STORM INLET--BEEHIVE	728.84	(N,E,W)10"CLAY=726.09
50101	STORM INLET--BEEHIVE	728.80	(N,E,S)W10"PVC=725.45
50079	STORM INLET--BEEHIVE	728.85	(N,E,S)W10"PVC=725.30
50052	STORM INLET--BEEHIVE	728.93	(S)W10"PVC=725.78

EXISTING CONDITIONS &
DEMOLITION PLAN - SOUTHEAST
1960 WEST LUCENT LANE
NAPERVILLE, ILLINOIS
KARIS CRITICAL

JACOB & HEFNER
ASSOCIATES

333 Butterfield Rd, Suite 300, Downers Grove, IL 60515
PHONE: (630) 652-6600, FAX: (630) 652-4601
www.jacobandhefner.com

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REFERENCES

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www.coffmanas.com

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333 Butt
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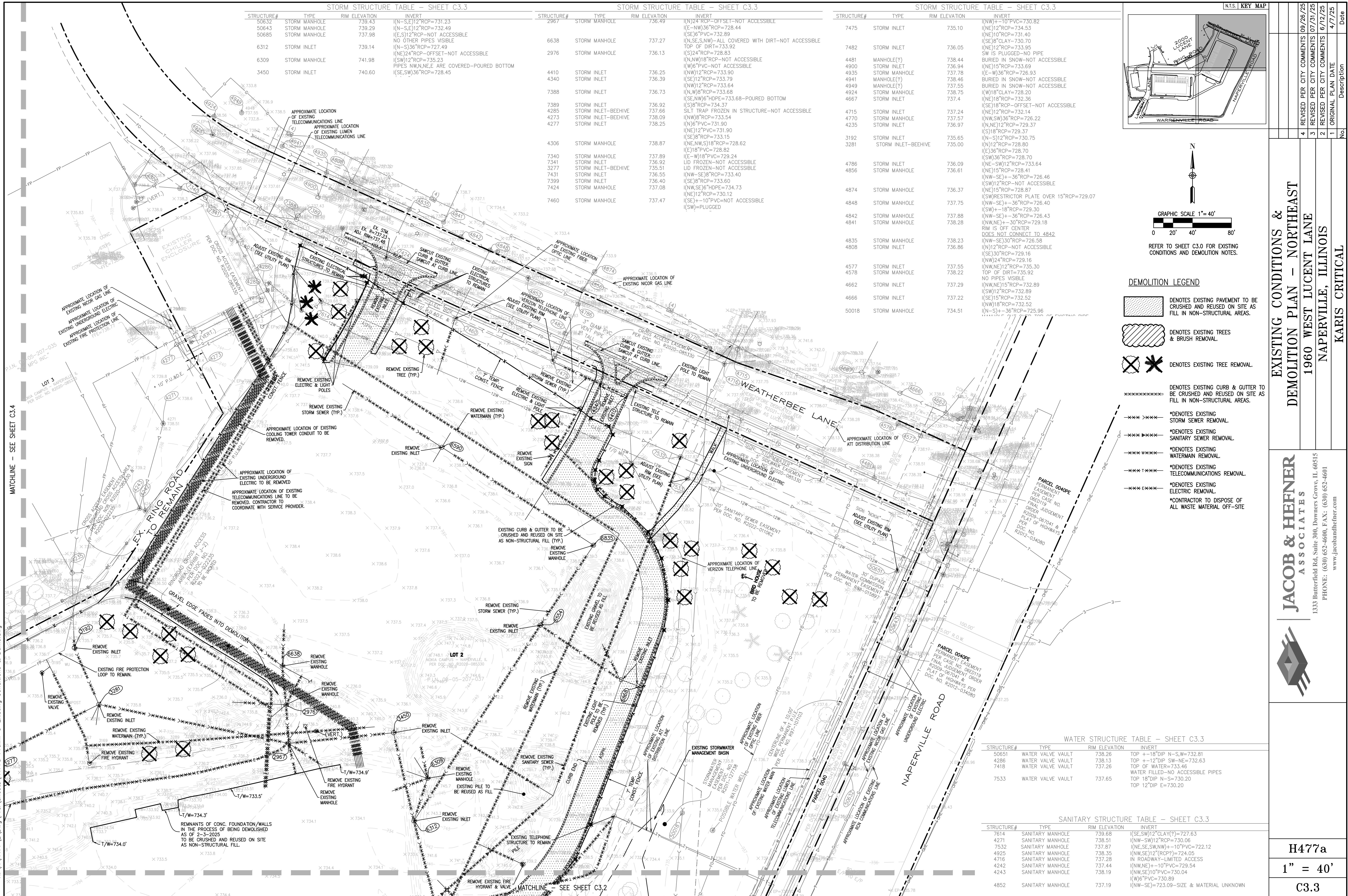
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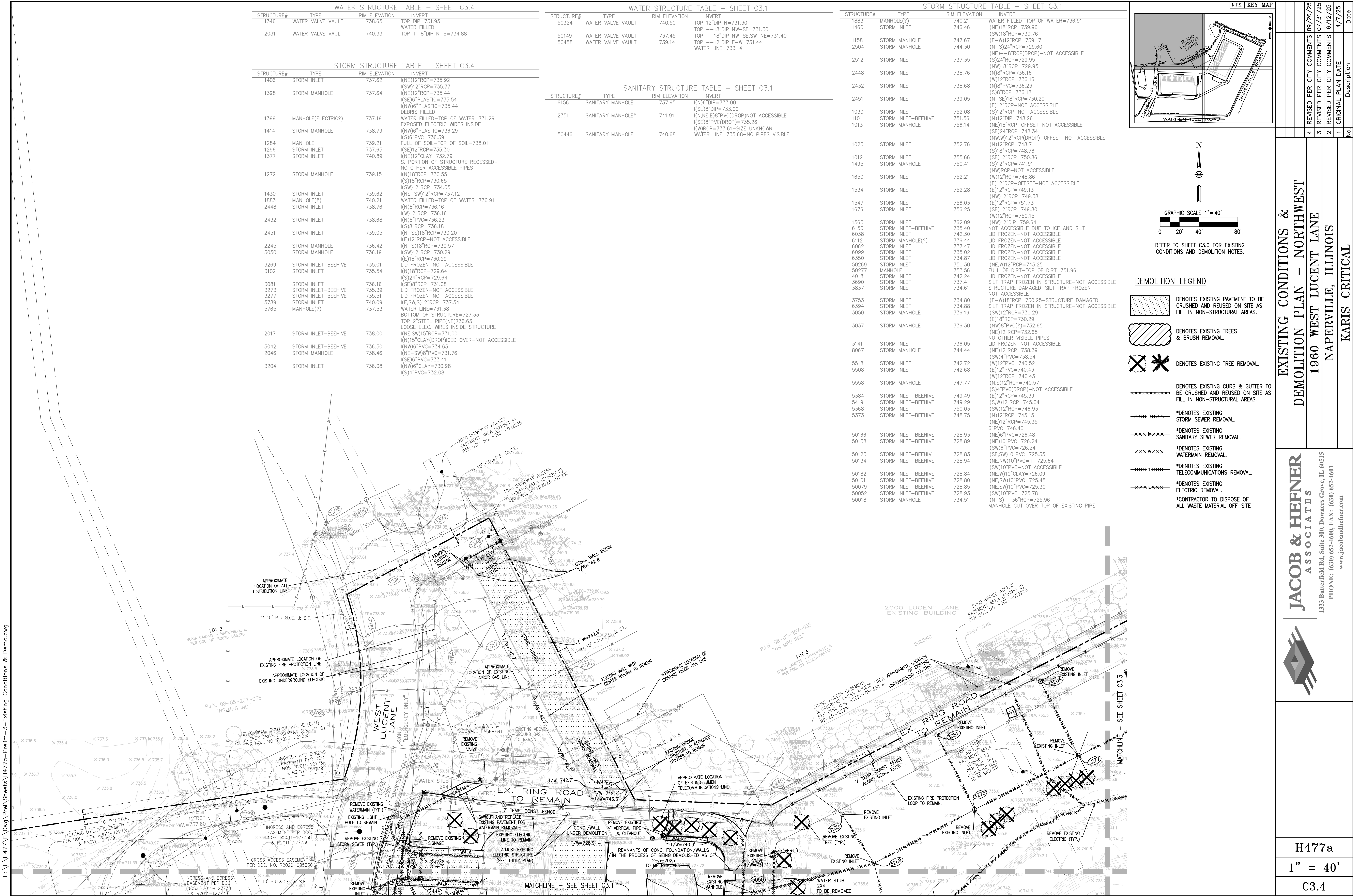
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C3.2

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STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1883

MANHOLE(?)

740.21

WATER FILLED-TOP OF WATER=736.91
(N)18"RCP=739.96
(S)18"RCP=739.76
(E-W)12"RCP=739.17
(N-S)24"RCP=729.60
(NE)4--8"RCP(DROP)-NOT ACCESSIBLE
(S)24"RCP=729.95
(NW)18"RCP=729.95
(N)8"RCP=736.16
(W)12"RCP=736.16
(N)8"PVC=736.23
(S)8"RCP=736.18
(N-SE)18"RCP=730.20
(E)12"RCP-NOT ACCESSIBLE
(S)12"RCP-NOT ACCESSIBLE
(N)12"DIP=748.26
(NE)18"RCP-OFFSET-NOT ACCESSIBLE
(SE)24"RCP=748.34
(NW,W)12"RCP(DROP)-OFFSET-NOT ACCESSIBLE
(N)12"RCP=748.71
(S)18"RCP=748.76
(SE)12"RCP=750.86
(S)12"RCP=741.91
(NW)RCP-NOT ACCESSIBLE
(W)12"RCP=748.86
(E)12"RCP-OFFSET-NOT ACCESSIBLE
(E)12"RCP=749.13
(NW)12"RCP=749.38
(E)12"RCP=751.73
(SE)12"RCP=749.80
(W)12"RCP=750.15
(NW)12"DIP=759.64
NOT ACCESSIBLE DUE TO ICE AND SILT
LID FROZEN-NOT ACCESSIBLE
LID FROZEN-NOT ACCESSIBLE
LID FROZEN-NOT ACCESSIBLE
LID FROZEN-NOT ACCESSIBLE
(NE,W)12"RCP=745.25
FULL OF DIRT-TOP OF DIRT=751.96
LID FROZEN-NOT ACCESSIBLE
SILT TRAP FROZEN IN STRUCTURE-NOT ACCESSIBLE
STRUCTURE DAMAGED-SILT TRAP FROZEN
NOT ACCESSIBLE
(E-W)18"RCP=730.25-STRUCTURE DAMAGED
SILT TRAP FROZEN IN STRUCTURE-NOT ACCESSIBLE
(S)W)12"RCP=730.29
(E)18"RCP=730.29
(NW)8"PVC(?)=732.65
(N)12"RCP=732.65
NO OTHER VISIBLE PIPES
LID FROZEN-NOT ACCESSIBLE
(NE)12"RCP=738.39
(S)W)4"PVC=738.54
(W)12"PVC=740.52
(E)12"PVC=740.43
(W)12"RCP=740.43
(N)12"RCP=740.57
(S)4"PVC(DROP)-NOT ACCESSIBLE
(E)12"RCP=745.39
(S,W)12"RCP=745.04
(SW)12"RCP=746.93
(N)12"RCP=745.15
(NE)12"RCP=745.35
6"PVC=746.40
(NE)6"PVC=726.48
(NE)10"PVC=726.24
(S)W)6"PVC=726.24
(E,SE,SW)10"PVC=725.35
(NE,NW)10"PVC=+-725.64
(SW)10"PVC-NOT ACCESSIBLE
(NE,W)10"CLAY=726.09
(NE,SW)10"PVC=725.45
INLET, SW)10"PVC=725.30
(SW)10"PVC=725.78
(N-S)4--16"RCP=725.96
MANHOLE CUT OVER TOP OF EXISTING PIPE

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2512

STORM INLET

737.35

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2448

STORM INLET

738.76

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2432

STORM INLET

739.05

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2451

STORM INLET

739.05

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1030

STORM INLET

752.08

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1101

STORM INLET-BEEHIVE

751.56

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1013

STORM MANHOLE

756.14

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1023

STORM INLET

752.76

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1012

STORM INLET

755.66

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1495

STORM MANHOLE

750.41

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1650

STORM INLET

752.21

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1534

STORM INLET

752.28

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1547

STORM INLET

756.03

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1676

STORM INLET

756.25

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1563

STORM INLET

762.09

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6150

STORM INLET-BEEHIVE

735.40

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6038

STORM INLET

742.30

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6112

STORM MANHOLE(?)

736.44

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6062

STORM INLET

737.47

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6099

STORM INLET

735.02

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6350

STORM INLET

734.87

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50269

STORM INLET

750.30

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50277

MANHOLE

753.56

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

4018

STORM INLET

742.24

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3690

STORM INLET

737.41

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3837

STORM INLET

734.61

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3753

STORM INLET

734.80

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6394

STORM INLET

734.88

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3050

STORM MANHOLE

736.19

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3037

STORM MANHOLE

736.30

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3141

STORM INLET

736.05

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

8067

STORM MANHOLE

744.44

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5518

STORM INLET

742.72

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5508

STORM INLET

742.68

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5558

STORM MANHOLE

747.77

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5384

STORM INLET-BEEHIVE

749.49

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5419

STORM INLET-BEEHIVE

749.29

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5368

STORM INLET

750.03

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5373

STORM INLET-BEEHIVE

748.75

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50166

STORM INLET-BEEHIVE

728.93

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50138

STORM INLET-BEEHIVE

728.89

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50123

STORM INLET-BEEHIVE

728.83

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50134

STORM INLET-BEEHIVE

728.94

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50182

STORM INLET-BEEHIVE

728.84

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50101

STORM INLET-BEEHIVE

728.80

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50079

STORM INLET-BEEHIVE

728.85

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50052

STORM INLET-BEEHIVE

728.93

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50018

STORM MANHOLE

734.51

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1883

MANHOLE(?)

740.21

WATER FILLED-TOP OF WATER=736.91
(N)18"RCP=739.96
(S)18"RCP=739.76
(E-W)12"RCP=739.17
(N-S)24"RCP=729.60
(NE)4--8"RCP(DROP)-NOT ACCESSIBLE
(S)24"RCP=729.95
(NW)18"RCP=729.95
(N)8"RCP=736.16
(W)12"RCP=736.16
(N)8"PVC=736.23
(S)8"RCP=736.18
(N-SE)18"RCP=730.20
(E)12"RCP-NOT ACCESSIBLE
(S)12"RCP-NOT ACCESSIBLE
(N)12"DIP=748.26
(NE)18"RCP-OFFSET-NOT ACCESSIBLE
(SE)24"RCP=748.34
(NW,W)12"RCP(DROP)-OFFSET-NOT ACCESSIBLE
(N)12"RCP=748.71
(S)18"RCP=748.76
(SE)12"RCP=750.86
(S)12"RCP=741.91
(NW)RCP-NOT ACCESSIBLE
(W)12"RCP=748.86
(E)12"RCP-OFFSET-NOT ACCESSIBLE
(E)12"RCP=749.13
(NW)12"RCP=749.38
(E)12"RCP=751.73
(SE)12"RCP=749.80
(W)12"RCP=750.15
(NW)12"DIP=759.64
NOT ACCESSIBLE DUE TO ICE AND SILT
LID FROZEN-NOT ACCESSIBLE
LID FROZEN-NOT ACCESSIBLE
LID FROZEN-NOT ACCESSIBLE
LID FROZEN-NOT ACCESSIBLE
(NE,W)12"RCP=745.25
FULL OF DIRT-TOP OF DIRT=751.96
LID FROZEN-NOT ACCESSIBLE
SILT TRAP FROZEN IN STRUCTURE-NOT ACCESSIBLE
STRUCTURE DAMAGED-SILT TRAP FROZEN
NOT ACCESSIBLE
(E-W)18"RCP=730.25-STRUCTURE DAMAGED
SILT TRAP FROZEN IN STRUCTURE-NOT ACCESSIBLE
(S)W)12"RCP=730.29
(E)18"RCP=730.29
(NW)8"PVC(?)=732.65
(N)12"RCP=732.65
NO OTHER VISIBLE PIPES
LID FROZEN-NOT ACCESSIBLE
(NE)12"RCP=738.39
(S)W)4"PVC=738.54
(W)12"PVC=740.52
(E)12"PVC=740.43
(W)12"RCP=740.43
(N)12"RCP=740.57
(S)4"PVC(DROP)-NOT ACCESSIBLE
(E)12"RCP=745.39
(S,W)12"RCP=745.04
(SW)12"RCP=746.93
(N)12"RCP=745.15
(NE)12"RCP=745.35
6"PVC=746.40
(NE)6"PVC=726.48
(NE)10"PVC=726.24
(S)W)6"PVC=726.24
(E,SE,SW)10"PVC=725.35
(NE,NW)10"PVC=+-725.64
(SW)10"PVC-NOT ACCESSIBLE
(NE,W)10"CLAY=726.09
(NE,SW)10"PVC=725.45
INLET, SW)10"PVC=725.30
(SW)10"PVC=725.78
(N-S)4--16"RCP=725.96
MANHOLE CUT OVER TOP OF EXISTING PIPE

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2512

STORM INLET

737.35

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2448

STORM INLET

738.76

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2432

STORM INLET

739.05

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

2451

STORM INLET

739.05

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1030

STORM INLET

752.08

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1101

STORM INLET-BEEHIVE

751.56

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1013

STORM MANHOLE

756.14

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1023

STORM INLET

752.76

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1012

STORM INLET

755.66

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1495

STORM MANHOLE

750.41

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1650

STORM INLET

752.21

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1534

STORM INLET

752.28

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1547

STORM INLET

756.03

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1676

STORM INLET

756.25

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

1563

STORM INLET

762.09

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6150

STORM INLET-BEEHIVE

735.40

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6038

STORM INLET

742.30

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6112

STORM MANHOLE(?)

736.44

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6062

STORM INLET

737.47

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6099

STORM INLET

735.02

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6350

STORM INLET

734.87

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50269

STORM INLET

750.30

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50277

MANHOLE

753.56

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

4018

STORM INLET

742.24

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3690

STORM INLET

737.41

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3837

STORM INLET

734.61

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3753

STORM INLET

734.80

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

6394

STORM INLET

734.88

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3050

STORM MANHOLE

736.19

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3037

STORM MANHOLE

736.30

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

3141

STORM INLET

736.05

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

8067

STORM MANHOLE

744.44

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5518

STORM INLET

742.72

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5508

STORM INLET

742.68

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5558

STORM MANHOLE

747.77

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5384

STORM INLET-BEEHIVE

749.49

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5419

STORM INLET-BEEHIVE

749.29

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5368

STORM INLET

750.03

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

5373

STORM INLET-BEEHIVE

748.75

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50166

STORM INLET-BEEHIVE

728.93

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50138

STORM INLET-BEEHIVE

728.89

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50123

STORM INLET-BEEHIVE

728.83

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50134

STORM INLET-BEEHIVE

728.94

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50182

STORM INLET-BEEHIVE

728.84

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50101

STORM INLET-BEEHIVE

728.80

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50079

STORM INLET-BEEHIVE

728.85

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50052

STORM INLET-BEEHIVE

728.93

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

50018

STORM MANHOLE

734.51

STRUCTURE#

TYPE

RIM ELEVATION

INVERT

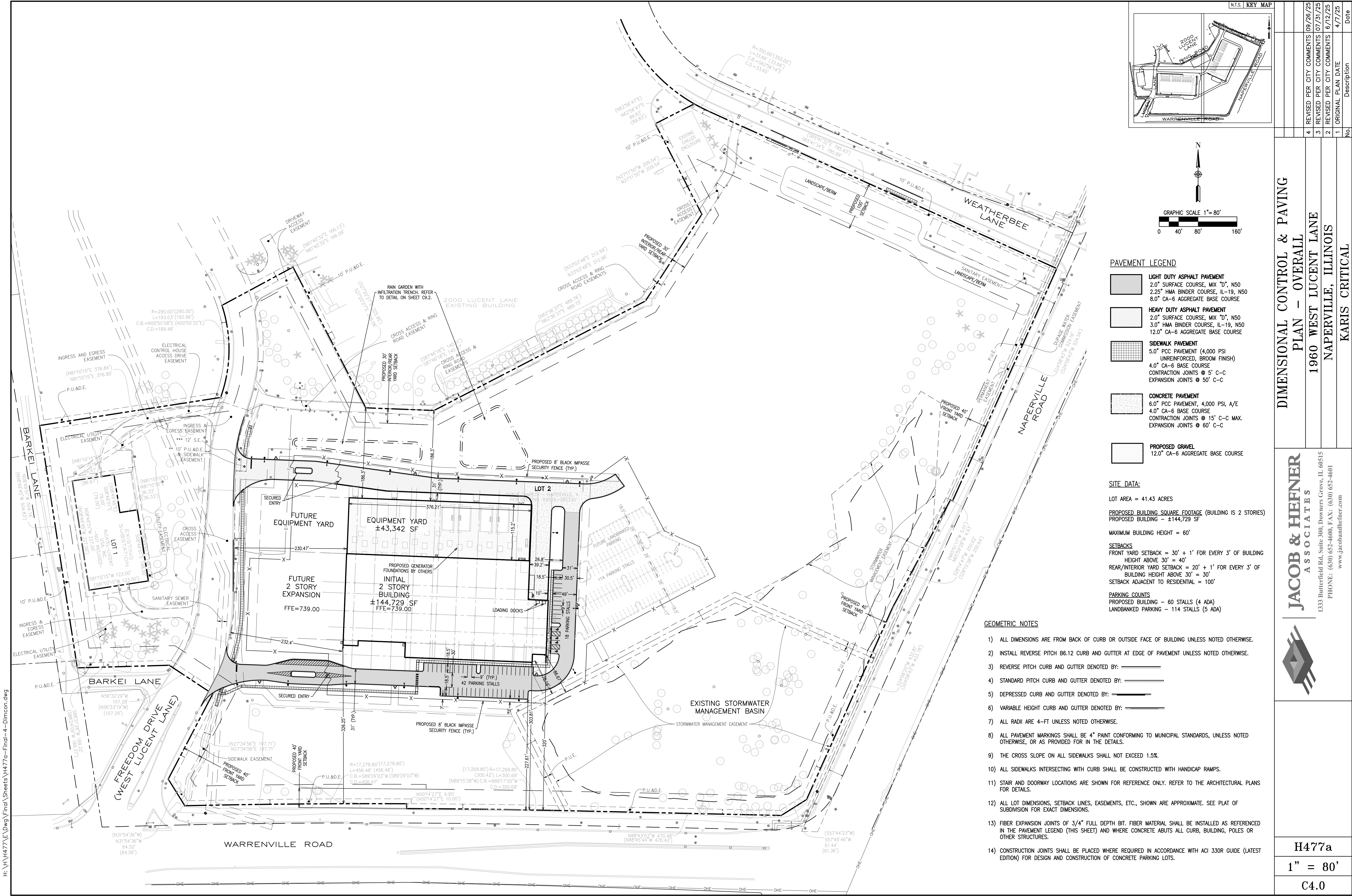
1883

MANHOLE(?)

740.21

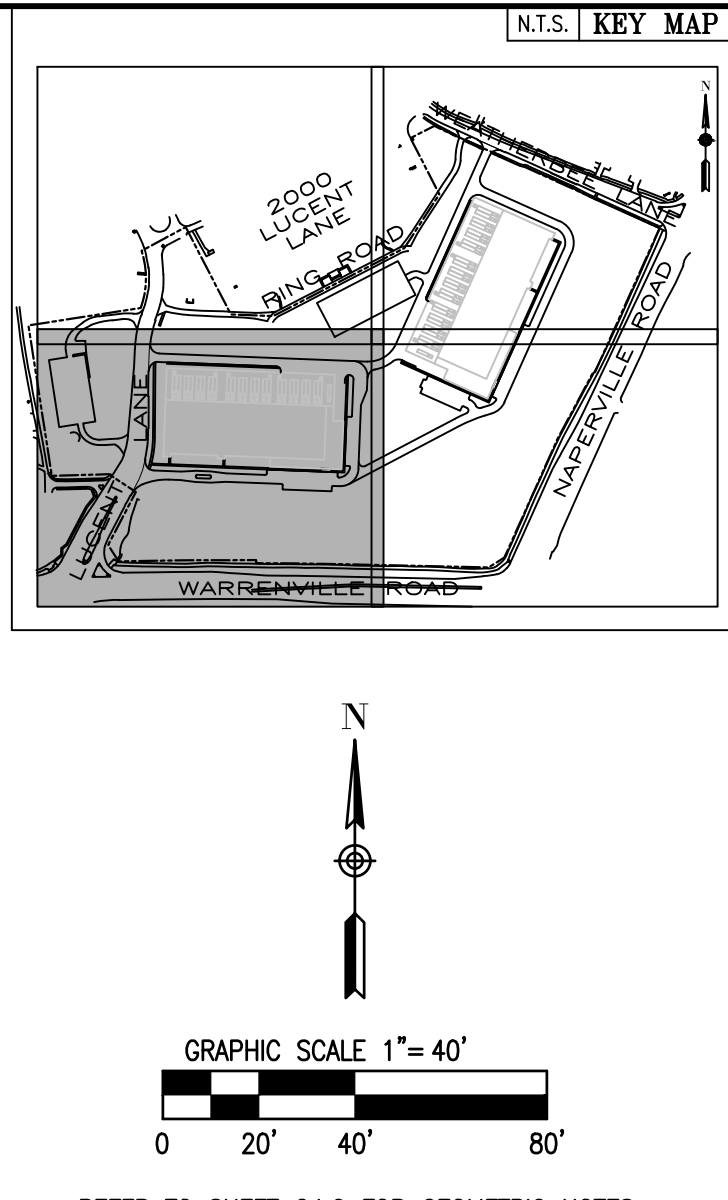
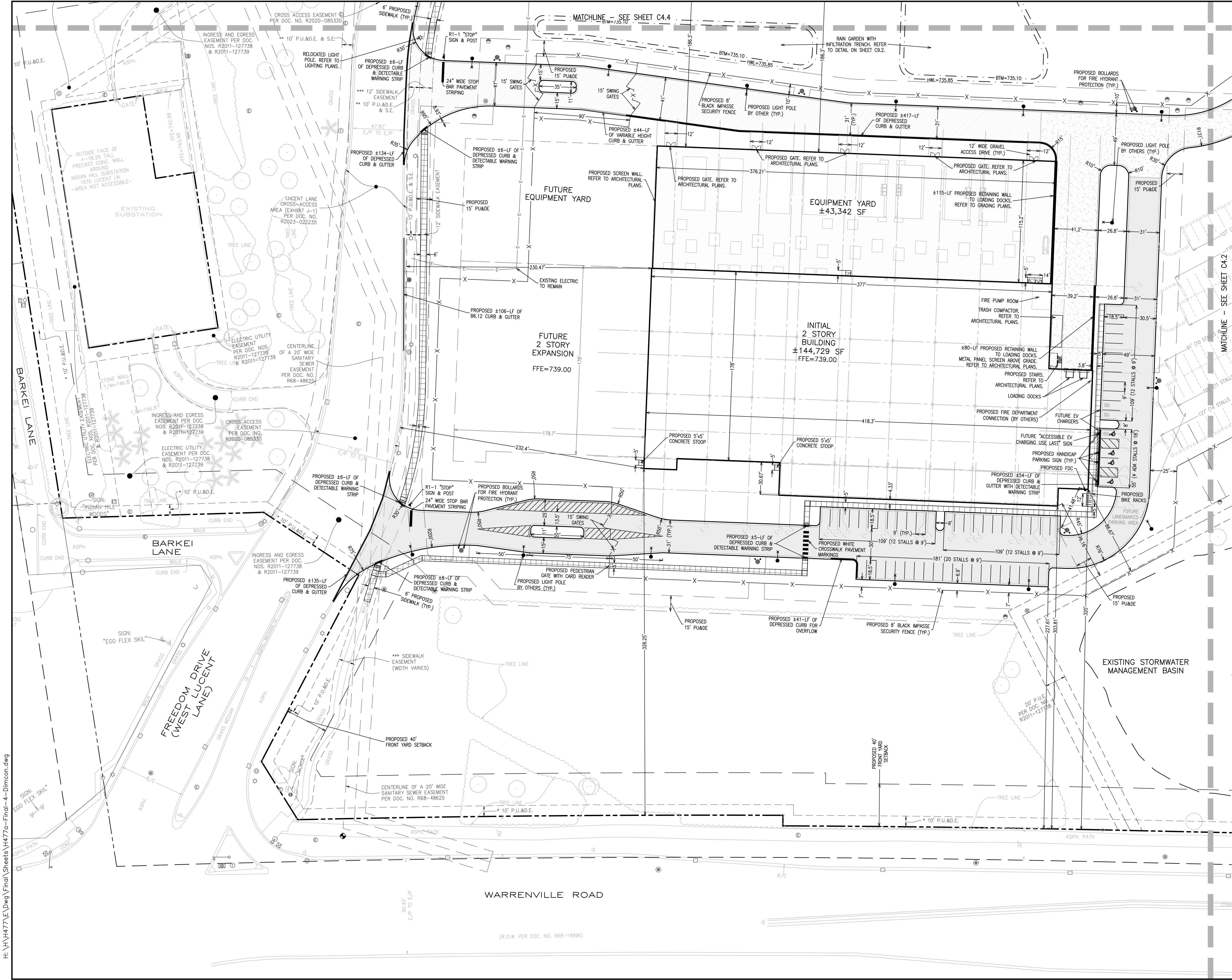
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(N)18"RCP=739.96
(S)18"RCP=739.76
(E-W)12"RCP=739.17
(N-S)24"RCP=729.60
(NE)4--8"RCP(DROP)-NOT ACCESSIBLE
(S)24"RCP=729.95
(NW)18"RCP=729.95
(N)8"RCP=736.16
(W)12"RCP=736.16
(N)8"PVC=736.23
(S)8"RCP=736.18
(N-SE)18"RCP=730.20
(E)12"RCP-NOT ACCESSIBLE
(S)12"RCP-NOT ACCESSIBLE
(N)12"DIP=748.26
(NE)18"RCP-OFFSET-NOT ACCESSIBLE
(SE)24"RCP=748.34
(NW,W)12"RCP(DROP)-OFFSET-NOT ACCESSIBLE
(N)12"RCP=748.71
(S)18"RCP=748.76
(SE)12"RCP=750.86
(S)12"RCP=741.91
(NW)RCP-NOT ACCESSIBLE
(W)12"RCP=748.86
(E)12"RCP-OFFSET-NOT ACCESSIBLE
(E)12"RCP=749.13
(NW)12"RCP=749.38
(E)12"RCP=751.73
(SE)12

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No.		Description	Date
4	REVISED PER CITY COMMENTS	09/26/25	
3	REVISED PER CITY COMMENTS	07/31/25	
2	REVISED PER CITY COMMENTS	6/12/25	
1	ORIGINAL PLAN DATE	4/7/25	
DIMENSIONAL CONTROL & PAVING PLAN - OVERALL		KARIS CRITICAL	
1960 WEST LUCENT LANE			
NAPERVILLE, ILLINOIS			
JACOB & HEFNER ASSOCIATES		1333 Butterfield Rd, Suite 300, Downers Grove, IL 60515	
		PHONE: (630) 652-4600, FAX: (630) 652-4601	
		www.jacobandhefner.com	
H477a			
1" = 80'			
C4.0			

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- REFER TO SHEET C4.0 FOR GEOMETRIC NOTES.
- PAVEMENT LEGEND**
- LIGHT DUTY ASPHALT PAVEMENT**
2.0" SURFACE COURSE, MIX "D", N50
2.25" HMA BINDER COURSE, IL-19, N50
8.0" CA-6 AGGREGATE BASE COURSE
 - HEAVY DUTY ASPHALT PAVEMENT**
2.0" SURFACE COURSE, MIX "D", N50
3.0" HMA BINDER COURSE, IL-19, N50
12.0" CA-6 AGGREGATE BASE COURSE
 - SIDEWALK PAVEMENT**
5.0" PCC PAVEMENT (4,000 PSI
UNREINFORCED, BROOM FINISH)
4.0" CA-6 BASE COURSE
CONTRACTION JOINTS @ 5' C-C
EXPANSION JOINTS @ 50' C-C
 - CONCRETE PAVEMENT**
6.0" PCC PAVEMENT, 4,000 PSI, A/E
4.0" CA-6 BASE COURSE
CONTRACTION JOINTS @ 15' C-C MAX.
EXPANSION JOINTS @ 60' C-C
 - PROPOSED GRAVEL**
12.0" CA-6 AGGREGATE BASE COURSE
- CURB & GUTTER LEGEND**
- REVERSE PITCH CURB & GUTTER
 - STANDARD PITCH CURB & GUTTER
 - DEPRESSED CURB & GUTTER
 - VARIABLE HEIGHT CURB & GUTTER

DIMENSIONAL CONTROL & PAVING PLAN - SOUTHWEST			
1960 WEST LUCENT LANE			
NAPERVILLE, ILLINOIS			
KARIS CRITICAL			
4	REVISED PER CITY COMMENTS	09/26/25	Date
3	REVISED PER CITY COMMENTS	07/31/25	
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No.	Description		

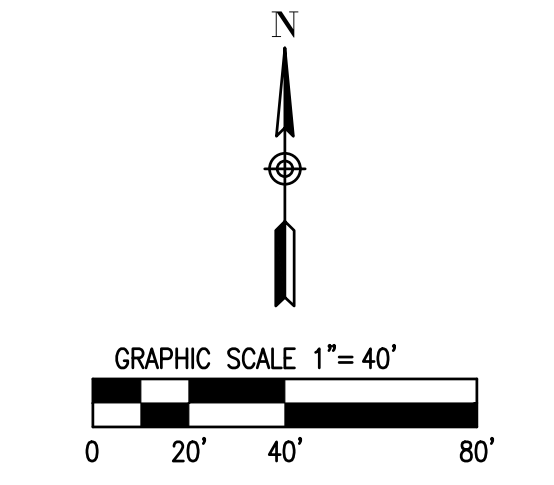
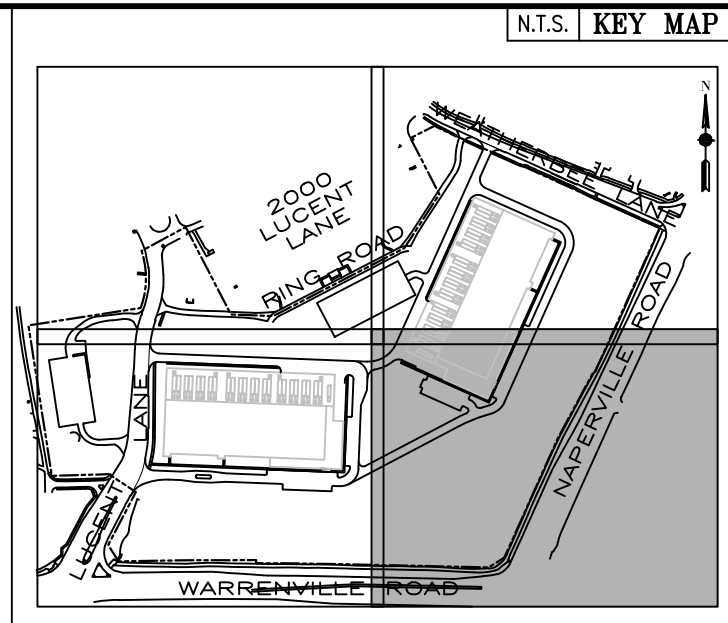
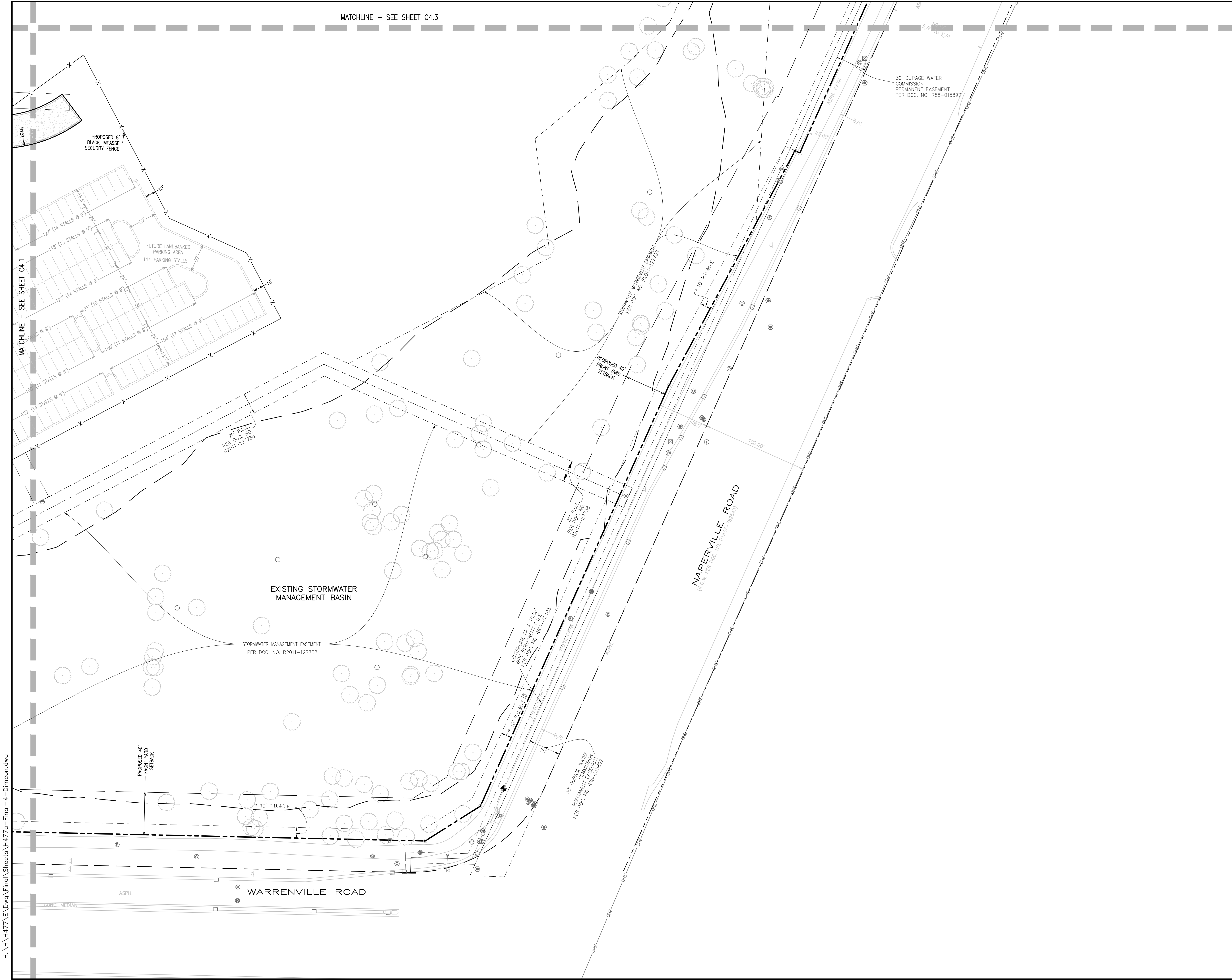
JACOB & HEFNER ASSOCIATES
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H477a

1" = 40'

C4.1

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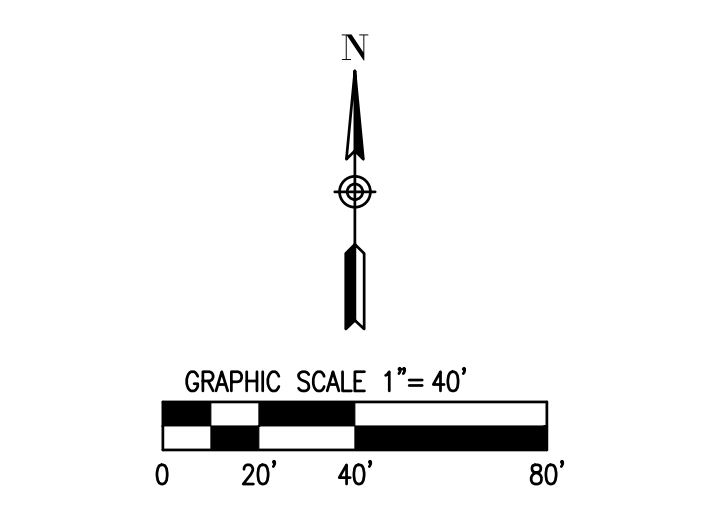
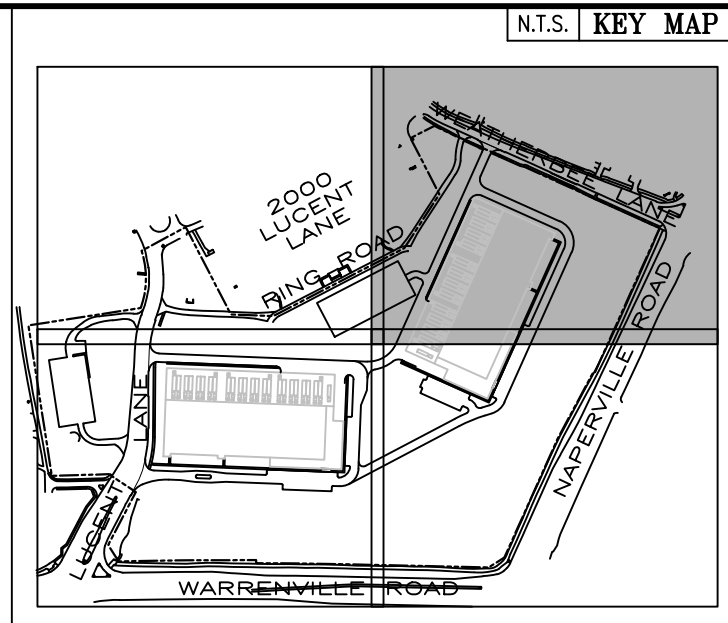
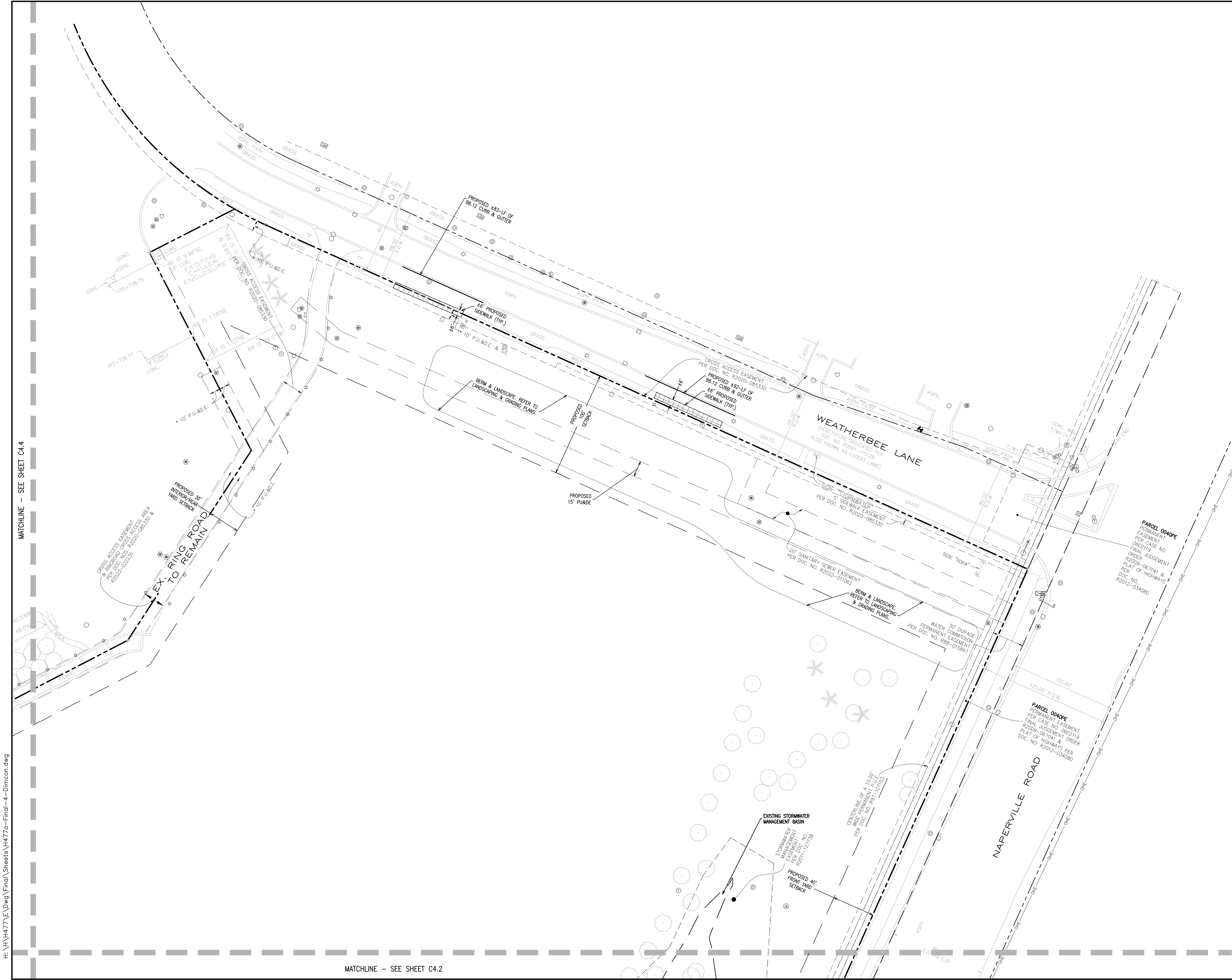
DIMENSIONAL CONTROL & PAVING PLAN - SOUTHEAST 1960 WEST LUCENT LANE NAPERVILLE, ILLINOIS KARIS CRITICAL		JACOB & HEFNER ASSOCIATES 1333 Butterfield Rd, Suite 300, Downers Grove, IL 60515 PHONE: (630) 652-4600, FAX: (630) 652-4601 www.jacobandhefner.com
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No.	Description	Date

H477a

1" = 40'

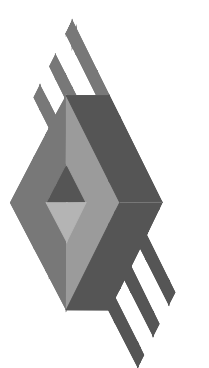
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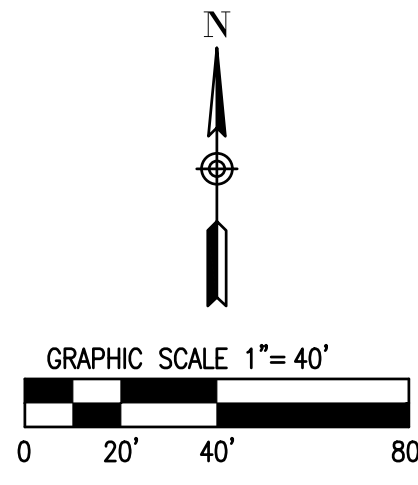
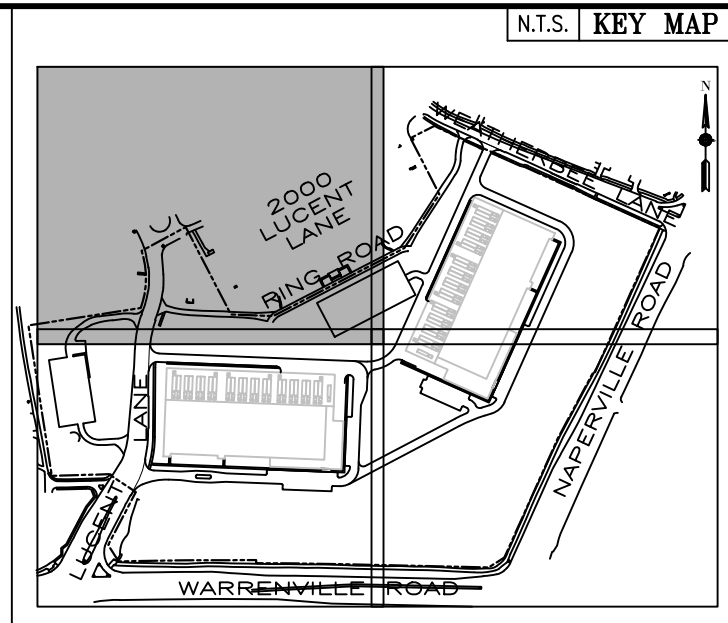
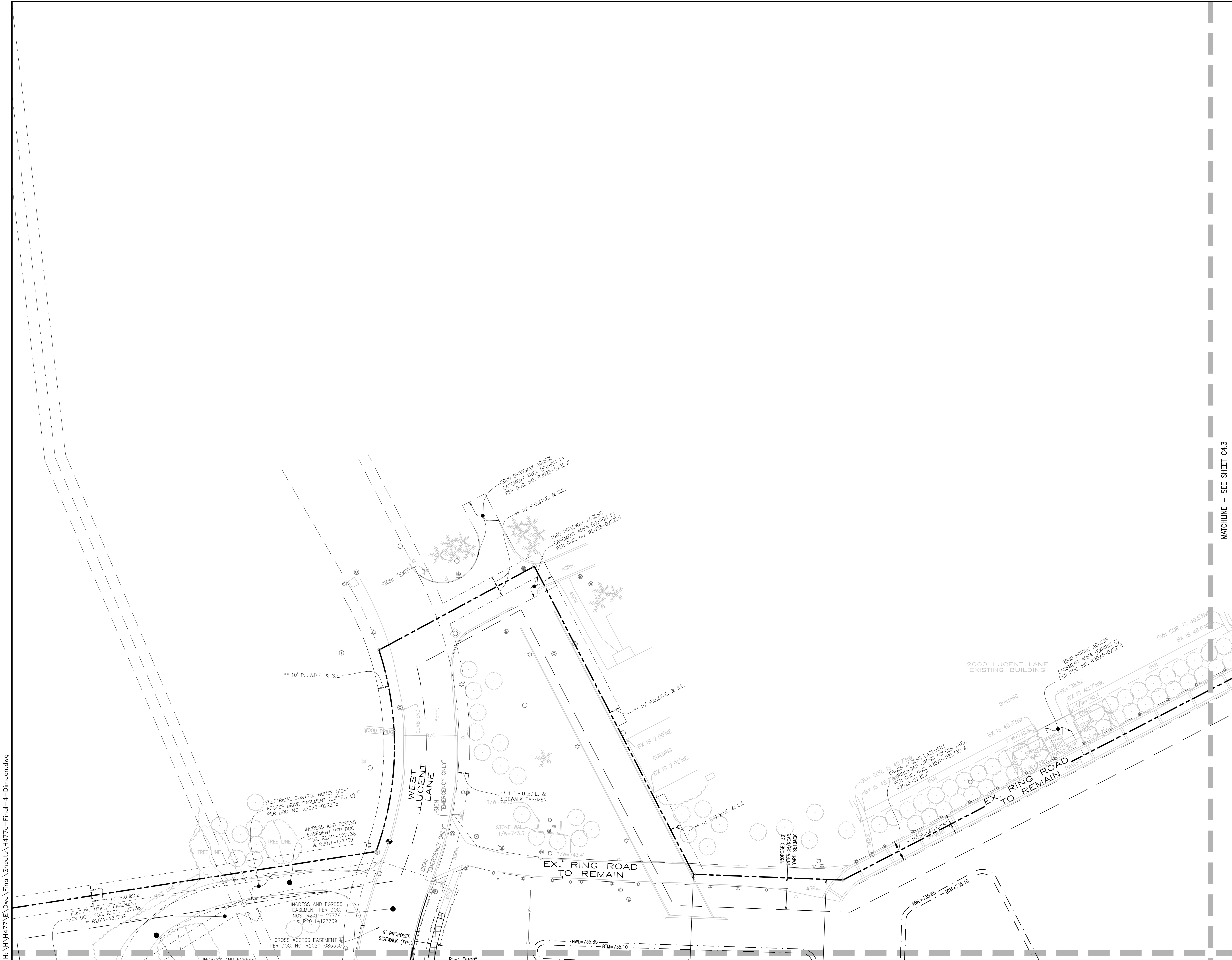


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DIMENSIONAL CONTROL & PAVING PLAN - NORTHEAST	
1960 WEST LUCENT LANE	
NAPERVILLE, ILLINOIS	
KARIS CRITICAL	
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C4.3	



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REFER TO SHEET C4.0 FOR GEOMETRIC NOTES.

PAVEMENT LEGEND

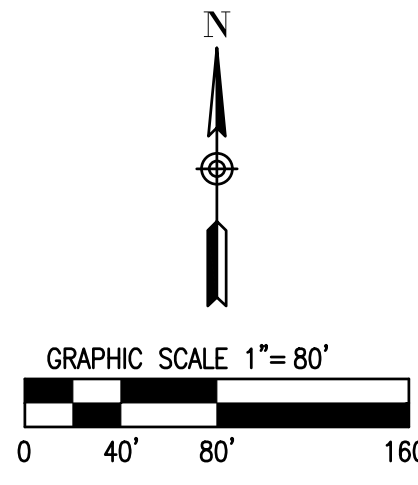
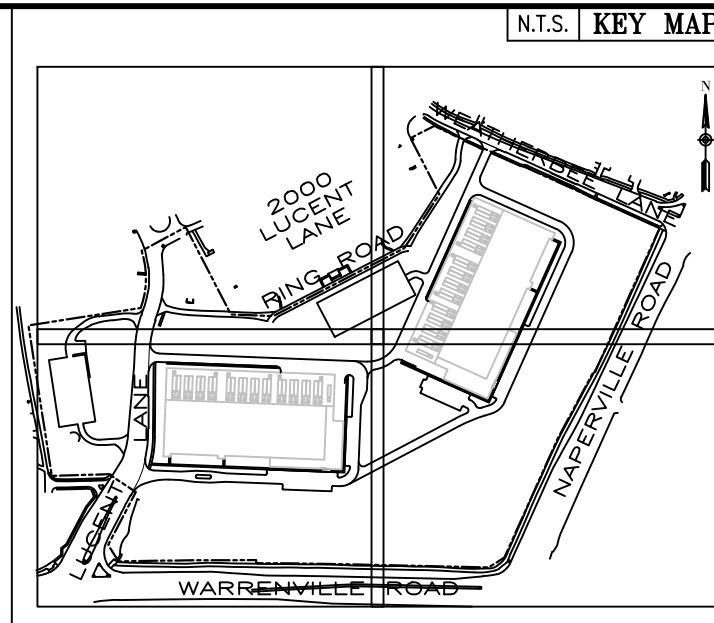
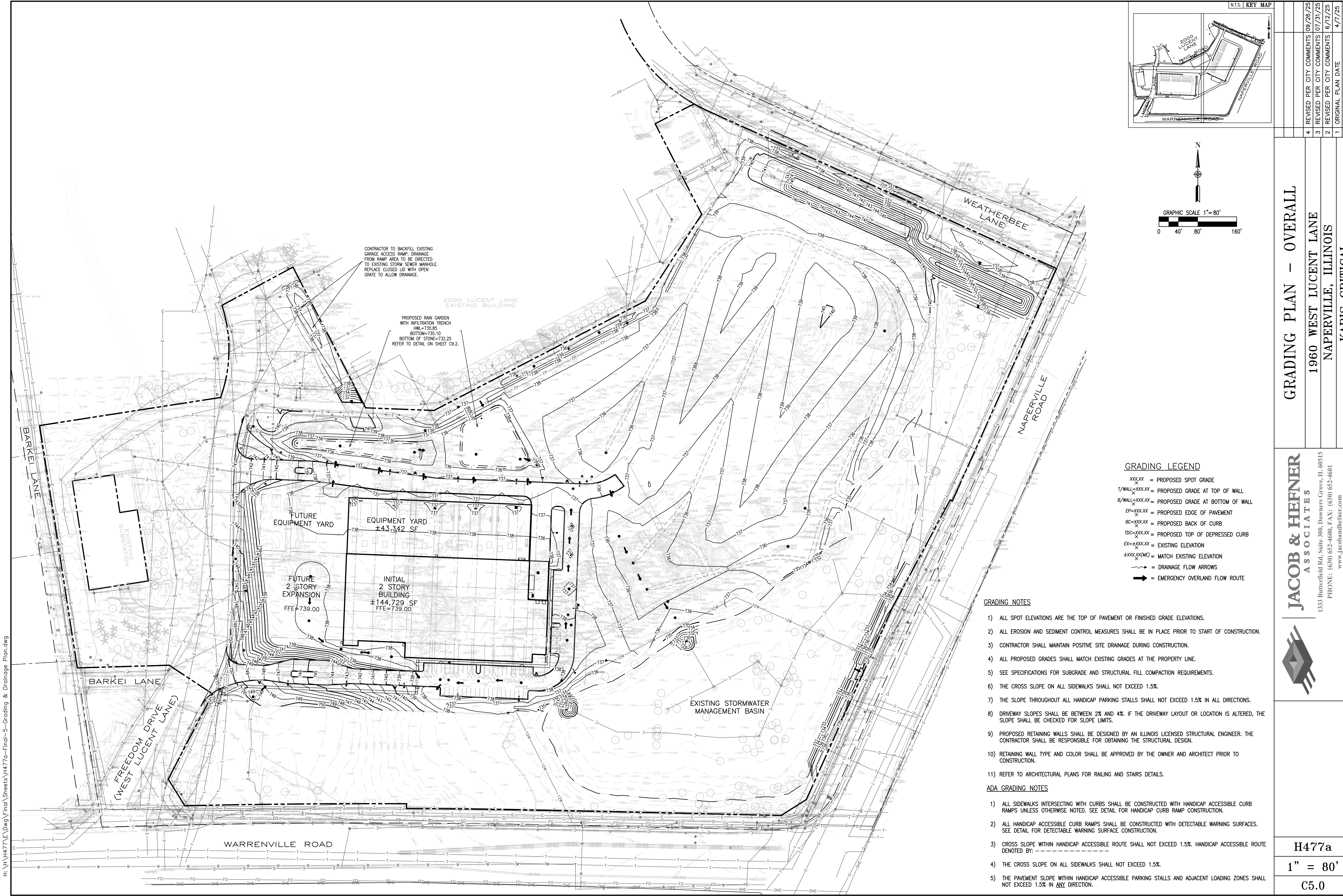
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|---|--|
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 4.0" C-A6 BASE COURSE
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|  | <p>CONCRETE PAVEMENT</p> <p>6.0" PCC PAVEMENT, 4,000 PSI, A/E
 4.0" C-A6 BASE COURSE
 CONTRACTION JOINTS @ 15' C-C MAX.
 EXPANSION JOINTS @ 60' C-C</p> |
|  | <p>PROPOSED GRAVEL</p> <p>12.0" CA-6 AGGREGATE BASE COURSE</p> |

CURB & GUTTER LEGEND

-  REVERSE PITCH CURB & GUTTER
 STANDARD PITCH CURB & GUTTER
 DEPRESSED CURB & GUTTER
 VARIABLE HEIGHT CURB & GUTTER

MATCHLINE - SEE SHEET C4.3

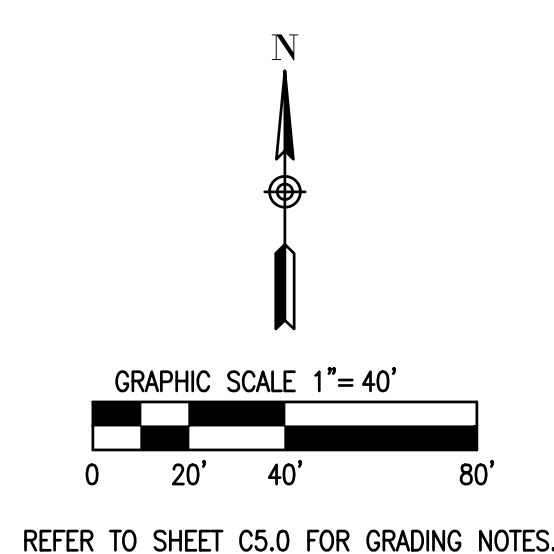
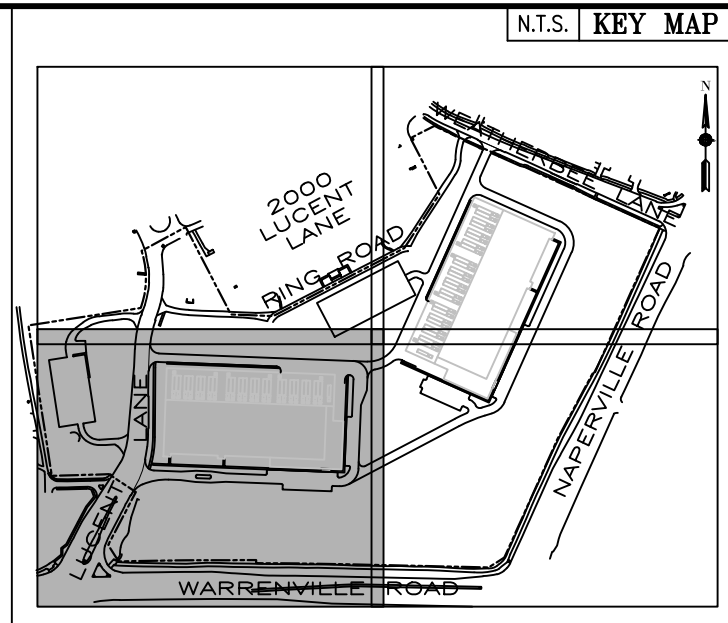
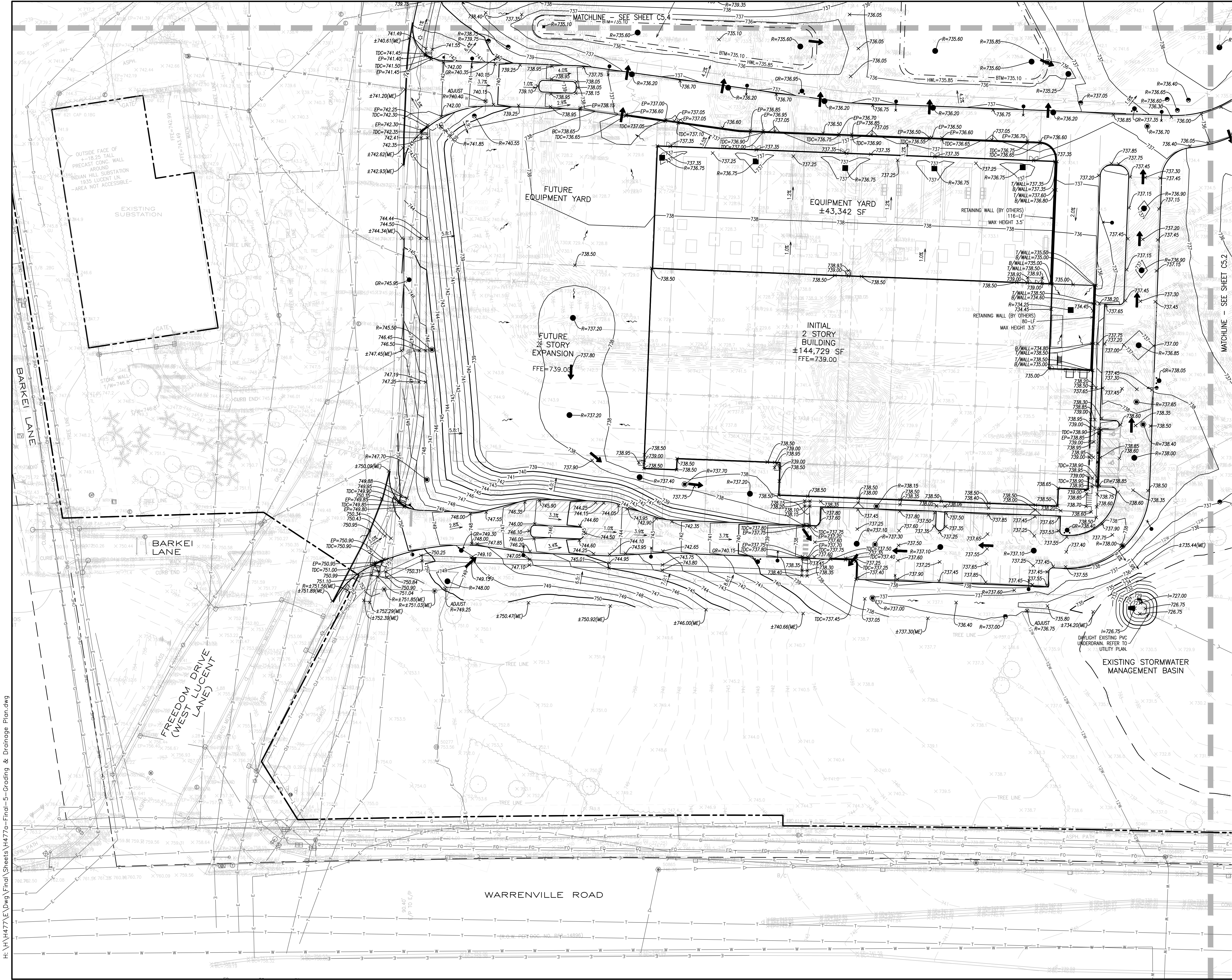
	H477a			JACOB & HEFNER ASSOCIATES 1333 Butterfield Rd., Suite 300, Downers Grove, IL 60515 PHONE: (630) 652-4600, FAX: (630) 652-4601 www.jacobandhefner.com	DIMENSIONAL CONTROL & PAVING PLAN – NORTHWEST 1960 WEST LUCENT LANE NAPERVILLE, ILLINOIS KARIS CRITICAL	<table border="1"> <thead> <tr> <th>No.</th><th>Description</th><th>Date</th></tr> </thead> <tbody> <tr> <td>1</td><td>ORIGINAL PLAN DATE</td><td>4/7/25</td></tr> <tr> <td>2</td><td>REVISED PER CITY COMMENTS</td><td>6/12/25</td></tr> <tr> <td>3</td><td>REVISED PER CITY COMMENTS</td><td>07/31/25</td></tr> <tr> <td>4</td><td>REVISED PER CITY COMMENTS</td><td>09/26/25</td></tr> </tbody> </table>	No.	Description	Date	1	ORIGINAL PLAN DATE	4/7/25	2	REVISED PER CITY COMMENTS	6/12/25	3	REVISED PER CITY COMMENTS	07/31/25	4	REVISED PER CITY COMMENTS	09/26/25
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C4.4	1" = 40'																				



- GRADING LEGEND**
- xxx.xx = PROPOSED SPOT GRADE
 - T/WALL=xxx.xx = PROPOSED GRADE AT TOP OF WALL
 - B/WALL=xxx.xx = PROPOSED GRADE AT BOTTOM OF WALL
 - EP=xxx.xx = PROPOSED EDGE OF PAVEMENT
 - BC=xxx.xx = PROPOSED BACK OF CURB
 - TDC=xxx.xx = PROPOSED TOP OF DEPRESSED CURB
 - EX=±xxx.xx = EXISTING ELEVATION
 - ±xxx.xx(ME) = MATCH EXISTING ELEVATION
 - = DRAINAGE FLOW ARROWS
 - = EMERGENCY OVERLAND FLOW ROUTE

- GRADING NOTES**
- 1) ALL SPOT ELEVATIONS ARE THE TOP OF PAVEMENT OR FINISHED GRADE ELEVATIONS.
 - 2) ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE IN PLACE PRIOR TO START OF CONSTRUCTION.
 - 3) CONTRACTOR SHALL MAINTAIN POSITIVE SITE DRAINAGE DURING CONSTRUCTION.
 - 4) ALL PROPOSED GRADES SHALL MATCH EXISTING GRADES AT THE PROPERTY LINE.
 - 5) SEE SPECIFICATIONS FOR SUBGRADE AND STRUCTURAL FILL COMPACTION REQUIREMENTS.
 - 6) THE CROSS SLOPE ON ALL SIDEWALKS SHALL NOT EXCEED 1.5%.
 - 7) THE SLOPE THROUGHOUT ALL HANDICAP PARKING STALLS SHALL NOT EXCEED 1.5% IN ALL DIRECTIONS.
 - 8) DRIVEWAY SLOPES SHALL BE BETWEEN 2% AND 4%. IF THE DRIVEWAY LAYOUT OR LOCATION IS ALTERED, THE SLOPE SHALL BE CHECKED FOR SLOPE LIMITS.
 - 9) PROPOSED RETAINING WALLS SHALL BE DESIGNED BY AN ILLINOIS LICENSED STRUCTURAL ENGINEER. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING THE STRUCTURAL DESIGN.
 - 10) RETAINING WALL TYPE AND COLOR SHALL BE APPROVED BY THE OWNER AND ARCHITECT PRIOR TO CONSTRUCTION.
 - 11) REFER TO ARCHITECTURAL PLANS FOR RAILING AND STAIRS DETAILS.
- ADA GRADING NOTES**
- 1) ALL SIDEWALKS INTERSECTING WITH CURBS SHALL BE CONSTRUCTED WITH HANDICAP ACCESSIBLE CURB RAMPS UNLESS OTHERWISE NOTED. SEE DETAIL FOR HANDICAP CURB RAMP CONSTRUCTION.
 - 2) ALL HANDICAP ACCESSIBLE CURB RAMPS SHALL BE CONSTRUCTED WITH DETECTABLE WARNING SURFACES. SEE DETAIL FOR DETECTABLE WARNING SURFACE CONSTRUCTION.
 - 3) CROSS SLOPE WITHIN HANDICAP ACCESSIBLE ROUTE SHALL NOT EXCEED 1.5%. HANDICAP ACCESSIBLE ROUTE DENOTED BY: - - - - -
 - 4) THE CROSS SLOPE ON ALL SIDEWALKS SHALL NOT EXCEED 1.5%.
 - 5) THE PAVEMENT SLOPE WITHIN HANDICAP ACCESSIBLE PARKING STALLS AND ADJACENT LOADING ZONES SHALL NOT EXCEED 1.5% IN ANY DIRECTION.

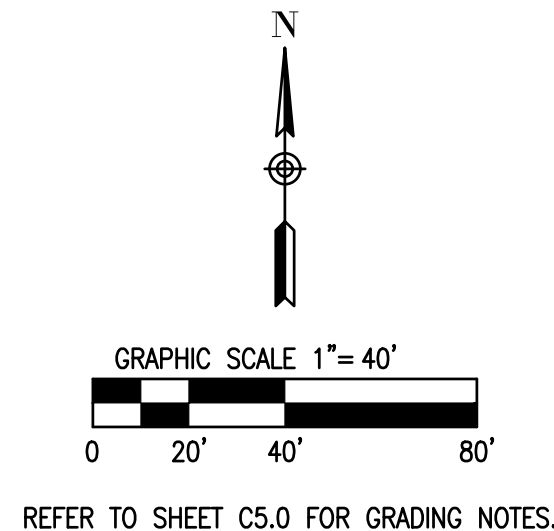
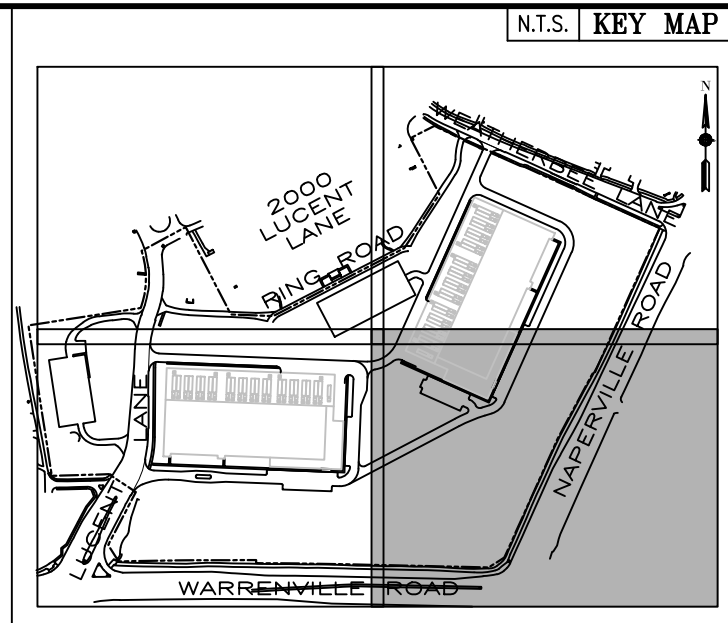
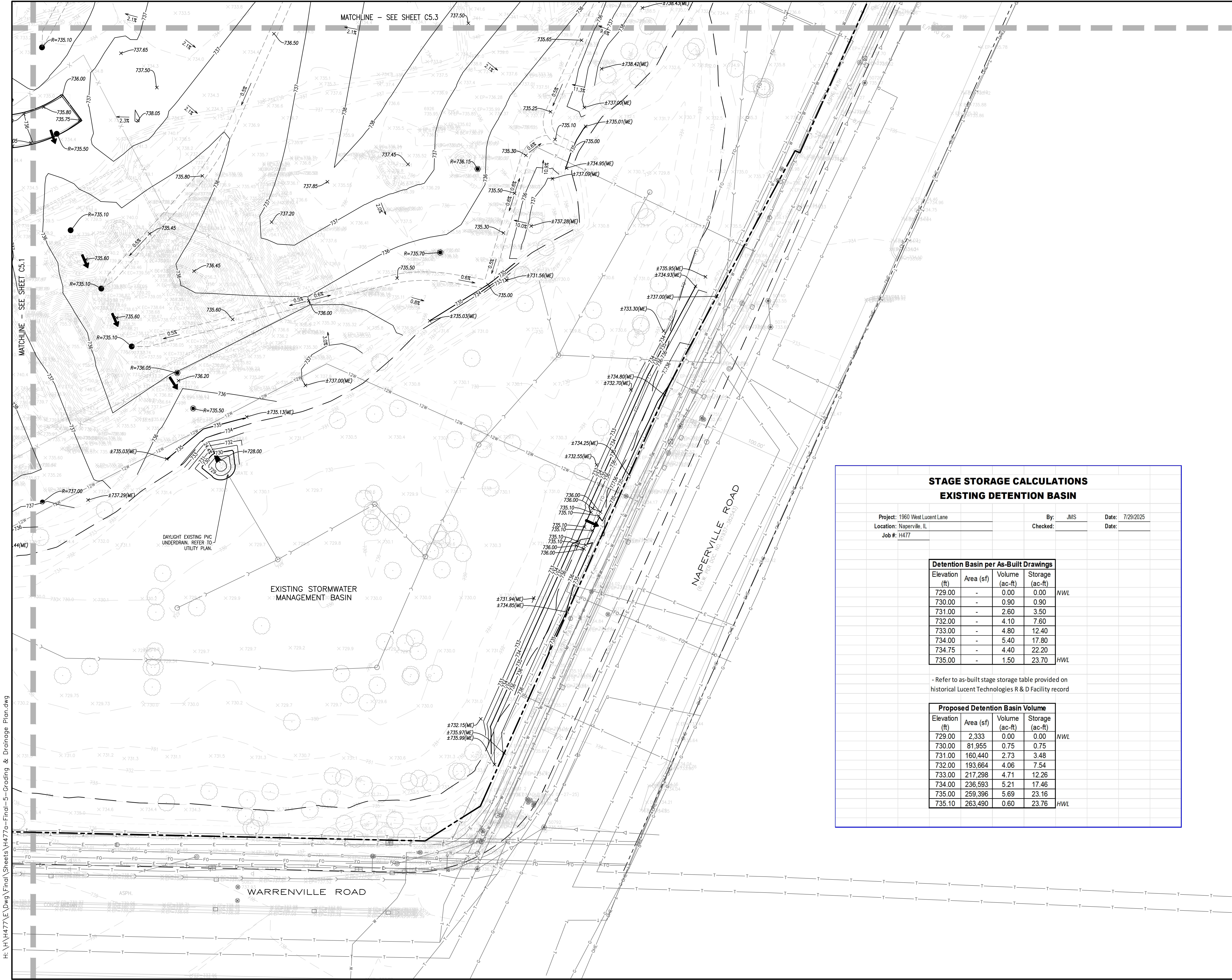
GRADING PLAN - OVERALL	
1960 WEST LUCENT LANE	
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GRADING PLAN - SOUTHWEST	
1960 WEST LUCENT LANE	
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**STAGE STORAGE CALCULATIONS
EXISTING DETENTION BASIN**

Project: 1960 West Lucent Lane By: JMS Date: 7/29/2025
Location: Naperville, IL Checked: Date:
Job #: H477

Detention Basin per As-Built Drawings				
Elevation (ft)	Area (sf)	Volume (ac-ft)	Storage (ac-ft)	
729.00	-	0.00	0.00	NWL
730.00	-	0.90	0.90	
731.00	-	2.60	3.50	
732.00	-	4.10	7.60	
733.00	-	4.80	12.40	
734.00	-	5.40	17.80	
734.75	-	4.40	22.20	
735.00	-	1.50	23.70	HWL

- Refer to as-built stage storage table provided on historical Lucent Technologies R & D Facility record

Proposed Detention Basin Volume				
Elevation (ft)	Area (sf)	Volume (ac-ft)	Storage (ac-ft)	
729.00	2,333	0.00	0.00	NWL
730.00	81,955	0.75	0.75	
731.00	160,440	2.73	3.48	
732.00	193,664	4.06	7.54	
733.00	217,298	4.71	12.26	
734.00	236,593	5.21	17.46	
735.00	259,396	5.69	23.16	
735.10	263,490	0.60	23.76	HWL

4 REVISED PER CITY COMMENTS 09/26/25

3 REVISED PER CITY COMMENTS 07/31/25

2 REVISED PER CITY COMMENTS 6/12/25

1 ORIGINAL PLAN DATE 4/7/25

No.

Description

Date

GRADING PLAN - SOUTHEAST

1960 WEST LUCENT LANE

NAPERVILLE, ILLINOIS

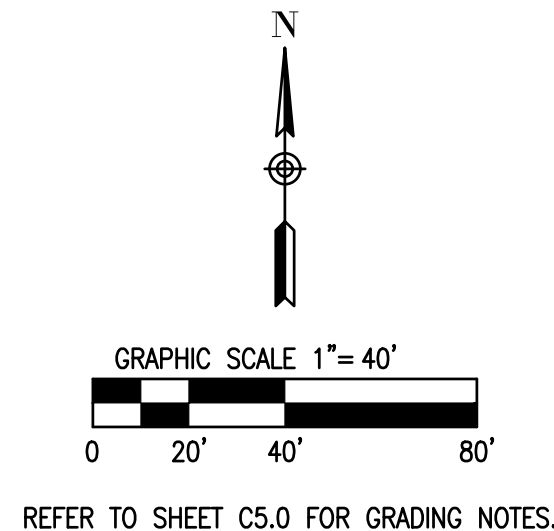
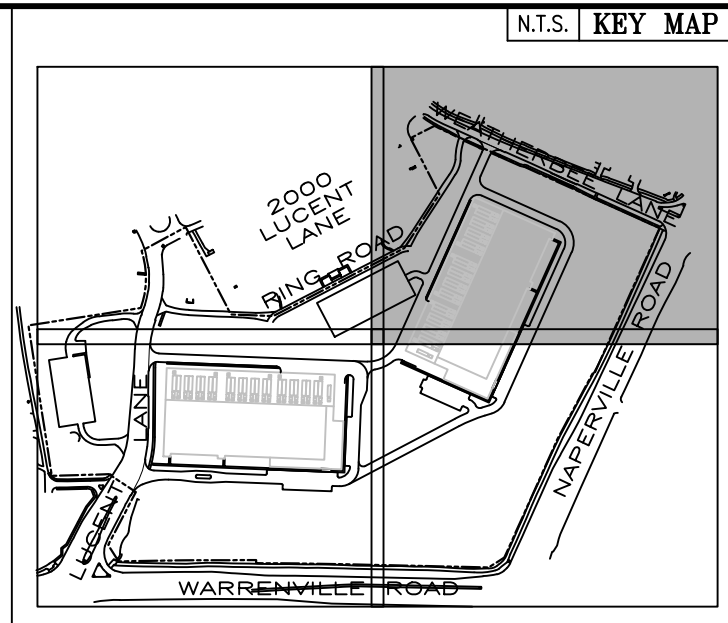
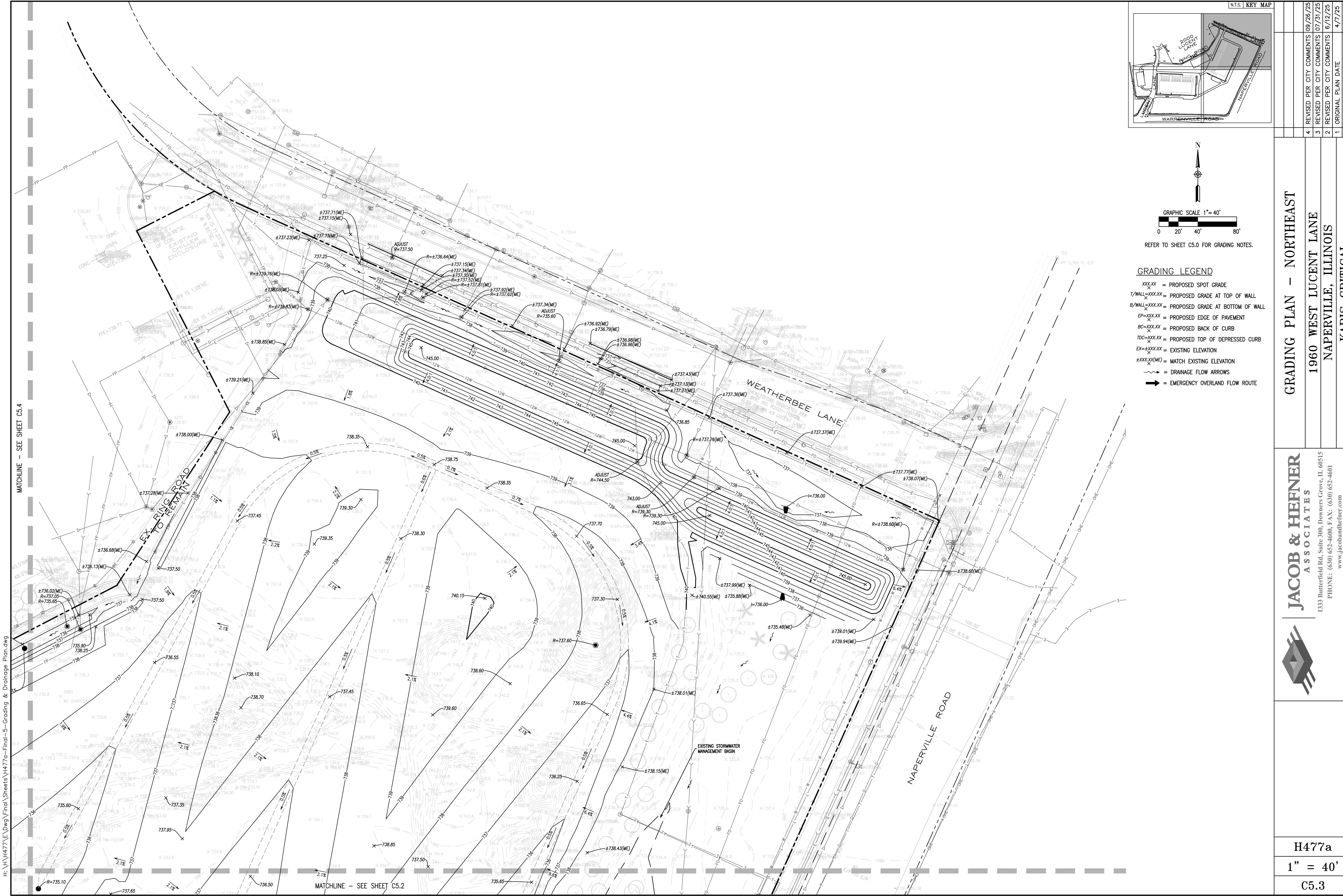
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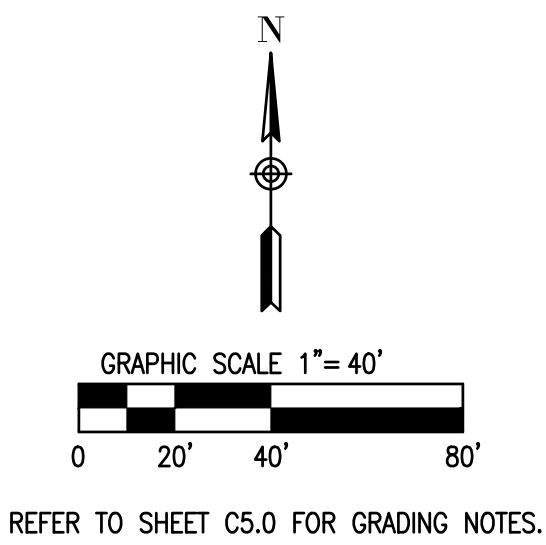
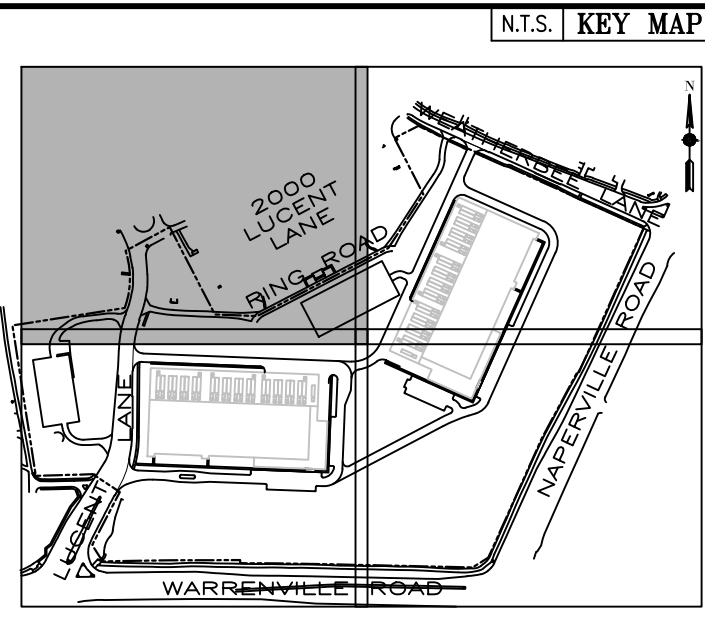
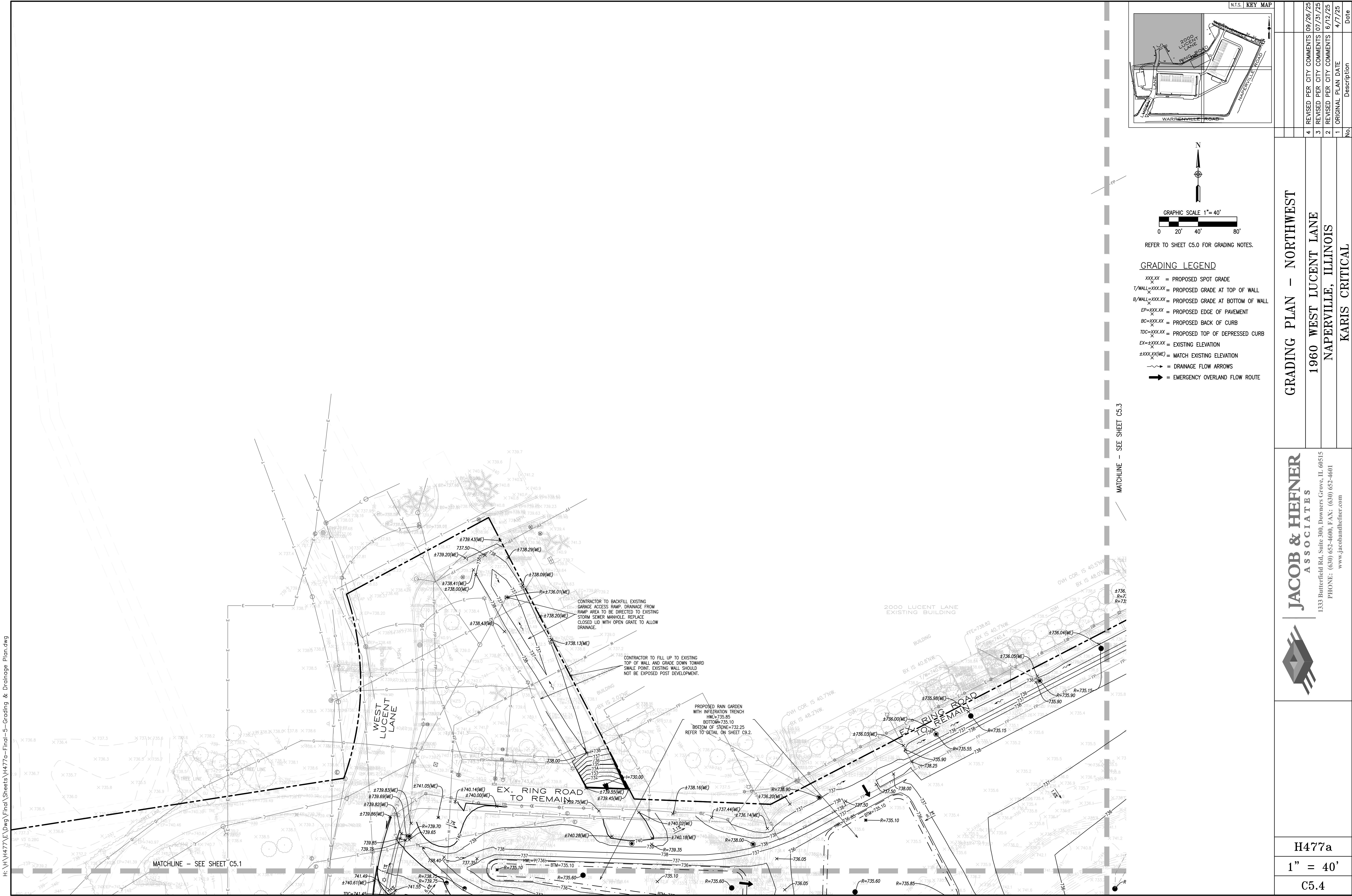
1" = 40'

C5.2



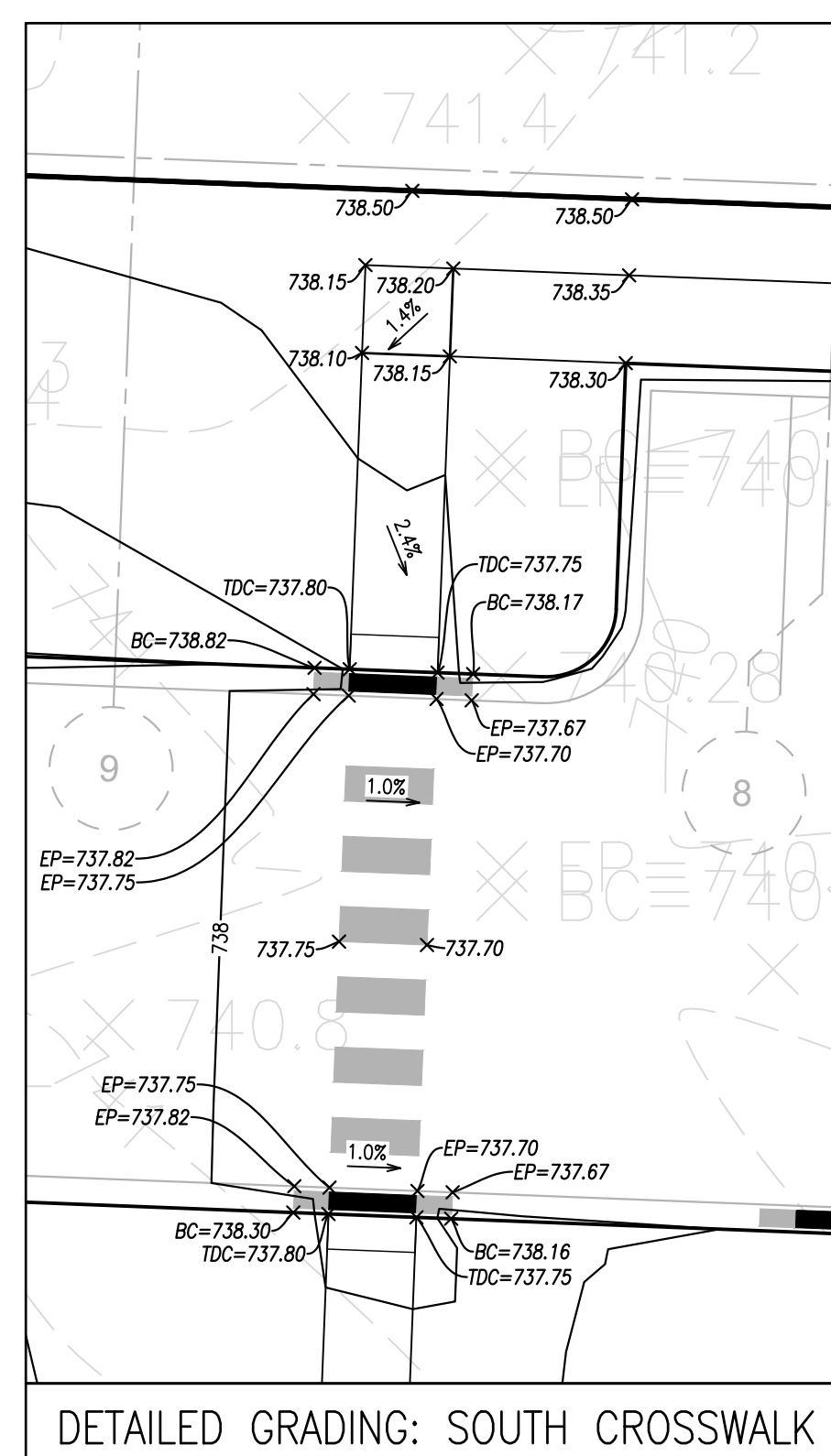
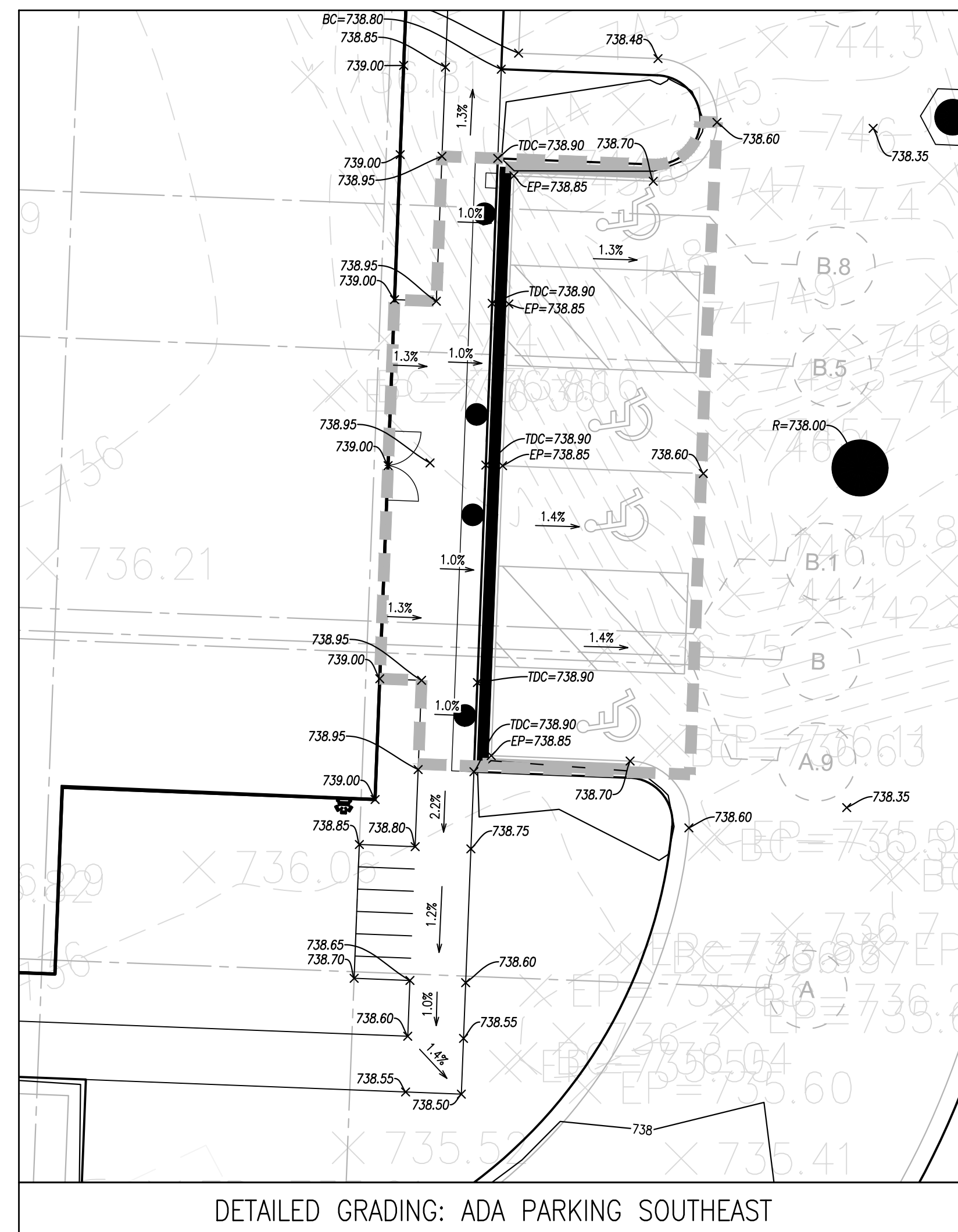
- GRADING LEGEND
- XXX.XX = PROPOSED SPOT GRADE
 - T/WALL=XXX.XX = PROPOSED GRADE AT TOP OF WALL
 - B/WALL=XXX.XX = PROPOSED GRADE AT BOTTOM OF WALL
 - EP=XXX.XX = PROPOSED EDGE OF PAVEMENT
 - BC=XXX.XX = PROPOSED BACK OF CURB
 - TDC=XXX.XX = PROPOSED TOP OF DEPRESSED CURB
 - EX=±XXX.XX = EXISTING ELEVATION
 - ±XXX.XX(ME) = MATCH EXISTING ELEVATION
 - = DRAINAGE FLOW ARROWS
 - ➔ = EMERGENCY OVERLAND FLOW ROUTE

Revised Per City Comments		Date
4	REVISED PER CITY COMMENTS	09/26/25
3	REVISED PER CITY COMMENTS	07/31/25
2	REVISED PER CITY COMMENTS	6/12/25
1	ORIGINAL PLAN DATE	4/7/25
Description		Date
GRADING PLAN - NORTHEAST		
1960 WEST LUCENT LANE		
NAPERVILLE, ILLINOIS		
KARIS CRITICAL		
H477a		
1" = 40'		
C5.3		



- GRADING LEGEND**
- XXX.XX = PROPOSED SPOT GRADE
 - T/WALL=XXX.XX = PROPOSED GRADE AT TOP OF WALL
 - B/WALL=XXX.XX = PROPOSED GRADE AT BOTTOM OF WALL
 - EP=XXX.XX = PROPOSED EDGE OF PAVEMENT
 - BC=XXX.XX = PROPOSED BACK OF CURB
 - TDC=XXX.XX = PROPOSED TOP OF DEPRESSION CURB
 - EX=±XXX.XX = EXISTING ELEVATION
 - ±XXX.XX(ME) = MATCH EXISTING ELEVATION
 - = DRAINAGE FLOW ARROWS
 - ➔ = EMERGENCY OVERLAND FLOW ROUTE

No.		Description	Date
4	REVISED PER CITY COMMENTS	09/26/25	
3	REVISED PER CITY COMMENTS	07/31/25	
2	REVISED PER CITY COMMENTS	6/12/25	
1	ORIGINAL PLAN DATE	4/7/25	
GRADING PLAN - NORTHWEST		KARIS CRITICAL	
1960 WEST LUCENT LANE			
NAPERVILLE, ILLINOIS			
JACOB & HEFNER ASSOCIATES			
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PHONE: (630) 652-4600. FAX: (630) 652-4601			
www.jacobandhefner.com			
H477a			
1" = 40'			
C5.4			

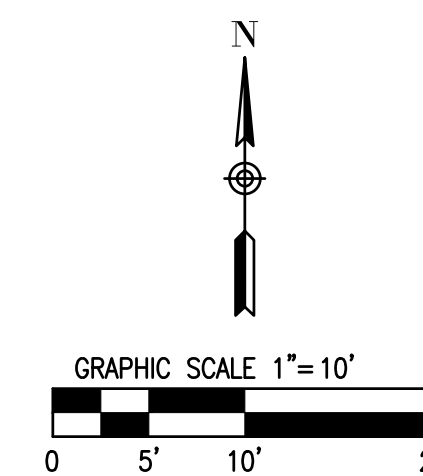
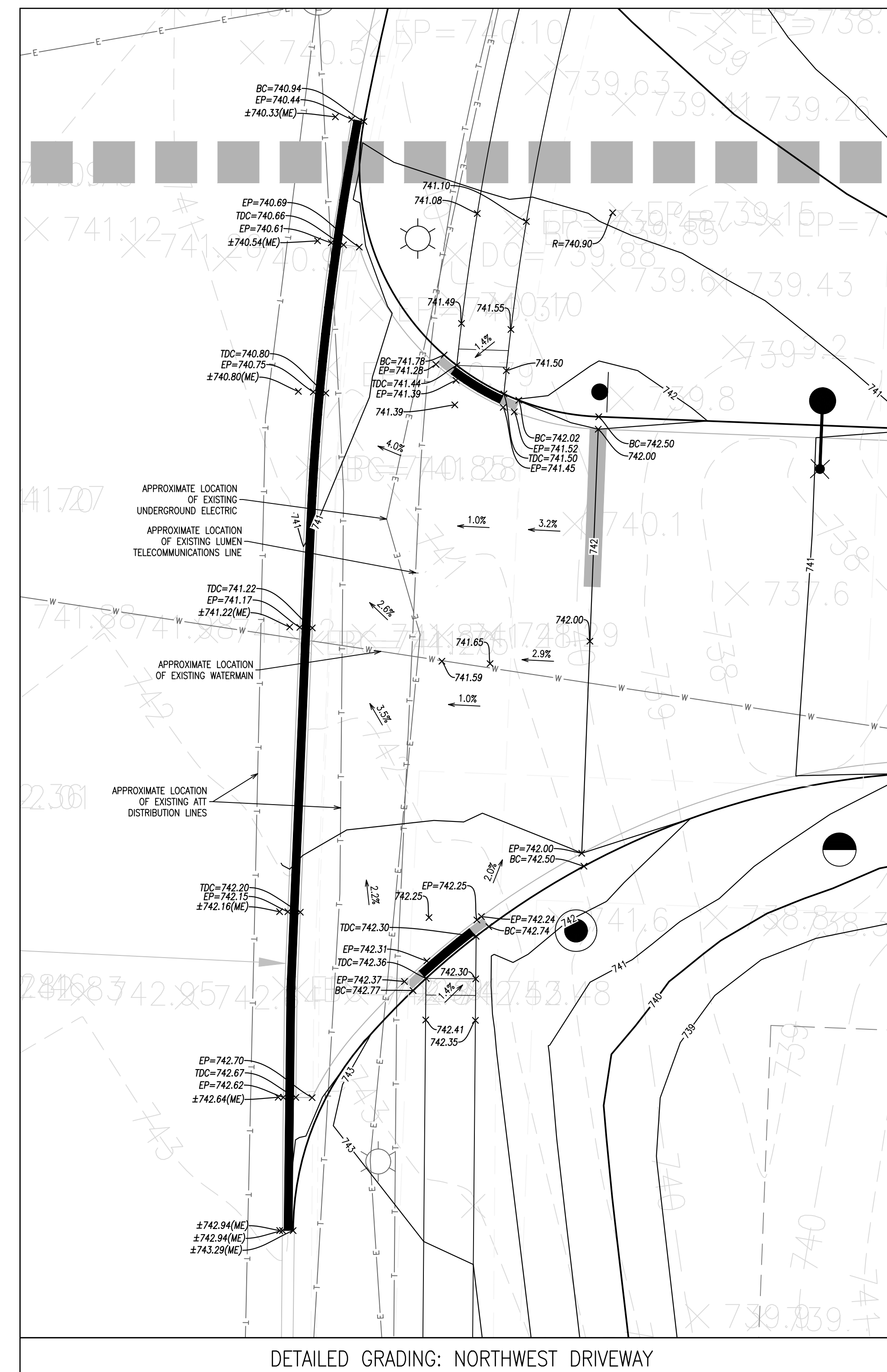
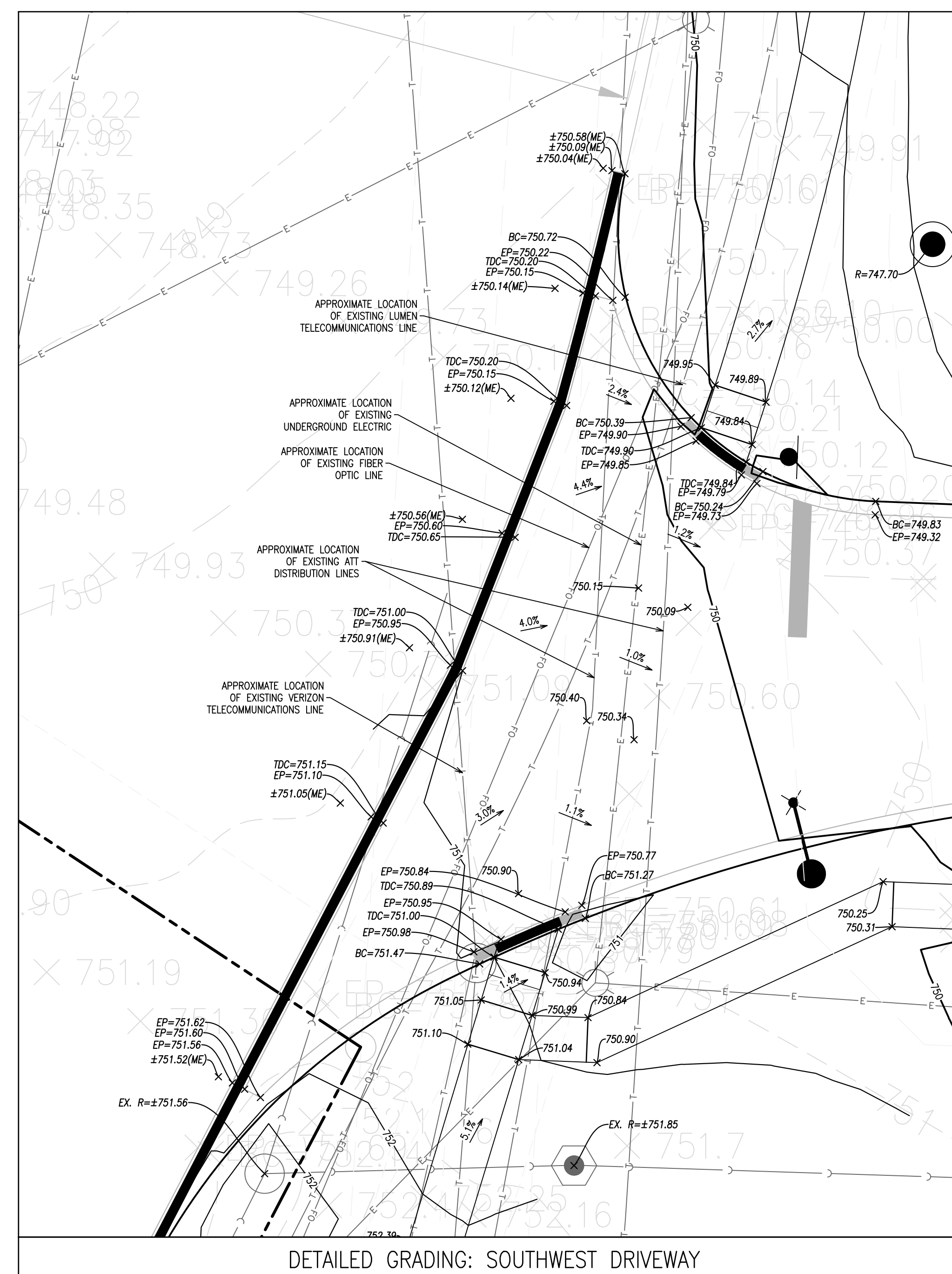


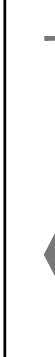
- ADA GRADING NOTES

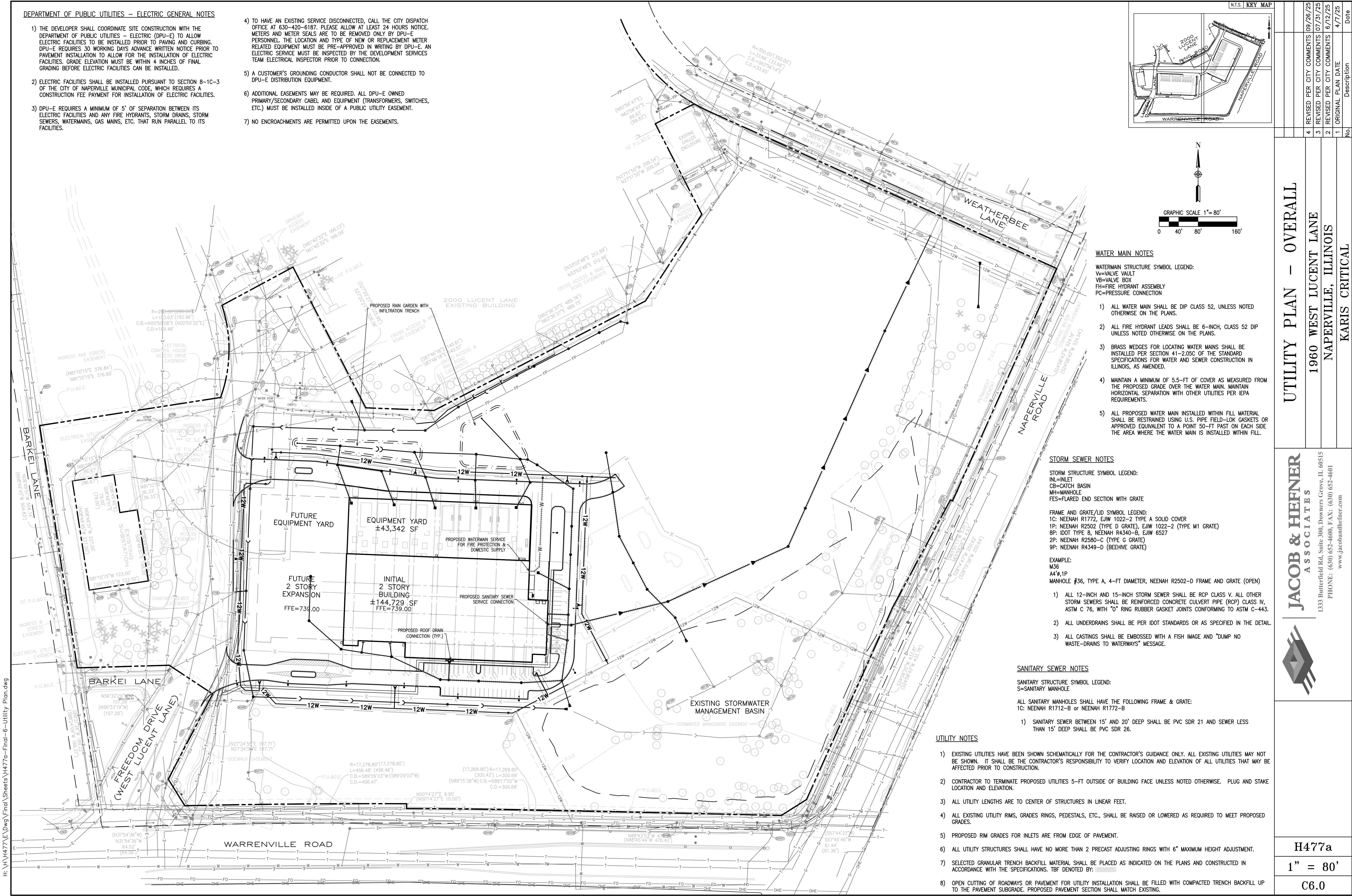
- 1) ALL SIDEWALKS INTERSECTING WITH CURBS SHALL BE CONSTRUCTED WITH HANDICAP ACCESSIBLE CURB RAMP UNLESS OTHERWISE NOTED. SEE DETAIL FOR HANDICAP CURB RAMP CONSTRUCTION.
- 2) ALL HANDICAP ACCESSIBLE CURB RAMP SHALL BE CONSTRUCTED WITH DETECTABLE WARNING SURFACES. SEE DETAIL FOR DETECTABLE WARNING SURFACE CONSTRUCTION.
- 3) CROSS SLOPE WITHIN HANDICAP ACCESSIBLE ROUTE SHALL NOT EXCEED 1.5%. HANDICAP ACCESSIBLE ROUTE DENOTED BY: - - - - -
- 4) THE CROSS SLOPE ON ALL SIDEWALKS SHALL NOT EXCEED 1.5%.
- 5) THE PAVEMENT SLOPE WITHIN HANDICAP ACCESSIBLE PARKING STALLS AND ADJACENT LOADING ZONES SHALL NOT EXCEED 1.5% IN ANY DIRECTION.
- 6) REFER TO SHEET C5.0 FOR OVERALL GRADING PLAN AND NOTES.

GRADING LEGEND

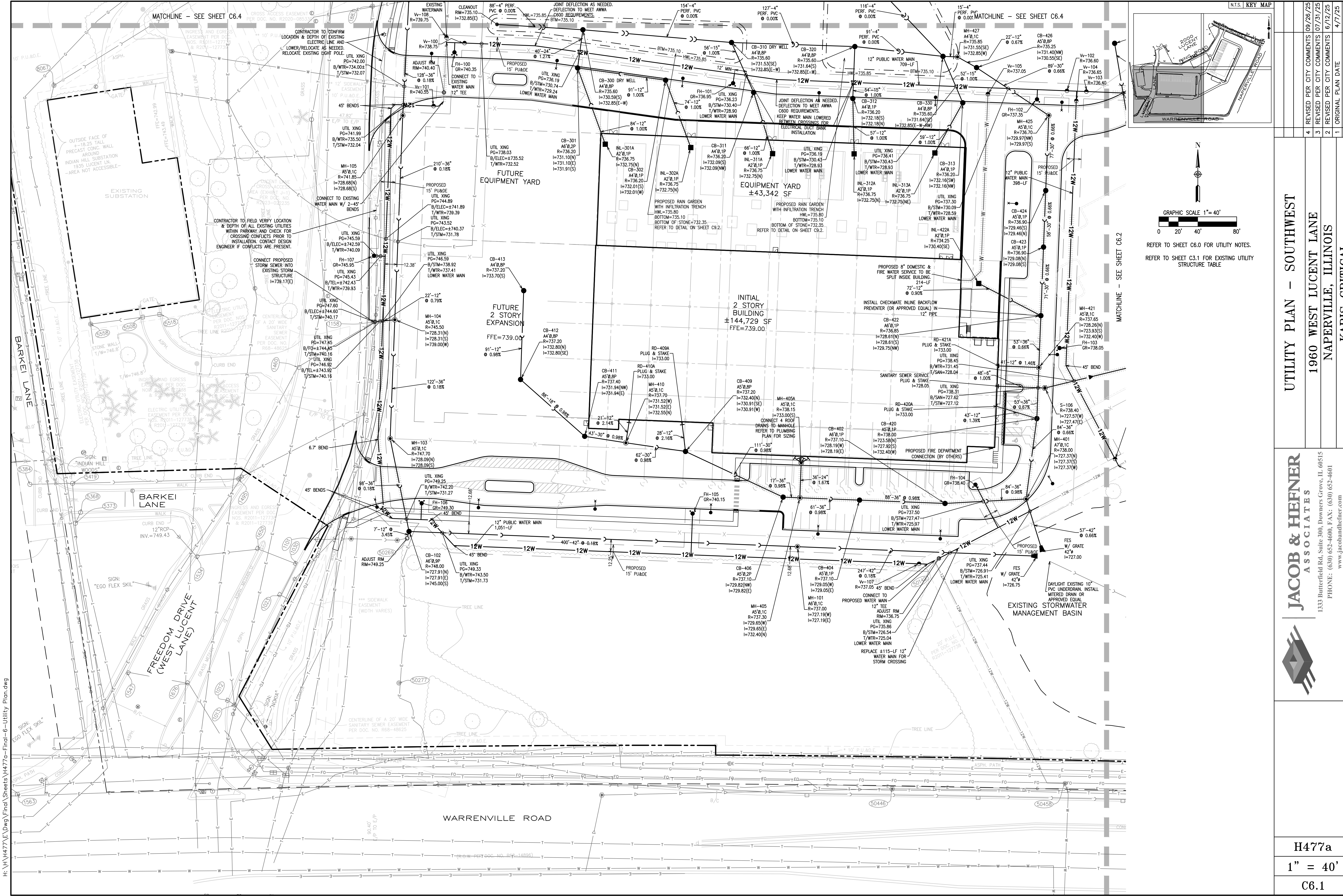
$\begin{array}{c} XXX.XX \\ \times \end{array}$ = PROPOSED SPOT GRADE
 $\begin{array}{c} BC=XXX.XX \\ \times \end{array}$ = PROPOSED BACK OF CURB
 $\begin{array}{c} EP=XXX.XX \\ \times \end{array}$ = PROPOSED EDGE OF PAVEMENT
 $\begin{array}{c} TDC=XXX.XX \\ \times \end{array}$ = PROPOSED TOP OF DEPRESSED CURB
 $\begin{array}{c} R=XXX.XX \\ \times \end{array}$ = PROPOSED RIM ELEVATION
 EX. $\begin{array}{c} R=XXX.XX \\ \times \end{array}$ = EXISTING RIM ELEVATION
 $\pm XXX.XX(ME)$ = MATCH EXISTING ELEVATION
 = DRAINAGE FLOW ARROWS
 = EMERGENCY OVERLAND FLOW ROUTE

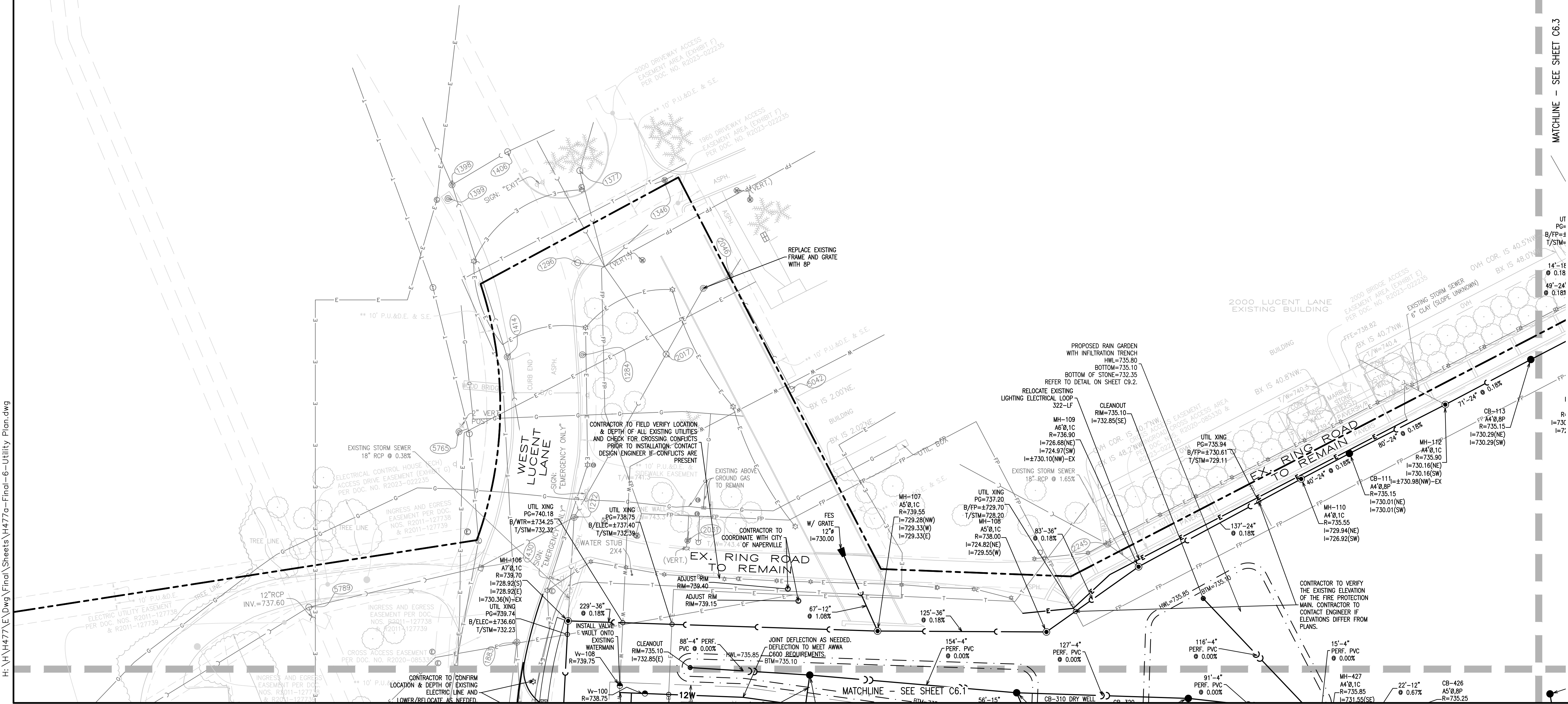


C5.5	1" = 10'	H477a	 JACOB & HEFNER ASSOCIATES 1333 Butterfield Rd, Suite 300, Downers Grove, IL 60515 PHONE: (630) 652-4600, FAX: (630) 652-4601 www.jacobandhefner.com	DETAILED GRADING PLAN					
				1960 WEST LUCENT LANE					
				NAPERVILLE, ILLINOIS					
				1 ORIGINAL PLAN DATE 4/7/25 2 REVISED PER CITY COMMENTS 6/12/25 3 REVISED PER CITY COMMENTS 07/31/25 4 REVISED PER CITY COMMENTS 09/26/25					
				No Description KARIS CRITICAL.					Date

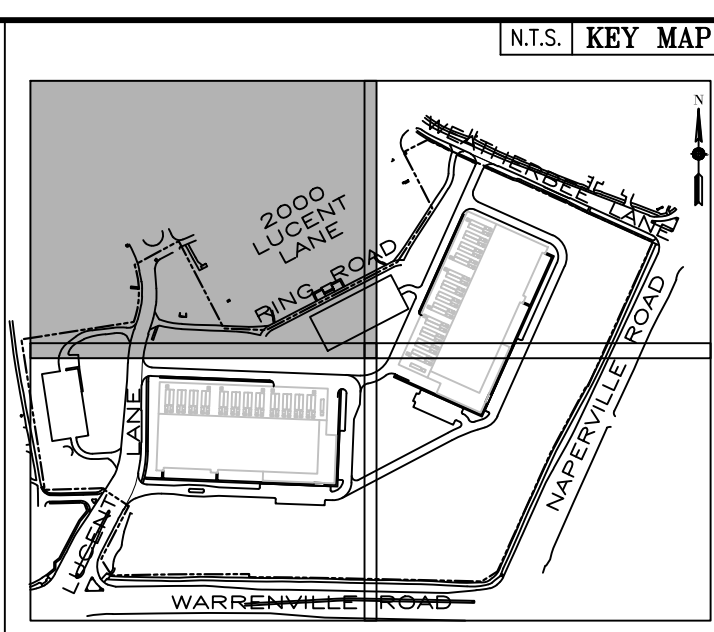


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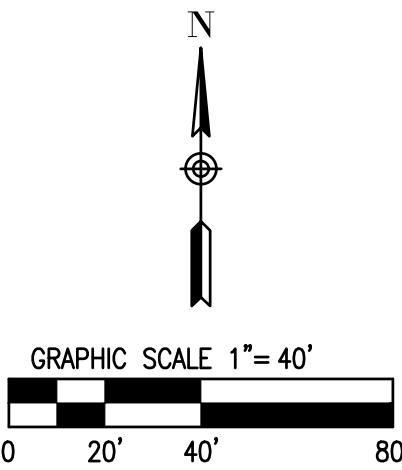




MATCHLINE - SEE SHEET C6.3



N.T.S.	KEY MAP
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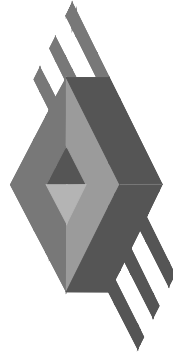
REFER TO SHEET C6.0 FOR UTILITY NOTES.
REFER TO SHEET C3.4 FOR EXISTING UTILITY
STRUCTURE TABLE

[illegible]

UTILITY PLAN – NORTHWEST

1960 WEST LUCENT LANE

NAPERVILLE, ILLINOIS

KARIS CRITICAL

JACOB & HEFNER
ASSOCIATES

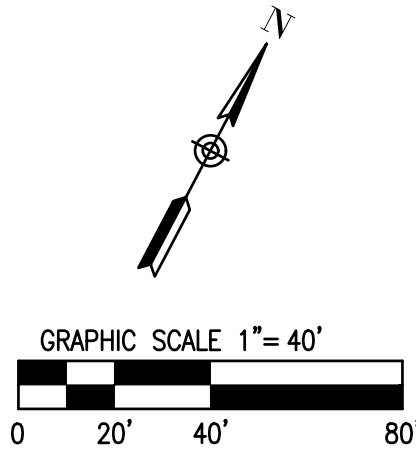
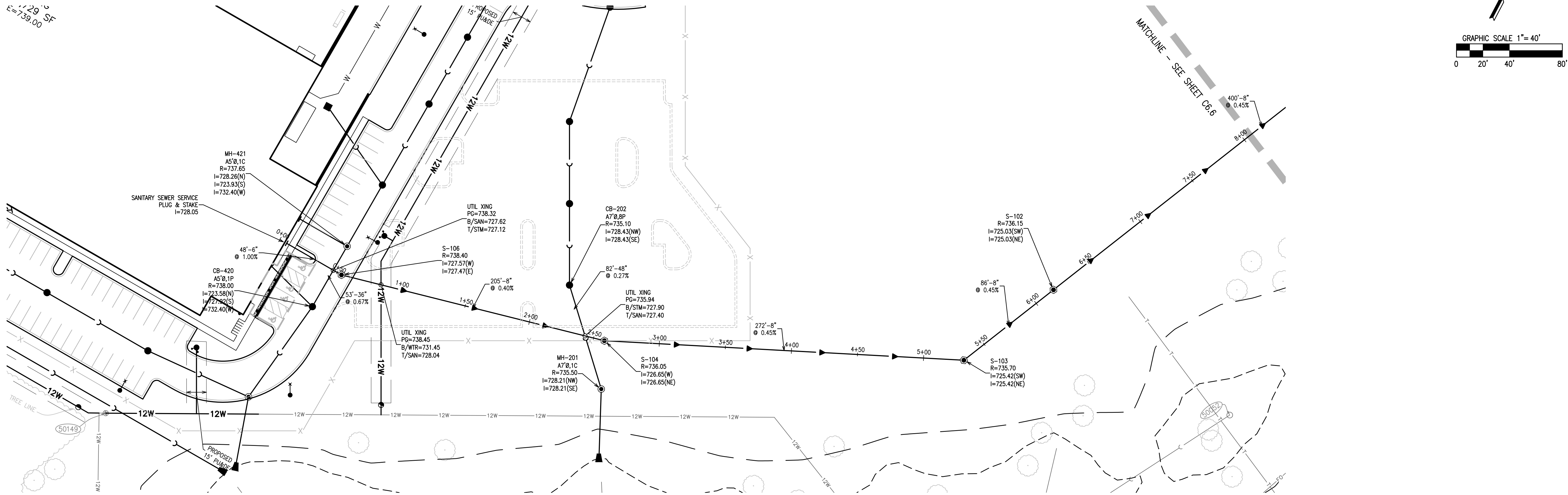
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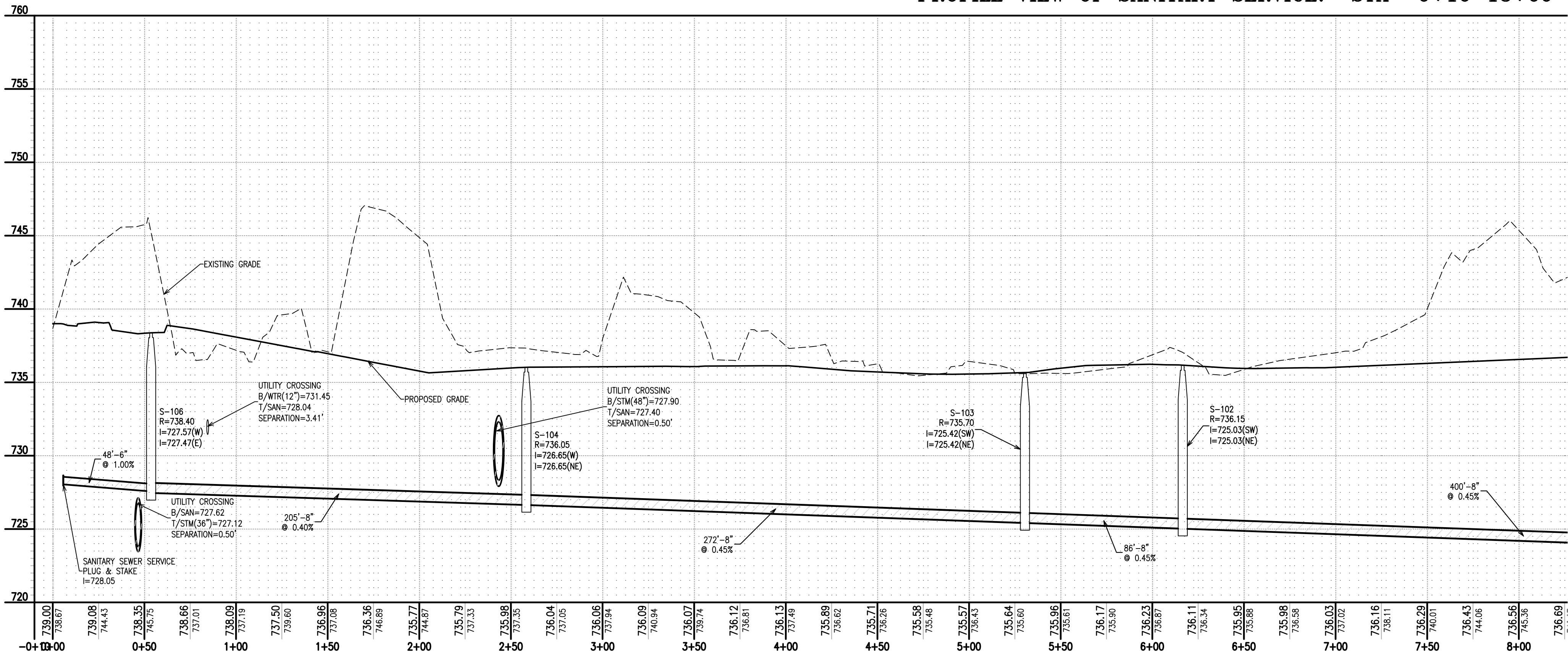
$$1'' = 40'$$

C6.4

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PROFILE VIEW OF SANITARY SERVICE: STA -0+10-13+00

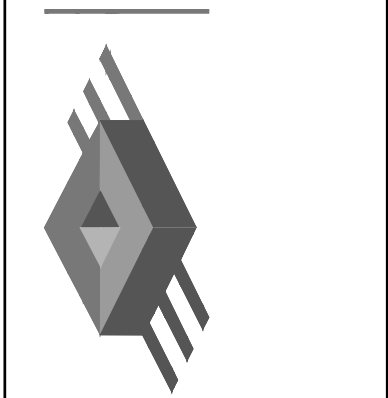


PLAN AND PROFILE NOTES
SEE SHEET C6.0 FOR UTILITY NOTES

PROFILE LEGEND	
	PROPOSED GROUND SURFACE
	EXISTING GROUND SURFACE
	STORM SEWER
	SANITARY SEWER

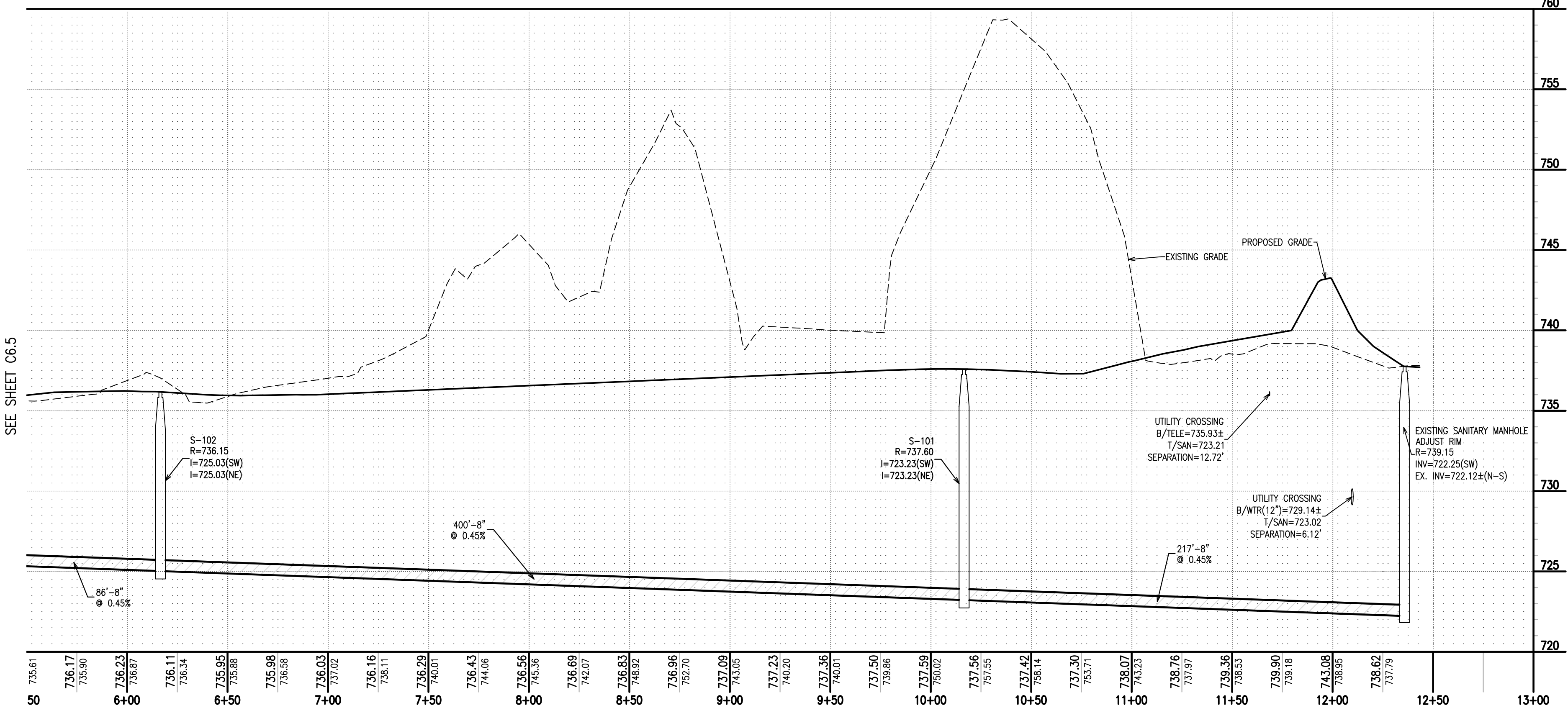
SEE SHEET C6.6

SANITARY PLAN & PROFILE		1960 WEST LUCENT LANE		NAPERVILLE, ILLINOIS		KARIS CRITICAL	
4	REVISED PER CITY COMMENTS	09/26/25					
3	REVISED PER CITY COMMENTS	07/31/25					
2	REVISED PER CITY COMMENTS	6/12/25					
1	ORIGINAL PLAN DATE	4/7/25					
No.	Description						Date



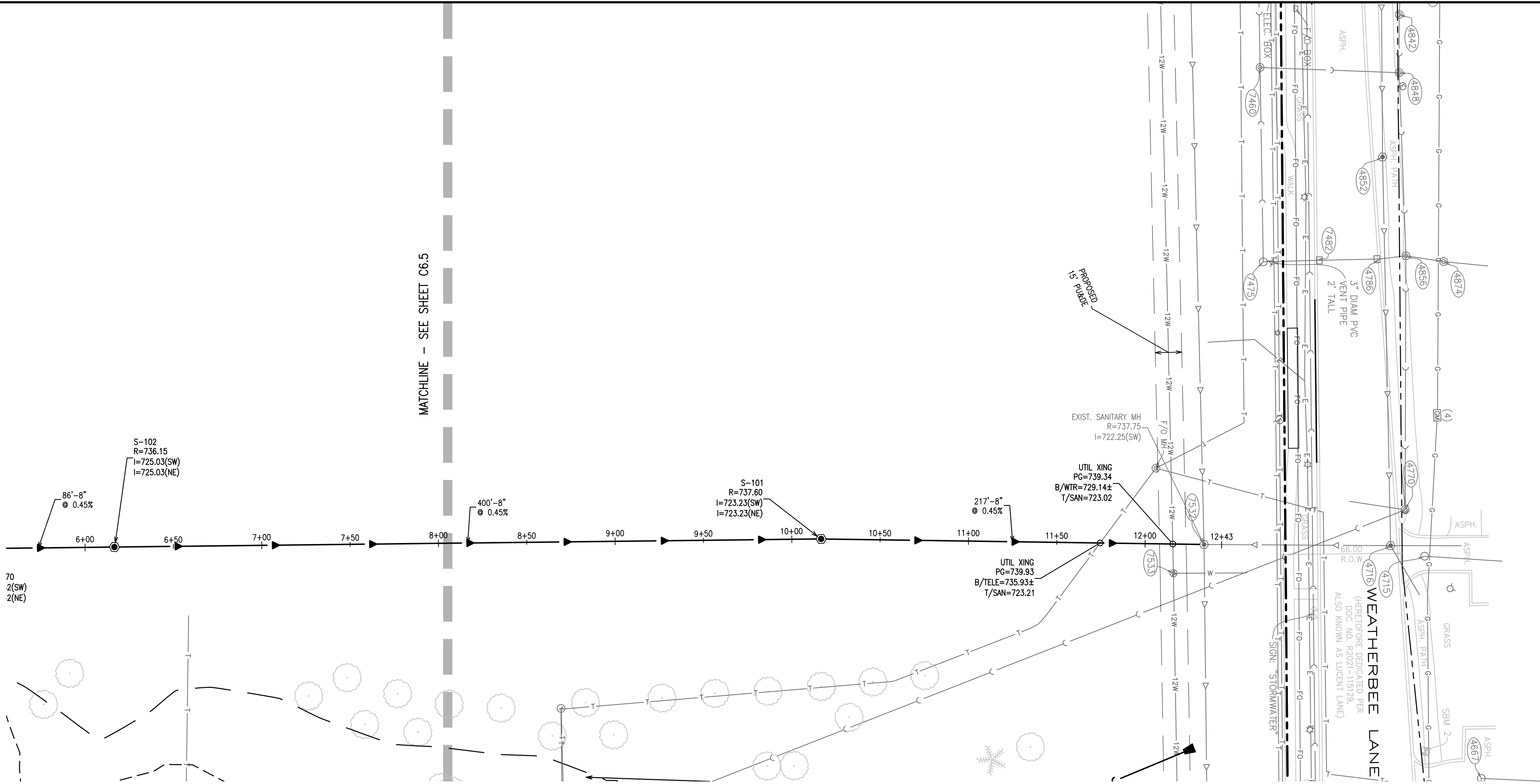
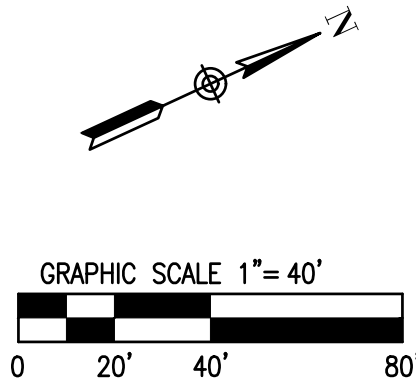
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OF SANITARY SERVICE: STA -0+10-13+00



PLAN AND PROFILE NOTES
SEE SHEET C6.0 FOR UTILITY NOTES

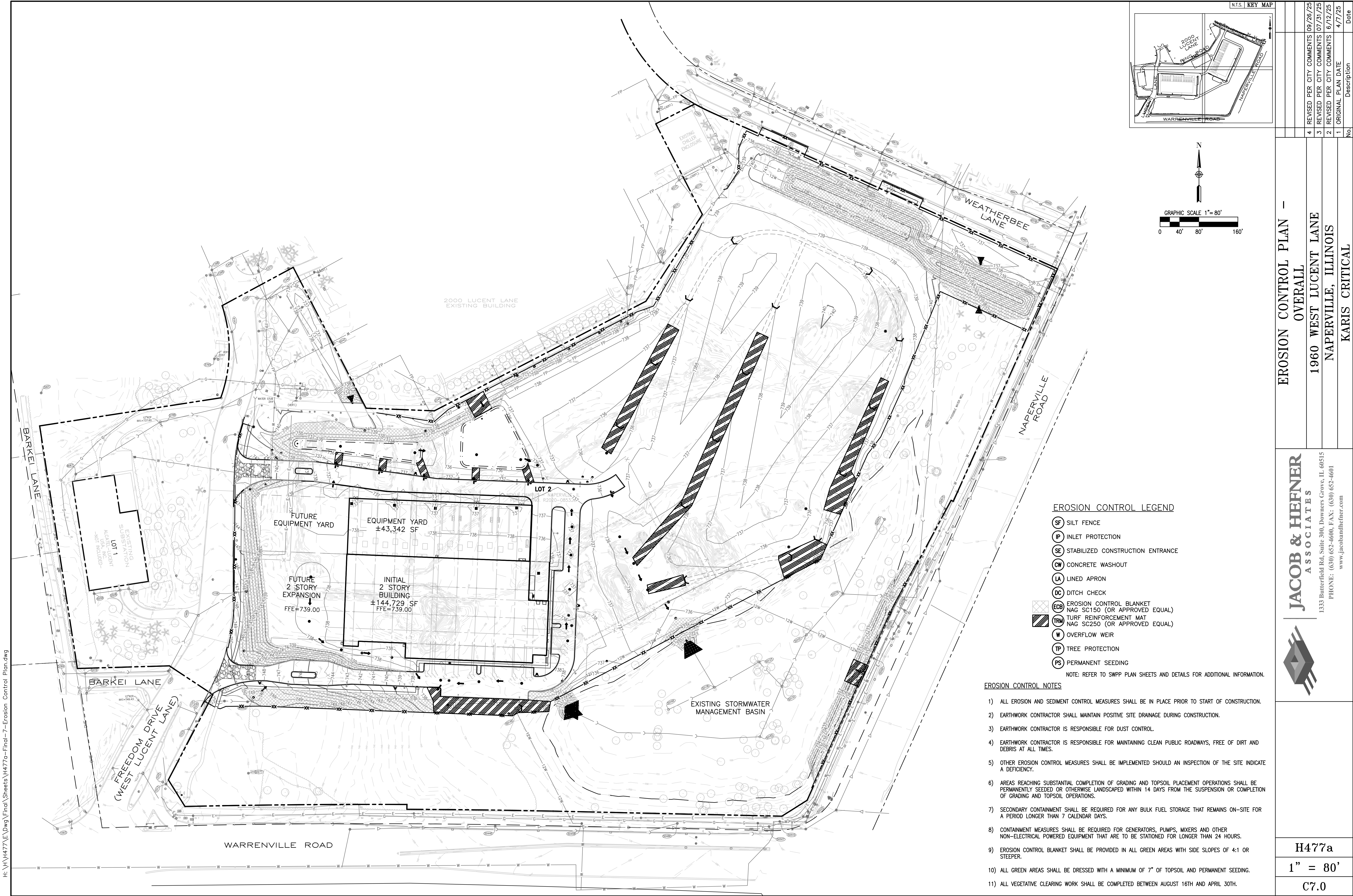
PROFILE LEGEND	
	PROPOSED GROUND SURFACE
	EXISTING GROUND SURFACE
	STORM SEWER
	SANITARY SEWER

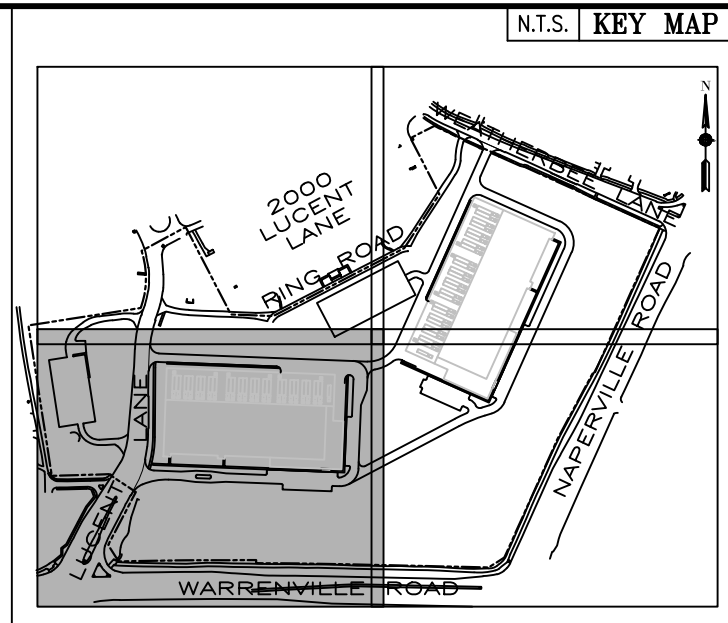
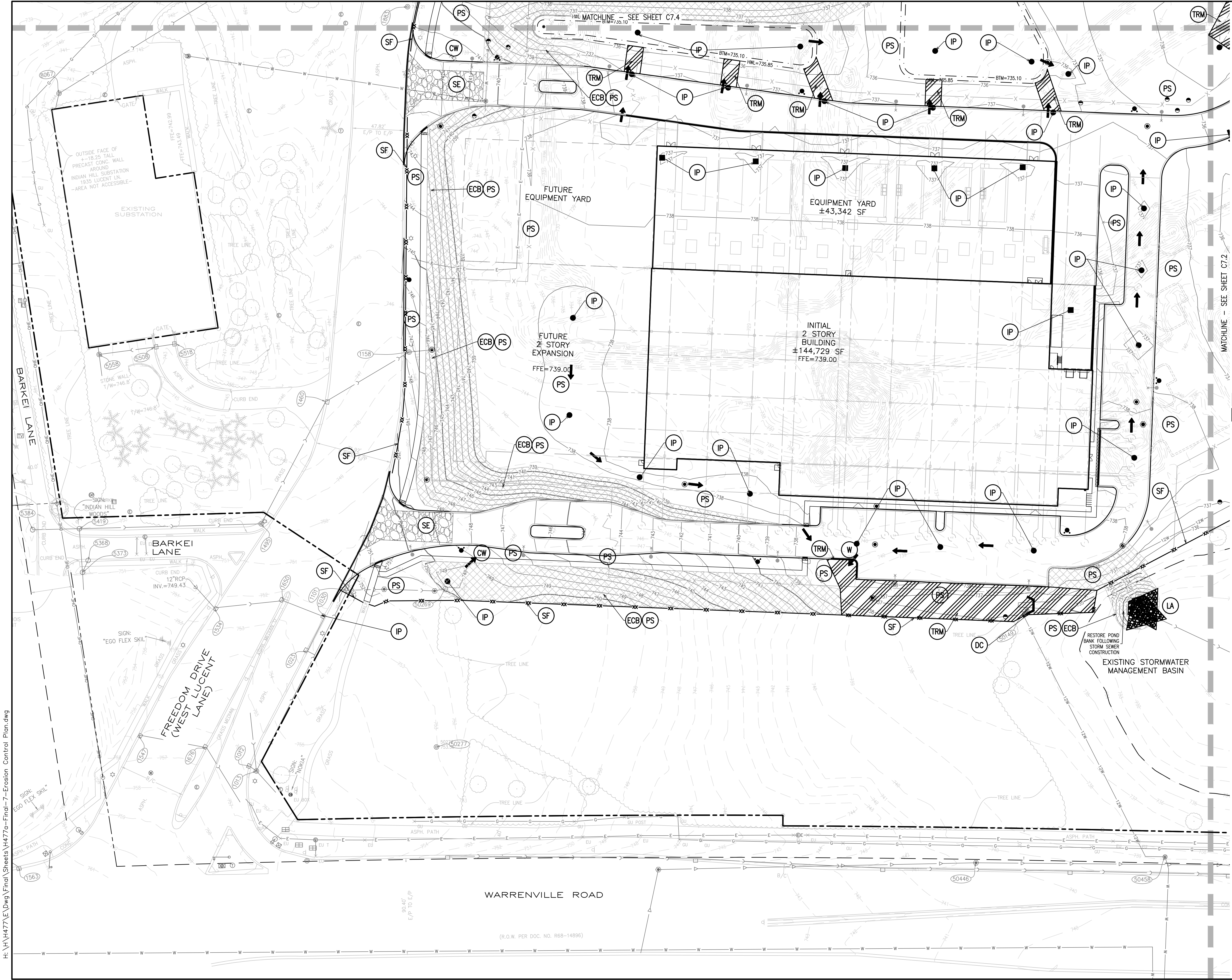


SANITARY PLAN & PROFILE		No.	Description	Date
1960 WEST LUCENT LANE		4	REVISED PER CITY COMMENTS	09/26/25
NAPERVILLE, ILLINOIS		3	REVISED PER CITY COMMENTS	07/31/25
KARIS CRITICAL		2	REVISED PER CITY COMMENTS	6/12/25
		1	ORIGINAL PLAN DATE	4/7/25

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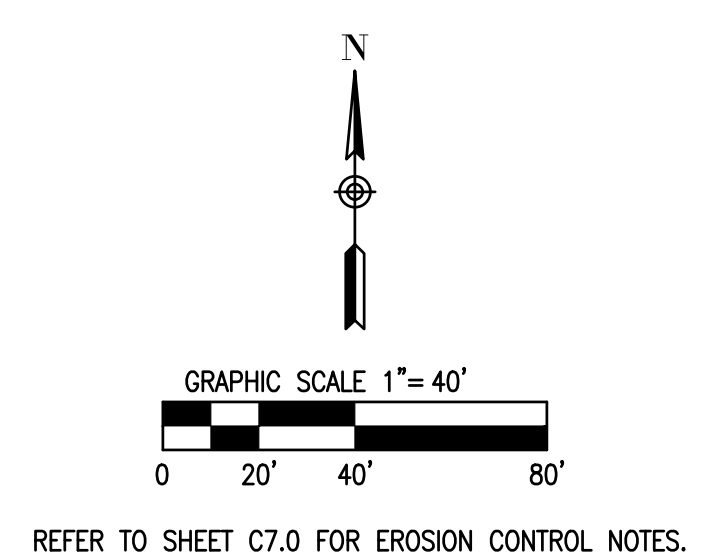
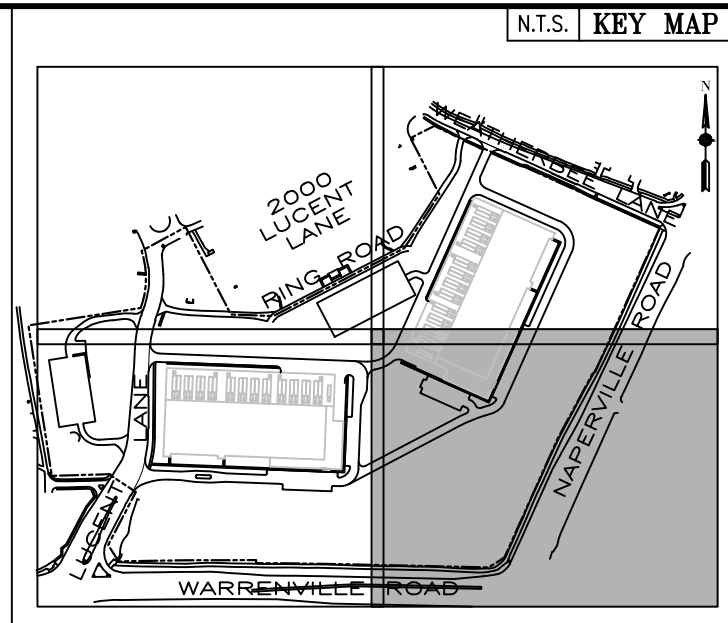
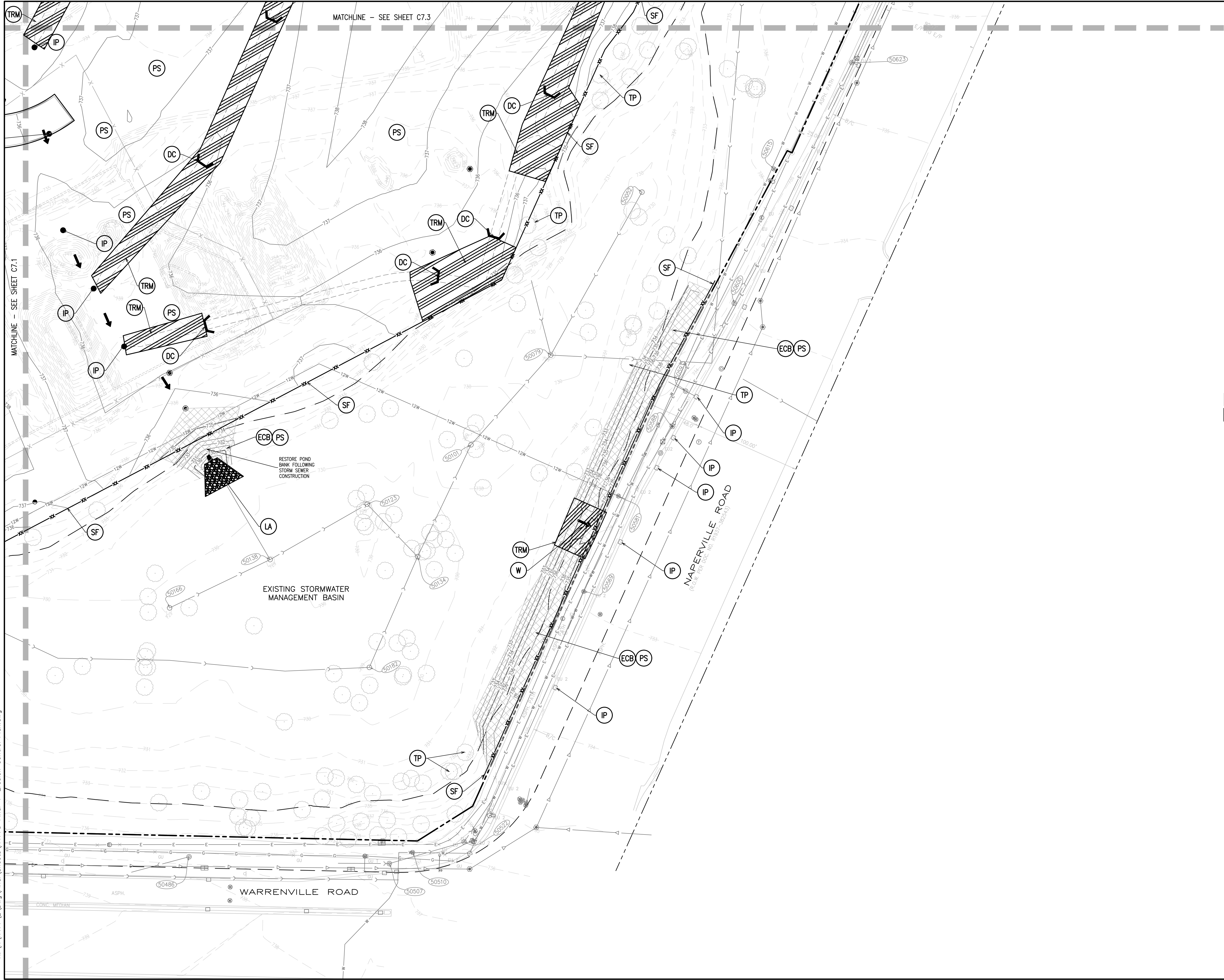




- REFER TO SHEET C7.0 FOR EROSION CONTROL NOTES.
- EROSION CONTROL LEGEND**
- (SF) SILT FENCE
 - (IP) INLET PROTECTION
 - (SE) STABILIZED CONSTRUCTION ENTRANCE
 - (CW) CONCRETE WASHOUT
 - (LA) LINED APRON
 - (DC) DITCH CHECK
 - (ECB) EROSION CONTROL BLANKET
NAG SC150 (OR APPROVED EQUAL)
 - (TRM) TURF REINFORCEMENT MAT
NAG SC250 (OR APPROVED EQUAL)
 - (W) OVERFLOW WEIR
 - (TP) TREE PROTECTION
 - (PS) PERMANENT SEEDING
- NOTE: REFER TO SWPP PLAN SHEETS AND DETAILS FOR ADDITIONAL INFORMATION.

EROSION CONTROL PLAN - SOUTHWEST	
1960 WEST LUCENT LANE NAPERVILLE, ILLINOIS	
KARIS CRITICAL	
JACOB & HEFNER ASSOCIATES	
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H477a	
1" = 40'	
C7.1	
No.	Description
4	REVISED PER CITY COMMENTS 09/26/25
3	REVISED PER CITY COMMENTS 07/31/25
2	REVISED PER CITY COMMENTS 6/12/25
1	ORIGINAL PLAN DATE 4/7/25
No.	Date

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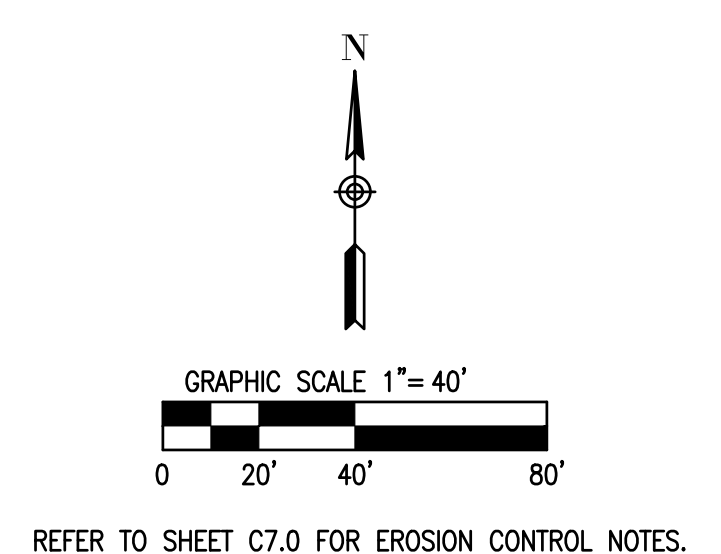
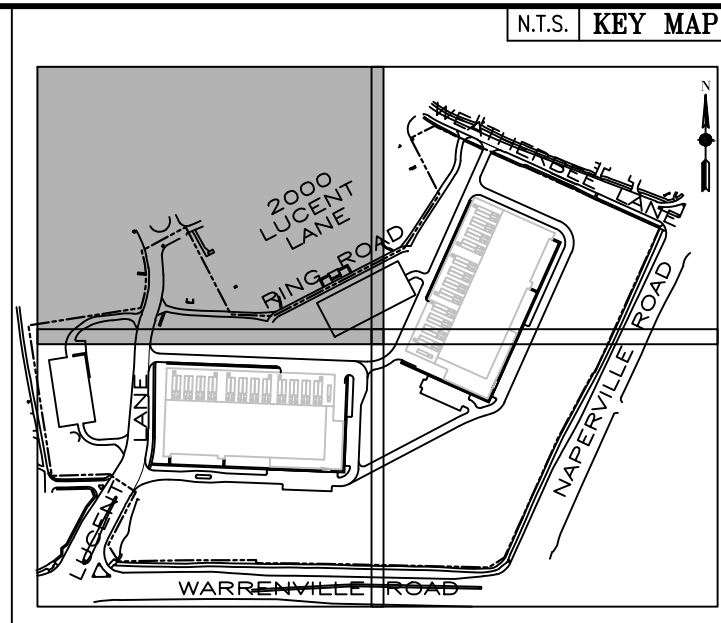
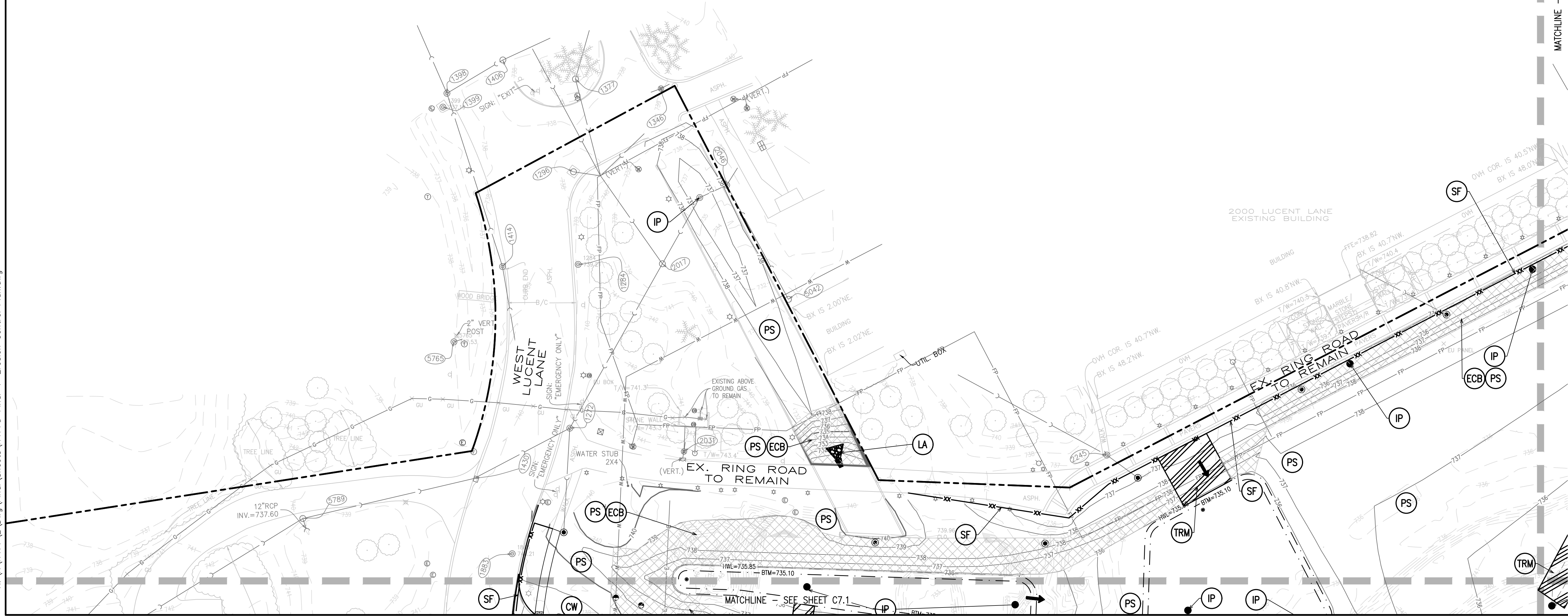
- REFER TO SHEET C7.0 FOR EROSION CONTROL NOTES.
- EROSION CONTROL LEGEND**
- (SF) SILT FENCE
 - (IP) INLET PROTECTION
 - (SE) STABILIZED CONSTRUCTION ENTRANCE
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NAG SC150 (OR APPROVED EQUAL)
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 - (W) OVERFLOW WEIR
 - (TP) TREE PROTECTION
 - (PS) PERMANENT SEEDING
- NOTE: REFER TO SWPP PLAN SHEETS AND DETAILS FOR ADDITIONAL INFORMATION.

EROSION CONTROL PLAN - SOUTHEAST					
1960 WEST LUCENT LANE				4	REVISED PER CITY COMMENTS 09/26/25
NAPERVILLE, ILLINOIS				3	REVISED PER CITY COMMENTS 07/31/25
KARIS CRITICAL				2	REVISED PER CITY COMMENTS 6/12/25
				1	ORIGINAL PLAN DATE 4/7/25
				No.	Description
					Date
H477a					
1" = 40'					
C7.2					

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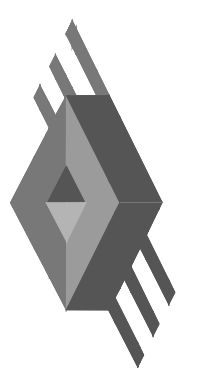
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- EROSION CONTROL LEGEND
- (SF) SILT FENCE
 - (IP) INLET PROTECTION
 - (SE) STABILIZED CONSTRUCTION ENTRANCE
 - (CW) CONCRETE WASHOUT
 - (LA) LINED APRON
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 - (TRM) TURF REINFORCEMENT MAT
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 - (W) OVERFLOW WEIR
 - (TP) TREE PROTECTION
 - (PS) PERMANENT SEEDING
- NOTE: REFER TO SWPP PLAN SHEETS AND DETAILS FOR ADDITIONAL INFORMATION.

MATCHLINE -- SEE SHEET C7.3

EROSION CONTROL PLAN -- NORTHWEST		09/26/25	
1960 WEST LUCENT LANE		07/31/25	
NAPERVILLE, ILLINOIS		6/12/25	
KARIS CRITICAL		4/7/25	
No.	Description	Date	



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H477a
1" = 40'
C7.4

CONTROL MEASURE GROUP	CONTROL MEASURE	APPL	KEY	CONTROL MEASURE CHARACTERISTICS	TEMP.	PERMIT	MAINTENANCE FREQUENCY
VEGETATIVE SOIL COVER	TEMPORARY SEEDING	X	TS	PROVIDES QUICK TEMPORARY COVER TO CONTROL EROSION WHEN PERMANENT SEEDING IS NOT DESIRED OR TIME OF YEAR IS INAPPROPRIATE.	X		REDO ANY FAILING AREAS.
	PERMANENT SEEDING	X	PS	PROVIDES PERMANENT VEGETATIVE COVER TO CONTROL EROSION, FILTERS SEDIMENT FROM WATER. MAY BE PART OF FINAL LANDSCAPE PLAN.		X	REDO ANY FAILING AREAS.
	DORMANT SEEDING		DS	SAME AS PERMANENT SEEDING EXCEPT IS DONE DURING DORMANT SEASON. HIGHER RATES OF SEED APPLICATION ARE REQUIRED.	X	X	RE-SEED IF NEEDED.
	SODDING		SO	QUICK PERMANENT COVER TO CONTROL EROSION. QUICK WAY TO ESTABLISH VEGETATION FILTER STRIP. CAN BE USED ON STEEP SLOPES OR IN DRAINAGE WAYS WHERE SEEDING MAY BE DIFFICULT.	X	X	N/A
NON VEGETATIVE SOIL COVER	MACHINE TRACKING		CC	PROVIDES SOIL ROUGHING FOR EROSION CONTROL.		X	N/A
	POLYMER		P	ADDS INSURANCE OF A SUCCESSFUL TEMPORARY OR PERMANENT SEEDING. PROVIDES TEMPORARY COVER WHERE VEGETATION CANNOT BE ESTABLISHED.	X	X	REAPPLY EVERY 1½ MONTHS.
	AGGREGATE COVER		AG	PROVIDES SOIL COVER ON ROADS AND PARKING LOTS AND AREAS WHERE VEGETATION CANNOT BE ESTABLISHED. PREVENTS MUD FROM BEING PICKED UP AND TRANSPORTED OFF-SITE.	X	X	CLEAN UP DIRT FROM STONE AS NEEDED.
	PAVING	X	PV	PROVIDES PERMANENT COVER ON PARKING LOTS AND ROADS OR OTHER AREAS WHERE VEGETATION CANNOT BE ESTABLISHED.		X	N/A
DIVERSIONS	RIDGE DIVERSION		RD	TYPICALLY USED ABOVE SLOPES TO COLLECT FLOW AND TRANSFER DOWNSLOPE.	X	X	CLEAN SILT OUT WHEN HALF-FULL.
	CHANNEL DIVERSION		CD	TYPICALLY USED TO DIVERT FLOW.	X	X	REPLACE PROTECTION WHEN NEEDED.
	COMBINATION DIVERSION		DIC	TYPICALLY USED ANYWHERE ON A SLOPE. SOIL TAKEN OUT OF CHANNEL IS USED TO BUILD THE RIDGE.	X	X	REPLACE PROTECTION WHEN NEEDED.
	CURB AND GUTTER	X	CG	SPECIAL CASE OF DIVERSION USED IN CONJUNCTION WITH A STREET TO DIVERT WATER FROM AN AREA NEEDING PROTECTION.		X	N/A
	BENCHES		B	SPECIAL CASE OF DIVERSION CONSTRUCTED WHEN WORKING ON CUT SLOPES TO SHORTEN LENGTH OF SLOPE AND ADD SLOPE STABILITY.	X	X	N/A
WATERWAYS	VEGETATIVE CHANNEL		VC	PROVIDED ADDED STABILITY TO CHANNEL. USED WHEN VELOCITY OF FLOW IS NOT EXTREMELY FAST.	X	X	REDO ANY FAILING AREAS.
	LINED CHANNEL		LC	USED WHEN VEGETATION WILL NOT PROTECT THE CHANNEL AGAINST HIGH VELOCITIES OF FLOW OR WHERE VEGETATION CANNOT BE ESTABLISHED.	X	X	REPLACE PROTECTION WHEN NEEDED.
ENCLOSED DRAINAGE	STORM SEWER	X	STM	CAN BE USED TO CONVEY SEDIMENT LADEN WATER TO SEDIMENT BASIN OR IN CONJUNCTION WITH A WATERWAY.		X	CLEAN SEDIMENT OUT.
	UNDER DRAIN	X	UD	USED TO LOWER WATER TABLE AND INTERCEPT GROUNDWATER FOR BETTER VEGETATION GROWTH AND SLOPE STABILITY. USED TO CARRY BASE FLOW IN WATERWAYS AND TO DETAIR SEDIMENT BASINS.	X	X	N/A
SPILLWAYS	STRAIGHT PIPE SPILLWAY		SPS	USED FOR RELATIVELY SMALL VERTICAL DROPS AND SMALL FLOWS OF WATER.		X	CLEAN OUT CONSTRUCTION DEBRIS.
	DROP INLET PIPE SPILLWAY		DBS	SAME AS PIPE SPILLWAY EXCEPT LARGER FLOWS AND LARGE VERTICAL DROPS CAN BE ACCOMMODATED.		X	CLEAN OUT CONSTRUCTION DEBRIS.
	WEIR SPILLWAY	X	W	USED FOR RELATIVELY SMALL VERTICAL DROPS AND FLOWS MUCH GREATER THAN PIPE STRUCTURES.	X	X	CLEAN OUT CONSTRUCTION DEBRIS.
	BOX INLET WEIR SPILLWAY		BS	SAME AS WEIR SPILLWAY EXCEPT LARGER FLOWS CAN BE ACCOMMODATED BECAUSE OF LOWER WEIR LENGTH.	X	X	CLEAN OUT CONSTRUCTION DEBRIS.
OUTLETS	LINED APRON	X	LA	PROTECTS DOWNSTREAM CHANNEL FROM HIGH VELOCITY OF FLOW DISCHARGING FROM STRUCTURES.	X	X	REPAIR DISLOODED STONES OR EROSION UNDER RIP-RAP AS NEEDED.
SEDIMENT BASINS	SEDIMENT BASIN		SB	USED TO COLLECT SMALLER PARTICLES - DETAIN WATER WITH CONTROLLED RELEASE.	X	X	CLEAN SEDIMENT OUT WHEN HALF-FULL.
	SEDIMENT TRAP		ST	USED TO COLLECT LARGER PARTICLES - DETAIN WATER WITH CONTROLLED RELEASE.	X	X	CLEAN SEDIMENT OUT WHEN HALF-FULL.
SEDIMENT FILTERS	SILT FENCE	X	SF	USED FOR SINGLE LOTS OR DRAINAGE AREAS LESS THAN 1/2 ACRE TO FILTER SEDIMENT FROM RUNOFF.	X		CLEAN SEDIMENT OUT WHEN SILT IS HALF-FULL. REPAIR ANY DAMAGED SILT FENCE WHEN NEEDED.
	VEGETATIVE FILTER		VF	USED ALONG DRAINAGE WAYS OR PROPERTY LINES TO FILTER SEDIMENT FROM RUNOFF. SIZE MUST BE INCREASED IN PROPORTION TO DRAINAGE AREA.	X	X	REDO ANY FAILING AREAS.
MUD AND DUST CONTROL	STABILIZED CONST. ENTRANCE	X	SE	PREVENT MUD FROM BEING PICKED UP AND CARRIED OFF-SITE.	X	X	SCRAPE MUD AND REPLACE STONE AS NEEDED.
	DUST CONTROL	X	DT	PREVENTS DUST FROM LEAVING CONSTRUCTION SITE.	X	X	RE-APPLY AS NEEDED.
EROSION CONTROL	EROSION CONTROL BLANKET	X	ECB	PROTECTS SOIL, SEED AND HELPS GROW VEGETATION.			REPLACE AS NEEDED.
	TURF REINFORCEMENT MAT	X	TRM	REINFORCES TURF IN CHANNELS AND SHORELINES.			REPLACE AS NEEDED.
	CELLULAR CONFINEMENT		CF	USED TO HOLED TOPSOIL ON STEEP SLOPES.			REPLACE AS NEEDED.
	GABIONS		GA	USED TO PREVENT EROSION IN VERY HIGH FLOW AREAS.			REPLACE AS NEEDED.
	GEOTEXTILE FABRIC		GF	USED FOR EROSION / SEDIMENT CONTROL/ SEPARATION / STABILIZATION.			REPLACE AS NEEDED.
	GEOBLOCK POROUS PAVEMENT		PP	USED FOR FIRE LANE ACCESS / VEGETATIVE PAVEMENT.			REPLACE AS NEEDED.
SEDIMENT CONTROL	INLET PROTECTION	X	IP	USED FOR PROTECTION OF INLETS.			REPLACE OR CLEAN WHEN CLOGGED.
	SLOPE INTERRUPT		SI	USED TO BREAK UP THE FLOW ON A SLOPE.			CLEAN OUT WHEN HALF-FULL OF SILT.
	DITCH CHECK	X	DC	USED FOR FLOW SEDIMENT CONTROL IN SWALES AND CHANNELS.			CLEAN OUT WHEN HALF-FULL OF SILT.
	FLOC LOG		FL	USED TO CLARIFY WATER THAT HAS SEDIMENT IN THE WATERY COLUMN.			REPLACE WHEN HALF DISSOLVED.
	SILT CURTAIN		SC	USED FOR SEDIMENT CONTROL IN STREAM / POND.			REPLACE WHEN FABRIC IS TORN OR HOLES BEGIN TO FORM.
	PUMPING DISCHARGE BAG		PB	USED FOR PUMP DISCHARGE LINES.			REPLACE WHEN HALF-FULL. FABRIC IS TORN, OR HOLES BEGIN TO FORM.
	CONCRETE WASHOUT	X	CW	FOR CONCRETE TRUCKS TO WASHOUT.			CLEAN OUT WHEN HALF-FULL. CLEAN WASHOUT GRAVEL AREA AS NEEDED.
	STREET SWEEPING	X	SS	USED TO PREVENT SILT BUILD UP IN STREETS.			CLEAN ONCE A WEEK, OR AS NEEDED TO KEEP STREET CLEAN.

SEEDING CHART

STABILIZATION TYPE	JAN.	FEB.	MAR.	APR.	MAY	JUNE	JULY	AUG.	SEPT.	OCT.	NOV.	DEC.
PERMANENT SEEDING			A			•	•					
DORMANT SEEDING	B									B		
TEMPORARY SEEDING			C				D					
SODDING			E**									
MULCHING	F											

- A KENTUCKY BLUEGRASS 90 LBS/ACRE MIXED WITH PERENNIAL RYEGRASS 30 LBS/ACRE.

C SPRING OATS 100 LBS/ACRE
- B KENTUCKY BLUEGRASS 135 LBS/ACRE MIXED WITH PERENNIAL RYEGRASS 45 LBS/ACRE + 2 TONS STRAW MULCH/ACRE.

D WHEAT OR CEREAL RYE 150 LBS/ACRE.
- E SOD

F STRAW MULCH 2 TONS/ACRE.

- * IRRIGATION NEEDED DURING JUNE AND JULY.
- ** IRRIGATION NEEDED FOR 2 TO 3 WEEKS AFTER APPLYING SOD.
- *** MOW LAWNS AS NECESSARY

OBSERVATION & MAINTENANCE SCHEDULE		
ACTIVITY	RESPONSIBLE PARTY	DURATION
STABILIZATION DURING CONSTRUCTION-MAINTENANCE	CONTRACTOR	DURING CONSTRUCTION
STABILIZATION DURING CONSTRUCTION-OBSERVATION	DEVELOPER/OWNER	WEEKLY & AFTER EACH RAINFALL EVENT IN EXCESS OF 0.5".
VEGETATION MAINTENANCE	CONTRACTOR	1 YEAR FROM COMPLETION
VEGETATION STABILIZATION MAINTENANCE	DEVELOPER/OWNER	ONGOING FROM CONSTRUCTION COMPLETION

NOTES:

This plan has been prepared to comply with the provisions of the NPDES Permit Number ILR10, issued by the Illinois Environmental Protection Agency for Stormwater Discharges from Construction Site Activities and Soil Erosion and Sediment Control Ordinance for the County.

1. Site Description.

- a. The following is a description of the construction activity following mass grading which is the subject of this plan:
- The proposed development consists of construction of Industrial Development
- The construction activities for site improvements will include:
- mass grading, pavement construction, installation of utilities including storm sewers, soil erosion and sedimentation control measures, as a minimum.
- b. The following is a description of the intended sequence of major activities which will disturb soils for major portions of the construction site, such as grubbing, excavation, and grading:
- The sequence of the construction activities may be as follows: 1) install silt filter fence and stabilized construction entrance, 2) mass grading, 3) underground utilities installation, 4) fine grading in pavement area and 5) pavement construction. The soil erosion and sedimentation control items will be constructed as needed during the above construction activities.
- c. The total area of the construction site is estimated to be 41.43 acres.
- The total area of the site that it is estimated to be disturbed by excavation, grading, or other activities, is 423.94 acres.
- d. The estimated runoff coefficients of the various areas of the site after construction activities are completed are contained in the project drainage study, titled Stormwater Management for 1960 West Lucent Lane prepared by Jacob & Hefner Associates, Inc. which is hereby incorporated by reference in this plan. The estimated proposed overall site runoff coefficient is _____.
- e. Existing data describing soils or quality of discharge (soils report if available) Fair/Poor/Good/Not Available
- f. Name of receiving water(s): Eastern Branch of the DuPage River
- g. Name of ultimate receiving water(s): DuPage River
- h. Wetland acreage: 0.00 acre

2. Controls.

- This section of the plan addresses the various controls that will be implemented for each of the major construction activities described in 1.b. above. For each measure discussed, the contractors will be responsible for its implementation as indicated. Each such contractor has signed the required certification on forms which are attached to, and are a part of, this plan.
- a. Erosion and Sediment Controls.
- (i) STABILIZATION PRACTICES. Provided below is a description of interim and permanent stabilization practices, including site-specific scheduling of the implementation of the practices. Site plans will ensure that existing vegetation is preserved where attainable and disturbed portions of the site will be stabilized. Except as provided in 2.a.(i)(A) and 2.b., stabilization measures shall be initiated within 1 working day in portions of the site where construction activities have temporarily or permanently ceased and must be completed within 14 calendar days after the construction activity in that portion of the site has ceased on all disturbed portions of the site where construction activity will not occur for a period of 14 or more calendar days.
- Where the initiation of stabilization measures after construction activity temporarily or permanently ceases is precluded by snow cover, stabilization measures shall be initiated as soon as practicable thereafter.
- The following interim and permanent stabilization practices, as a minimum, will be implemented to stabilize the disturbed area of the site:
1. Permanent seeding
 2. Silt filter fence
 3. Vegetative Filter
 4. Stabilized construction entrance
 5. Barrier filter
- (ii) STRUCTURAL PRACTICES. Provided below is a description of structural practices that will be implemented, to the degree attainable, to divert flows from exposed soils, store flows or otherwise limit runoff and the discharge of pollutants from exposed areas of the site. The installation of these devices may be subject to Section 404 of the Clean Water Act.
1. Detention Pond
 2. Storm sewer system
 3. Rip-rap for outlet protection
 4. Permanent seeding
- (iii) DUST CONTROL: Dust Control shall be provided per standard 825 of Illinois Urban Manual. Following are the dust control that can be used.
1. Irrigation
 2. Spray on adhesive
 3. Vegetative cover
 4. Mulching

- b. Stormwater Management.
- (i) Provided below is a description of measures that will be installed during the construction process to control pollutants in stormwater discharges that will occur after construction operations have been completed. The installation of these devices may be subject to Section 404 of the Clean Water Act.
- The practices selected for implementation were determined on the basis of the technical guidance contained in IEPA's Standard Specifications for Soil Erosion and Sedimentation Control, and other ordinances listed in the Specifications.
- The stormwater pollutant control measures shall include:
1. Silt filter fence
 2. Barrier filters
 3. Storm sewers
 4. Retention/Detention ponds
- (ii) Velocity dissipation devices will be placed at discharge locations and along the length of any outfall channel as necessary to provide a non-erosive velocity flow from the structure to a water course so that the natural physical and biological characteristics and functions are maintained and protected (e.g., maintenance of hydrologic conditions, such as the hydropereiod and hydrodynamics present prior to the initiation of construction activities).
- Stormwater Management Control includes:
1. Rip-rap for outlet protection
 2. Ditch check
- c. Other Controls.
- (i) Waste Disposal. The solid waste materials including trash, construction debris, excess construction materials, machinery, tools and other items will be collected and disposed off-site by the contractor. The contractor is responsible to acquire any permit required for such disposal. Burning on the site will not be permitted. No solid materials, including building materials, shall be discharged into Waters of the State, except as authorized by a Section 404 permit.
- (ii) The provisions of this plan shall ensure and demonstrate compliance with applicable State and/or local waste disposal, sanitary sewer or septic system regulations.
- The sanitary sewage will be discharged to the proposed sanitary sewer constructed per IEPA and local standards.
- d. Approved State or Local Plans.
- The management practices, controls, and other provisions contained in this plan are at least as protective as the requirements contained in the Illinois Environmental Protection Agency's Standards and Specifications for Soil Erosion and Sediment Control dated October 1987, Illinois Procedures and Standards for Urban Soil Erosion and Sedimentation Plan, and the Municipal Subdivision Ordinance. Requirements specified in sediment and erosion control site plans or site permits or stormwater management site plans or site permits approved by local officials that are applicable to protecting surface water resources are, upon submittal of an NOI to be authorized to discharge under this permit, incorporated by reference and are enforceable under this permit even if they are not specifically included in the plan.

3. Maintenance
- The following is a description of procedures that will be used to maintain, in good and effective operating conditions, vegetation, erosion and sediment control measures and other protective measures identified in this plan and Standard Specifications.
- Stabilized construction entrance: The entrance shall be maintained to prevent tracking of sediment onto public streets. This will be done by top dressing with additional stones, remove and replace top layer of stones or washing the entrance. The sediment washed on the public right-of-way will be removed immediately.
- Vegetative erosion control measures: The vegetative growth of temporary and permanent seeding, sodding, vegetative channels, vegetative filter, etc. shall be maintained periodically and supply adequate watering and fertilizer. The vegetative cover shall be removed and reseeded as necessary.
- Sedimentation basins/traps: The sediments shall be removed when 40-50 percent of the total original capacity is occupied by the sediment. In no case shall the sediment be built up to more than 1 foot below the crest elevation. At this stage, the basin shall be cleaned out and returned to its original volume.
- Silt filter fence: The damaged silt filter fence shall be restored to meet the standards or removed and replaced as needed.
- Straw bale barrier filters: The straw bale barrier filter shall be inspected frequently and shall be repaired or removed and replaced as needed.
- Rip-Rap outlet protection: It shall be inspected after high flows for any scour beneath the Rip-Rap or for stones that have been dislodged. It shall be repaired immediately.

4. Inspections

- The Owner, or Owner's representative shall provide qualified personnel to inspect disturbed areas of the construction site which have not been finally stabilized, structural control measures, and location where vehicles enter or exit the site. Such inspections shall be conducted at least once every seven (7) calendar days and within 24 hours of the end of a storm (or by the end of the following business or work day) that consists of 0.5 inches or greater of rainfall or a discharge occurs due to snowmelt.
- a. Disturbed areas and areas used for storage of materials that are exposed to precipitation shall be inspected for evidence of, or the potential for, pollutants entering the drainage system. Erosion and sediment control measures identified in the plan shall be observed to ensure that they are operating correctly. Where discharge locations or points are accessible, they shall be inspected to ascertain whether erosion control measures are effective in preventing significant impacts to receiving waters. Locations where vehicles enter or exit the site shall be inspected for evidence of off site sediment tracking.
- b. Based on the results of the inspection, the description of potential pollutant sources identified in section 1 above and pollution prevention measures identified in section 2 above shall be revised as appropriate as soon as practicable after such inspection. Any changes to this plan resulting from the required inspections shall be implemented within 7 calendar days following the inspection.
- c. A report summarizing the scope of the inspection, name(s) and qualifications of personnel making the inspection, the date(s) of the inspection, major observations relating to the implementation of this stormwater pollution prevention plan, and actions taken in accordance with section 4.b. shall be made and retained as part of the plan for at least three (3) years after the date of the inspection. The report shall be signed in accordance with Part VI.G. of the general permit.
- d. If any violation of the provisions of this plan is identified during the conduct of the construction work covered by this plan, the Resident Engineer or Resident Technician shall complete and file an "Incidence of Noncompliance" (ON) report for the identified violation. The Resident Engineer or Resident Technician shall use forms provided by the Illinois Environmental Protection Agency and shall include specific information on the cause of noncompliance, actions which were taken to prevent any further causes of noncompliance, and a statement detailing any environmental impact which may have resulted from the noncompliance. All reports of noncompliance shall be signed by a responsible authority in accordance with Part VI. G. of the general permit. The report of noncompliance shall be mailed to the following address:
- ALL PACKAGES:
Illinois Environmental Protection Agency
Division of Water Pollution Control
Attn: Compliance Assurance Section
2520 Iles Ave
Springfield, IL 62794-9276

ALL LETTERS:
Illinois Environmental Protection Agency
Division of Water Pollution Control
Attn: Compliance Assurance Section
Post Office Box 19276
Springfield, IL 62794-9276

5. Non-Stormwater Discharge

- Except for flows from fire fighting activities, sources of non-stormwater that may be combined with stormwater discharges associated with the residential activity addressed in this plan, are described below:
1. Water main flushing
 2. Fire hydrant flushing
 3. Watering for dust control
 4. Irrigation drainage for vegetative growth for seeding, etc.
- The pollution prevention measures, as described below, will be implemented for non-stormwater components of the discharge:
- The fire hydrant and water main shall not be flushed directly on the exposed area or subgrade of the pavement. Hoses shall be used to direct the flow into the storm sewer system.
- The erosion due to irrigation of seeding shall be considered minor.

NPDES CERTIFICATE

This certification statement is a part of the Storm Water Pollution Prevention Plan for the project described below, in accordance with NPDES Permit No. ILR00000000, issued by the Environmental Protection Agency on _____

PROJECT TITLE: SITE IMPROVEMENT PLANS FOR 1960 WEST LUCENT LANE

PROJECT LOCATION: NAPERVILLE DUPAGE ILLINOIS
CITY/VILLAGE COUNTY STATE

DEVELOPER: KARIS CRITICAL

I certify under penalty of law that I understand the terms of the general National Pollutant Discharge Elimination System (NPDES) permit that authorizes the storm water discharges associated with industrial activity from the construction site identified as part of this certification.

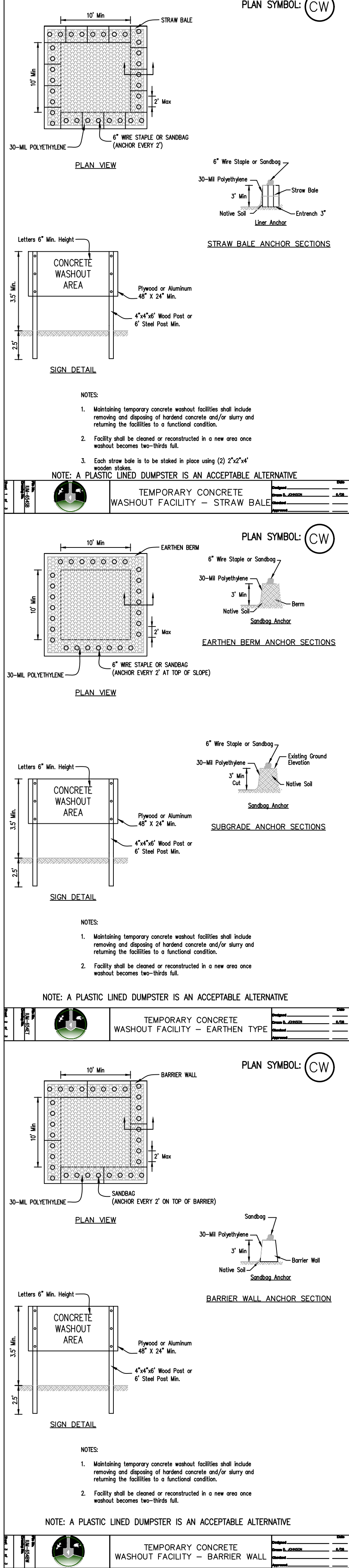
Signature: _____ Date: _____

CONTRACTOR/SUBCONTRACTOR CERTIFICATION STATEMENT

Name: _____ Signature _____

RIP-RAP DIMENSION TABLE

STRUCTURE NUMBER/POND	INLET PIPE SIZE (IN)	DISCHARGE Q (CFS)	LENGTH OF APRON L ₁ (FT)	MEDIAN RIPRAP SIZE (IN)	WIDTH OF APRON U/S FACE 3d (FT)	WIDTH OF APRON D/S FACE 3d+L ₁ (FT)	DEPTH OF RIP Y (IN)	AREA OF RIP RAP (SQ.YDS.)	VOLUME OF RIP RAP (CU.YDS.)
ALL	12	8	(DOT RR 3)	3.00	13.00	15	8.89	3.7	
ALL	15	10	8 (DOT RR 3)	3.75	13.75	15	9.72	4.1	
ALL	18	15	9 (DOT RR 4)	4.50	19.50	20	20.00	11.1	
ALL	21	15	9 (DOT RR 4)	5.25	20.25	20	21.25	11.8	
ALL	24	18	9 (DOT RR 4)	6.00	24.00	20	30.00	16.7	
ALL	27	18	9 (DOT RR 4)	6.75	24.75	20	31.50	17.5	
ALL	30	20	9 (DOT RR 4)	7.50	27.50	20	38.89	21.6	
ALL	36	24	12 (DOT RR 5)	9.00	33.00	28	56.00	43.6	
ALL	42	27	12 (DOT RR 5)	10.5	37.50	30	72.00	60.0	
ALL	48	27	15 (DOT RR 6)	12.0	39.00	32	76.50	68.0	
ALL	54	27	15 (DOT RR 6)	13.5	40.50	32	81.00	72.0	
ALL	60	36	15 (DOT RR 6)	15.0	51.00	32	132.00	118.0	
ALL	72	44	18 (DOT RR 7)	18.0	62.00	32	195.56	174.0	



STORMWATER POLLUTION PREVENTION PLAN

1960 WEST LUCENT LANE

NAPERVILLE, ILLINOIS

KARIS CRITICAL

REVISED PER CITY COMMENTS

09/26/25

REVISED PER CITY COMMENTS

07/31/25

REVISED PER CITY COMMENTS

6/12/25

ORIGINAL PLAN DATE

4/7/25

No.

Description

Date

JACOB & HEFNER ASSOCIATES

1333 Butterfield Rd, Suite 300, Downers Grove, IL 60515

PHONE: (630) 652-4600, FAX: (630) 652-4601

www.jacobandhefner.com

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NOTE: IF SPECIFICATIONS AND/OR DETAILS PROVIDED IN THESE PLANS CONFLICT WITH THE CITY OF NAPERVILLE SPECIFICATIONS AND/OR DETAILS, THE CITY OF NAPERVILLE SHALL SUPERCEDE.

