

Traffic Impact Study Naper Commons Development

Naperville, Illinois



Prepared For:



October 15, 2020

1. Introduction

This report summarizes the methodologies, results, and findings of a traffic impact study conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for a proposed residential development to be located in the northwest quadrant of the intersection of Naperville Road and Warrenville Road within the Nokia campus in Naperville, Illinois. As proposed, the site will be developed with approximately 174 single-family home lots and approximately 70 townhome units. Access to the proposed residential development will be provided via the two existing signalized access drives serving the Nokia Campus off Naperville Road and Warrenville Road.

The purpose of this study was to examine background traffic conditions, assess the impact that the proposed development will have on traffic conditions in the area, and determine if any roadway or access improvements are necessary to accommodate traffic generated by the proposed development.

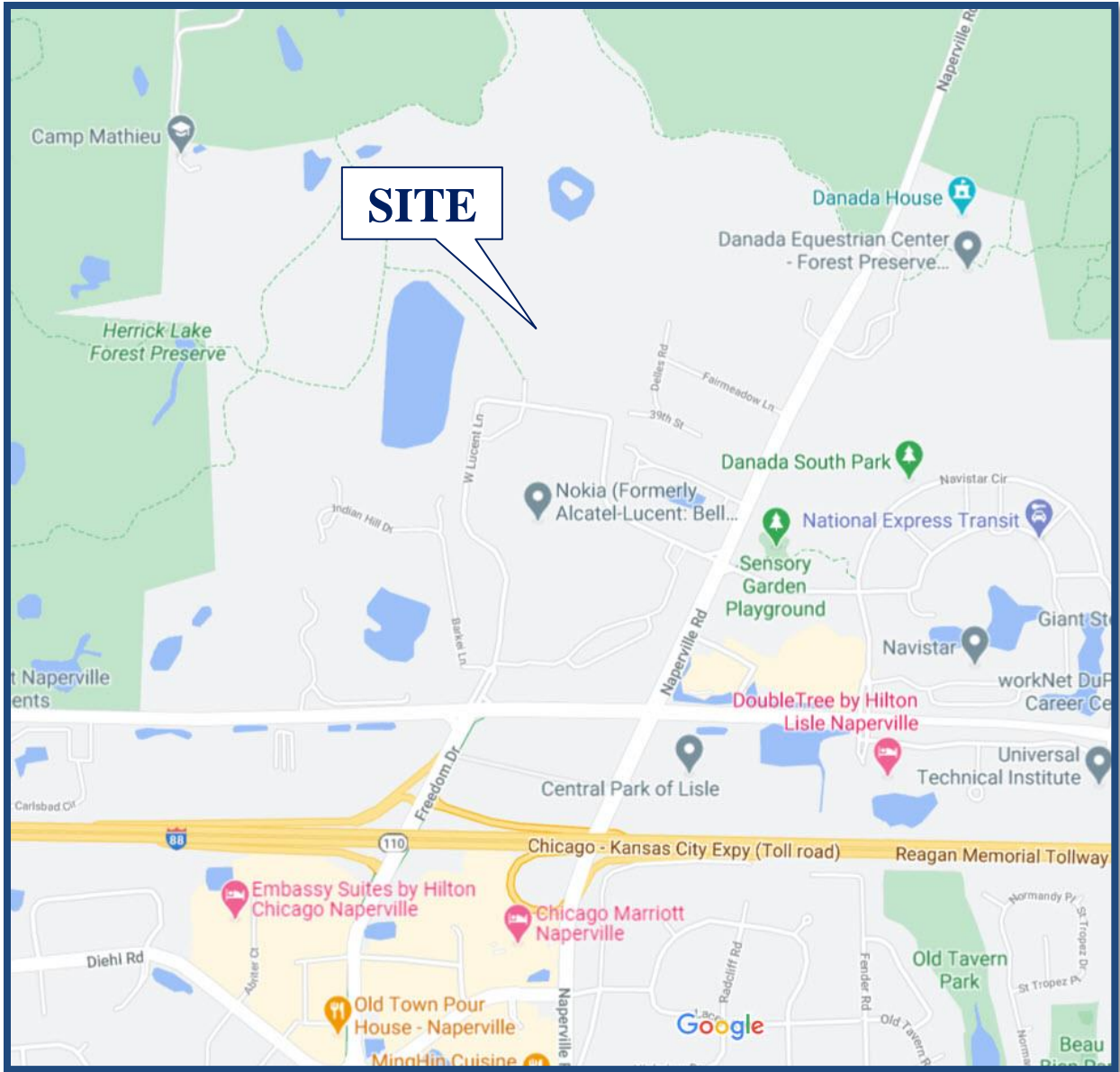
Figure 1 shows the location of the site in relation to the area roadway system. **Figure 2** shows an aerial view of the site area.

The sections of this report present the following:

- Existing roadway conditions
- A description of the proposed development
- Directional distribution of the development traffic
- Vehicle trip generation for the development
- Future traffic conditions including access to the development
- Traffic analyses for the weekday morning and weekday evening peak hours
- Recommendations with respect to adequacy of the site access and adjacent roadway system

Traffic capacity analyses were conducted for the weekday morning and weekday evening peak hours for the following conditions:

1. Existing Conditions - Analyzes the capacity of the existing roadway system using existing peak hour traffic volumes in the surrounding area.
2. No-Build Conditions – Analyzes the projected traffic volumes which includes the existing traffic volumes increased by an ambient area growth factor and the full occupancy of the Nokia campus.
3. Future Conditions – Analyzes the projected traffic volumes which includes the existing traffic volumes increased by an ambient area growth factor (growth not attributable to any particular development), the full occupancy of the Nokia campus and the traffic estimated to be generated by the proposed subject development.



Site Location

Figure 1



Aerial View of Site

Figure 2

2. Existing Conditions

Existing transportation conditions in the vicinity of the site were documented based on field visits conducted by KLOA, Inc. in order to obtain a database for projecting future conditions. The following provides a description of the geographical location of the site, physical characteristics of the area roadway system including lane usage and traffic control devices, and existing peak hour traffic volumes.




Site Location

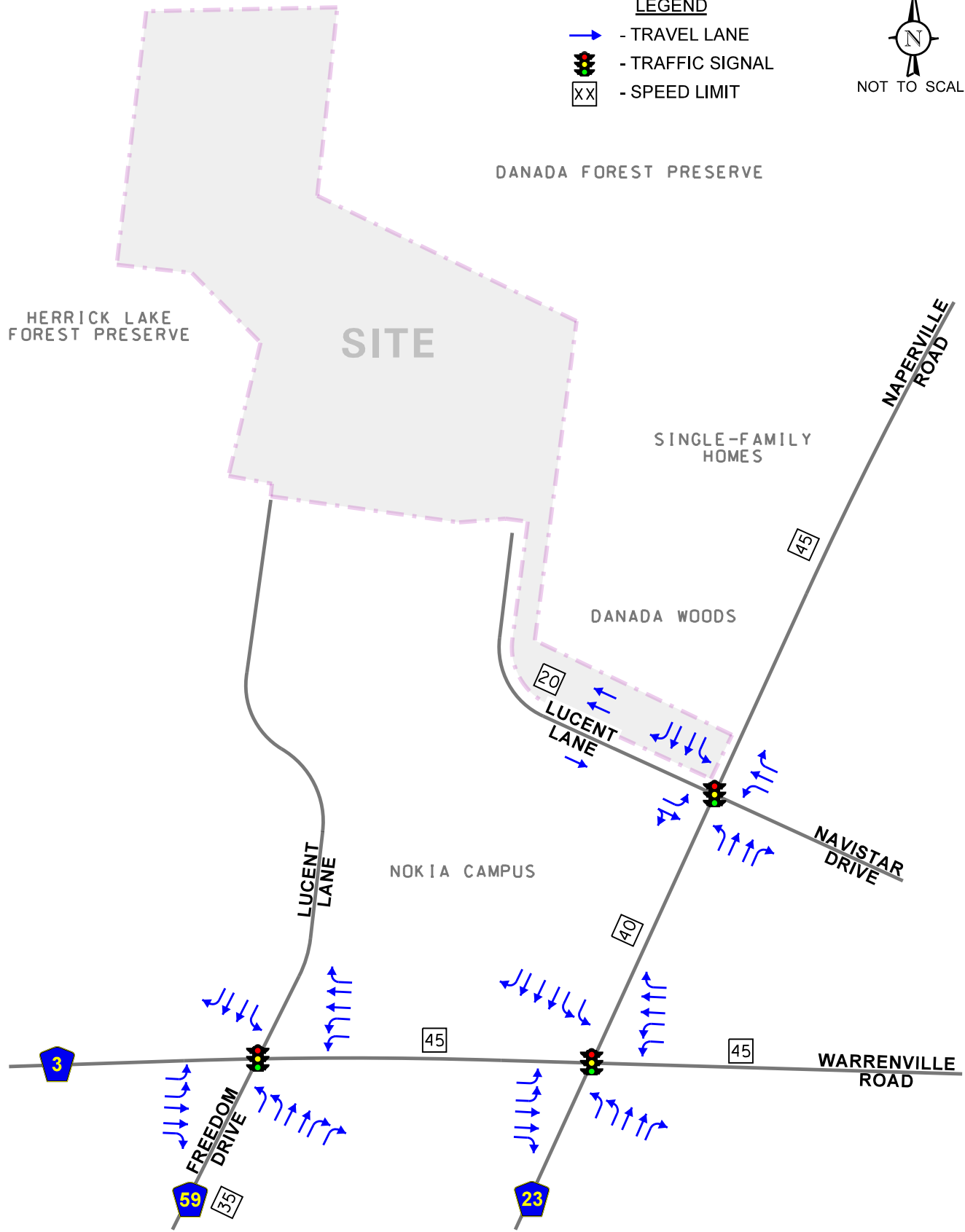
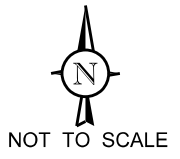
The site, which is located in the northwest quadrant of the intersection of Naperville Road with Warrenville Road within the Nokia campus, is currently occupied by a parking lot and vacant land. Land uses in the vicinity of the site include the Danada Forest Preserve to the north, the Herrick Lake Forest Preserve to the west, the Danada Woods townhome development and single-family homes to the east and the Nokia campus to the south.

Existing Roadway System Characteristics

The characteristics of the existing roadways near the development are described below. **Figure 3** illustrates the existing roadway characteristics.

Naperville Road (County Highway 23) is a north-south arterial roadway that in the vicinity of the site provides three through lanes in the southbound direction and two through lanes in the northbound direction. At its signalized intersection with Warrenville Road, Naperville Road provides dual left-turn lanes, two through lanes and an exclusive right-turn lane on the northbound approach. The southbound approach provides dual left-turn lanes, three through lanes and an exclusive right-turn lane. Naperville Road provides access to I-88 to the east south of Warrenville Road. At its signalized intersection with Lucent Lane/Navistar Drive, Naperville Road provides an exclusive left-turn lane, two through lanes and an exclusive right-turn lane on both approaches. Naperville Road is under the jurisdiction of the DuPage County Division of Transportation (DuDOT), carries an annual average daily traffic (AADT) volume of 33,100 vehicles south of Warrenville Road and 28,000 vehicles north of Warrenville Road (IDOT 2016), and has a posted speed limit of 40 miles per hour increasing to 45 miles per hour north of Lucent Lane/Navistar Drive.

- LEGEND**
-  - TRAVEL LANE
 -  - TRAFFIC SIGNAL
 -  - SPEED LIMIT



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Existing Roadway Characteristics

KLOA
Kenig, Lindgren, O'Hara, Aboona, Inc.
Job No: 20-177 Figure: 3

Warrenville Road (County Highway 3) is an east-west arterial roadway that provides two lanes in each direction divided by a raised median. At its signalized intersections with Naperville Road and Lucent Lane/Freedom Drive, Warrenville Road provides dual left-turn lanes, two through lanes and an exclusive right-turn lane on both approaches. Warrenville Road is under DuDOT's jurisdiction, carries an AADT volume of 16,600 vehicles east of Naperville Road and 20,000 vehicles west of Naperville Road (IDOT 2016), and has a posted speed limit of 45 mph.

Freedom Drive (County Highway 59) is a north-south major collector that extends from Diehl Road north to Warrenville Road where it intersects opposite Lucent Lane. Freedom Drive provides access to I-88 to and from the west and from the east. At its signalized intersection with Warrenville Road, Freedom Drive provides dual left-turn lanes, two through lanes and dual right-turn lanes on the northbound approach. The southbound approach is Lucent Lane and provides an exclusive left-turn lane, two through lanes and an exclusive right-turn lane. Freedom Drive is under DuDOT's jurisdiction, carries an AADT of 12,950 vehicles (IDOT 2016), and has a posted speed limit of 35 mph.

Lucent Lane/Navistar Drive are east-west private roadways serving the Nokia campus to the west and the Navistar campus as well as other businesses to the east. At its signalized intersection with Naperville Road, Lucent Lane provides an exclusive left-turn lane and a shared through/right-turn lane in the eastbound direction while Navistar Drive provides an exclusive left-turn lane, a through lane and an exclusive right-turn lane in the westbound direction.

Existing Traffic Volumes

In order to determine pre Covid-19 pandemic traffic conditions in the vicinity of the site, DuDOT provided KLOA, Inc. with turning movement traffic counts at the intersections of Warrenville Road with Freedom Drive/Lucent Lane and Naperville Road conducted in 2019 and at the intersection of Naperville Road with Lucent Lane/Navistar Drive conducted in 2018. The Naperville Road with Lucent Lane/Navistar Drive was adjusted to reflect 2019 traffic volumes based on the traffic counts at the intersection of Warrenville Road with Naperville Road.

The results of the traffic counts showed that the weekday morning peak hour of traffic occurs from 7:00 to 8:00 A.M. and the weekday evening peak hour of traffic occurs from 4:00 to 5:00 P.M.

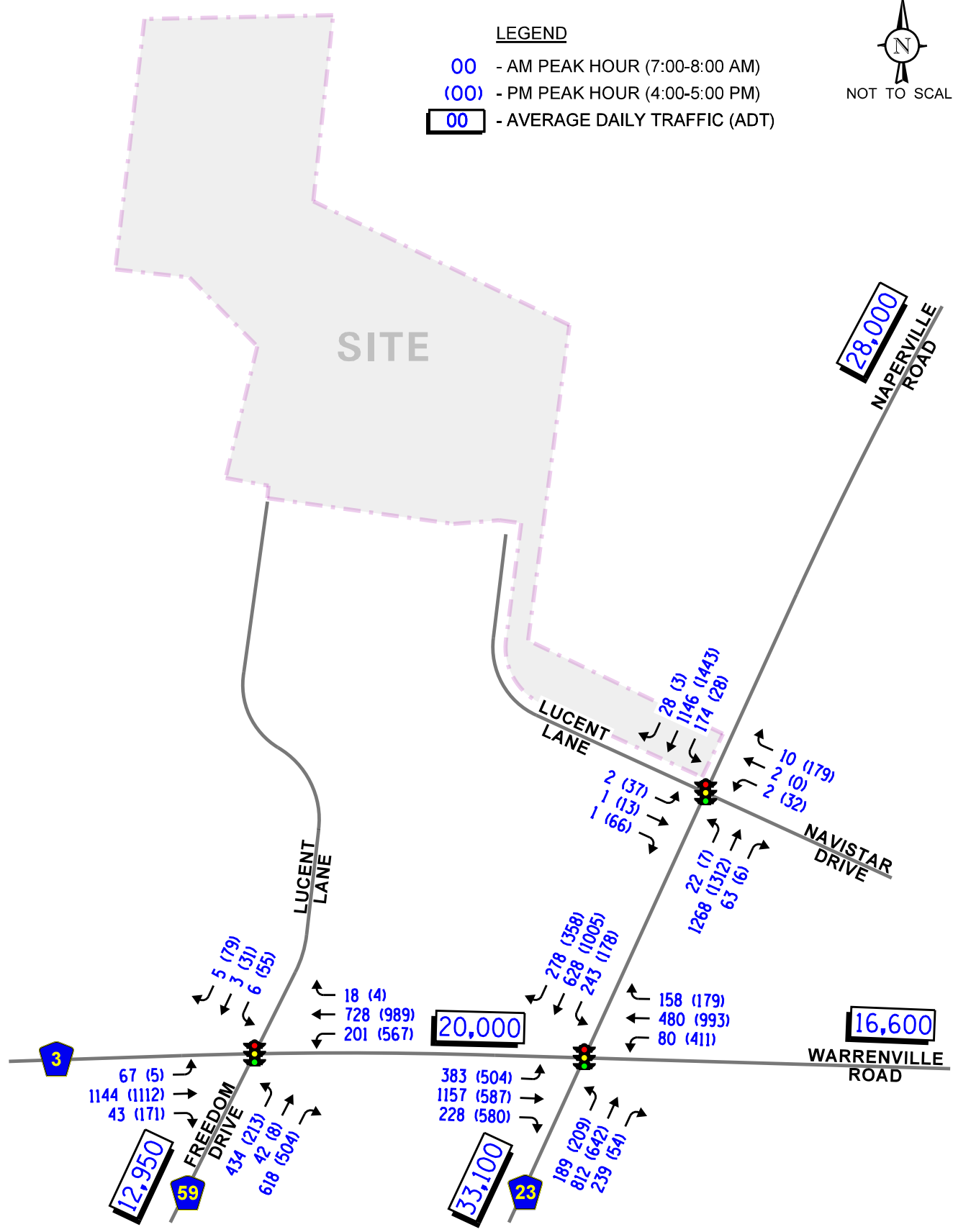
Figure 4 illustrates the existing peak hour traffic volumes. Copies of the traffic count summary sheets are included in the Appendix.



NOT TO SCALE

LEGEND

- 00 - AM PEAK HOUR (7:00-8:00 AM)
- (00) - PM PEAK HOUR (4:00-5:00 PM)
- 00 - AVERAGE DAILY TRAFFIC (ADT)



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Existing Traffic Volumes



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Figure: 4

Crash Analysis

KLOA, Inc. obtained crash data¹ for the past most recent available five years (2014 to 2018) for the study intersections. **Tables 1** through **3** summarize the crash data for the intersections of Naperville Road with Warrenville Road, Naperville Road with Lucent Lane/Navistar Drive, and Warrenville Road with Freedom Drive/Lucent Lane. A review of the crash data indicated that no fatalities were reported at any of the studied intersections.

Table 1

NAPERVILLE ROAD WITH WARRENVILLE ROAD – CRASH SUMMARY

Year	Type of Crash Frequency							Total
	Angle	Head On	Object	Rear End	Sideswipe	Turning	Other	
2014	0	0	0	21	0	5	0	26
2015	3	0	0	19	0	4	0	26
2016	2	0	0	13	3	6	0	24
2017	0	0	0	10	0	4	0	14
2018	<u>1</u>	<u>0</u>	<u>0</u>	<u>13</u>	<u>1</u>	<u>7</u>	<u>0</u>	<u>22</u>
Total	6	0	0	76	4	26	0	112
Average	1.2	0	0	15.2	<1	5.2	0	22.4

¹ IDOT DISCLAIMER: The motor vehicle crash data referenced herein was provided by the Illinois Department of Transportation. Any conclusions drawn from analysis of the aforementioned data are the sole responsibility of the data recipient(s). Additionally, for coding years 2015 to present, the Bureau of Data Collection uses the exact latitude/longitude supplied by the investigating law enforcement agency to locate crashes. Therefore, location data may vary in previous years since data prior to 2015 was physically located by bureau personnel.

Table 2
 NAPERVILLE ROAD WITH LUCENT LANE/NAVISTAR DRIVE – CRASH SUMMARY

Type of Crash Frequency								
Year	Angle	Head On	Object	Rear End	Sideswipe	Turning	Other	Total
2014	0	0	0	1	0	0	0	1
2015	0	0	0	1	0	1	0	2
2016	0	0	0	1	0	0	0	1
2017	0	0	0	0	0	2	0	2
2018	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>
Total	0	0	0	3	0	3	0	6
Average	0	0	0	< 1	0	< 1	0	1.2

Table 3
 WARRENVILLE ROAD WITH FREEDOM DRIVE/LUCENT LANE – CRASH SUMMARY

Type of Crash Frequency								
Year	Angle	Head On	Object	Rear End	Sideswipe	Turning	Other	Total
2014	0	0	0	8	0	2	0	10
2015	0	0	1	7	0	8	0	16
2016	0	1	0	5	0	2	0	8
2017	0	0	0	7	0	1	1	9
2018	<u>0</u>	<u>0</u>	<u>0</u>	<u>7</u>	<u>0</u>	<u>3</u>	<u>0</u>	<u>10</u>
Total	0	1	1	34	0	16	1	53
Average	0	< 1	< 1	6.8	0	3.2	< 1	4.8

3. Traffic Characteristics of the Proposed Development

In order to properly evaluate future traffic conditions in the surrounding area, it was necessary to determine the traffic characteristics of the proposed development, including the directional distribution and volumes of traffic that it will generate.

Proposed Site and Development Plan

As proposed, the plans call for developing the site with 174 single-family home lots and 70 townhomes. Access to the site will be provided via the existing signalized intersections of Lucent Lane with Naperville Road and Lucent Lane with Warrenville Road.

Directional Distribution

The directions from which residents will approach and depart the site were estimated based on existing travel patterns, as determined from the traffic counts. **Figure 5** illustrates the directional distribution of the development-generated traffic.

Estimated Site Traffic Generation

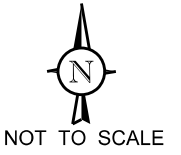
The volume of traffic generated by the proposed development was estimated using data published in the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 10th Edition. The “Single-Family Detached Housing” (Land-Use Code 210) was used for the single-family homes while the “Multi-Family Housing - Low Rise” (Land Use Code 220) was used for the townhomes.

Table 4 tabulates the peak hour vehicle trips anticipated for this development. The ITE *Trip Generation Manual* 10th Edition rates and equations are included in the Appendix.

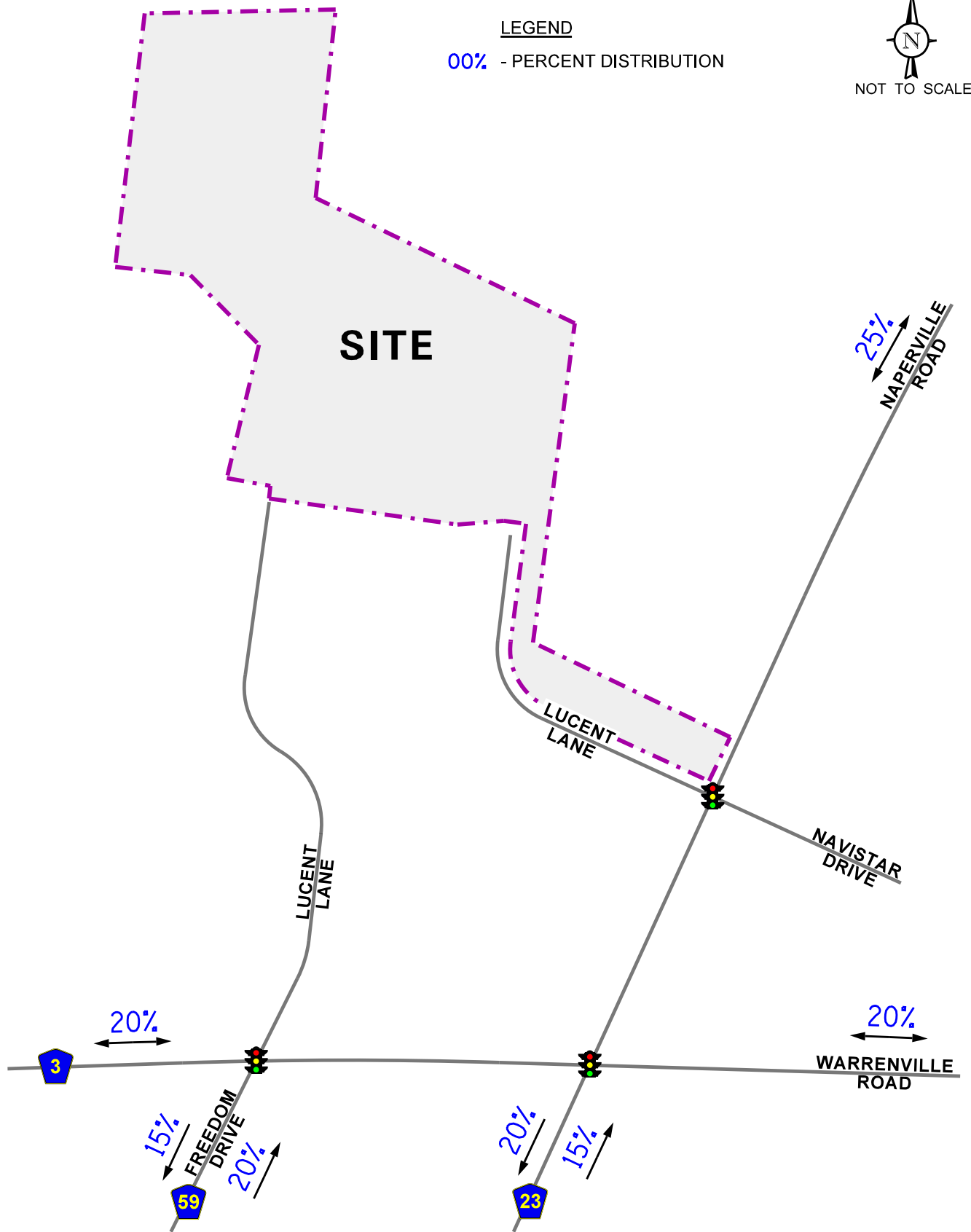
Table 4

ESTIMATED PEAK HOUR SITE-GENERATED TRAFFIC VOLUMES

ITE Land Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Daily		
		In	Out	Total	In	Out	Total	In	Out	Total
210	Single-Family (174 Units)	32	96	128	109	64	173	865	865	1,730
220	Townhomes (70 Units)	8	26	34	27	16	43	244	244	488
	Total	40	122	162	136	80	216	1,109	1,109	2,218



LEGEND
00% - PERCENT DISTRIBUTION



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Estimated Directional Distribution



Job No: 20-177

Figure: 5

4. Projected Traffic Conditions

The total projected traffic volumes include the existing traffic volumes, increase in background traffic due to growth, and the traffic estimated to be generated by the proposed subject development.

Development Traffic Assignment

The estimated weekday morning and weekday evening peak hour traffic volumes that will be generated by the proposed development were assigned to the roadway system in accordance with the previously described directional distribution (Figure 5). The total new traffic assignment for the development is illustrated in **Figure 6**.

Figure 7 shows the existing plus site traffic volumes.

Nokia Campus Vacancies

It is important to note that the Nokia campus is comprised of two buildings: 1960 Lucent Lane and 2000 Lucent Lane. The 1960 Lucent Lane is approximately 620,000 square feet in size and the 2000 Lucent Lane building is approximately 1,056,000 square feet in size. It is our understanding that in 2016 Nokia vacated the 1960 Lucent Lane and consolidated its work force in the 2000 Lucent Lane building. However, only 353,553 square feet are occupied at the 2000 Lucent Lane thus the two buildings have a total vacancy of approximately 1,322,447 square feet (620,000 s.f. + 702,447 s.f.)

To account for this, the volume of traffic to be generated by the full occupancy of the vacant space was estimated using data published in the *ITE Trip Generation Manual*, 10th Edition. The “General Office” (Land-Use Code 710) was used. **Table 5** tabulates the peak hour vehicle trips anticipated to be generated by the vacant office space. The *ITE Trip Generation Manual* 10th Edition rates and equations are included in the Appendix.

Table 5
NOKIA VACANCIES ESTIMATED PEAK HOUR GENERATED TRAFFIC VOLUMES

ITE Land Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Daily		
		In	Out	Total	In	Out	Total	In	Out	Total
710	Office (1,322,447 s.f.)	1,092	178	1,270	212	1,111	1,323	6,493	6,493	12,986

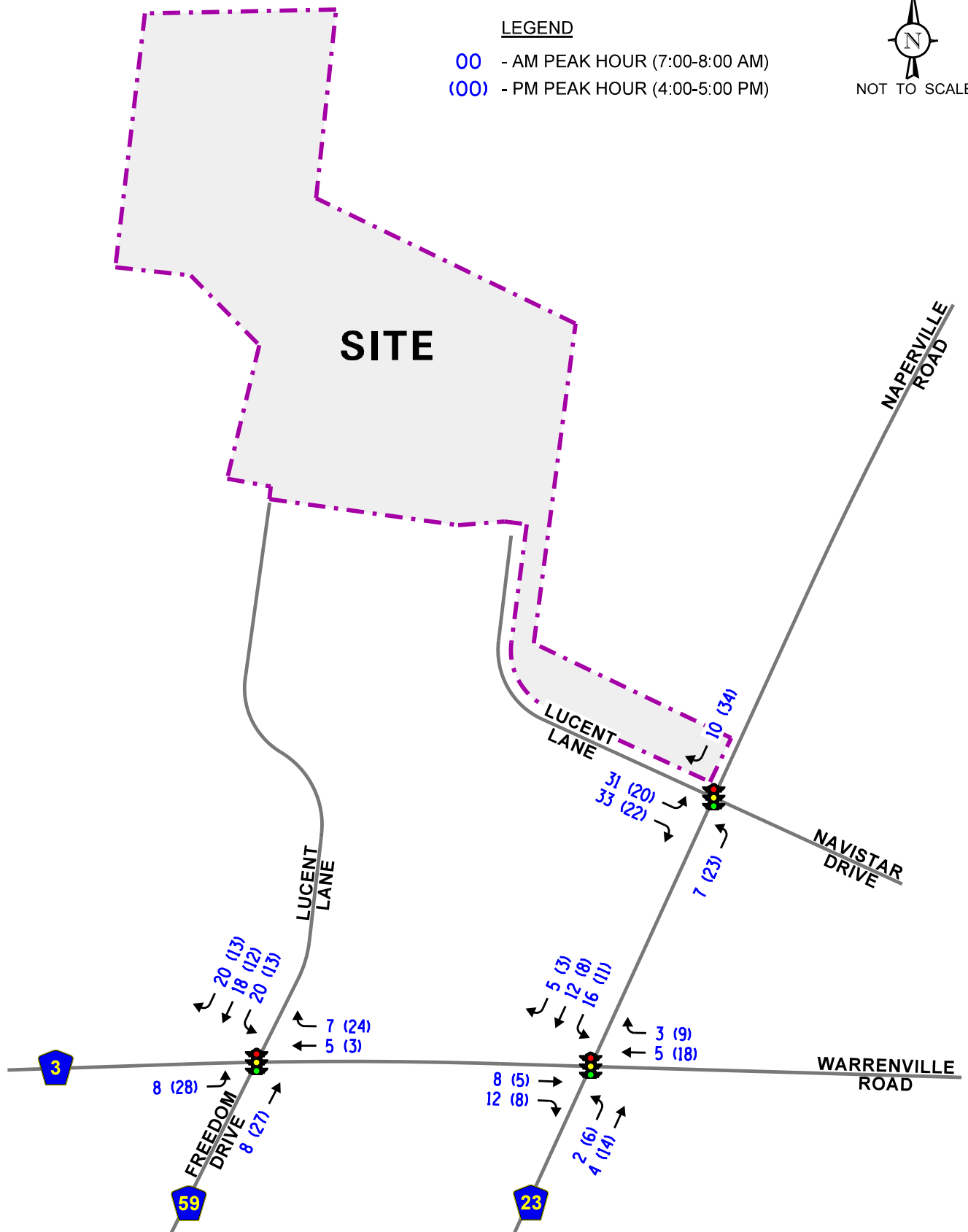
The estimated traffic volumes that will be generated by the vacant office space were assigned to the area roadways and is illustrated in **Figure 8**.

LEGEND

- 00 - AM PEAK HOUR (7:00-8:00 AM)
- (00) - PM PEAK HOUR (4:00-5:00 PM)



NOT TO SCALE



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Estimated Site Traffic Assignment



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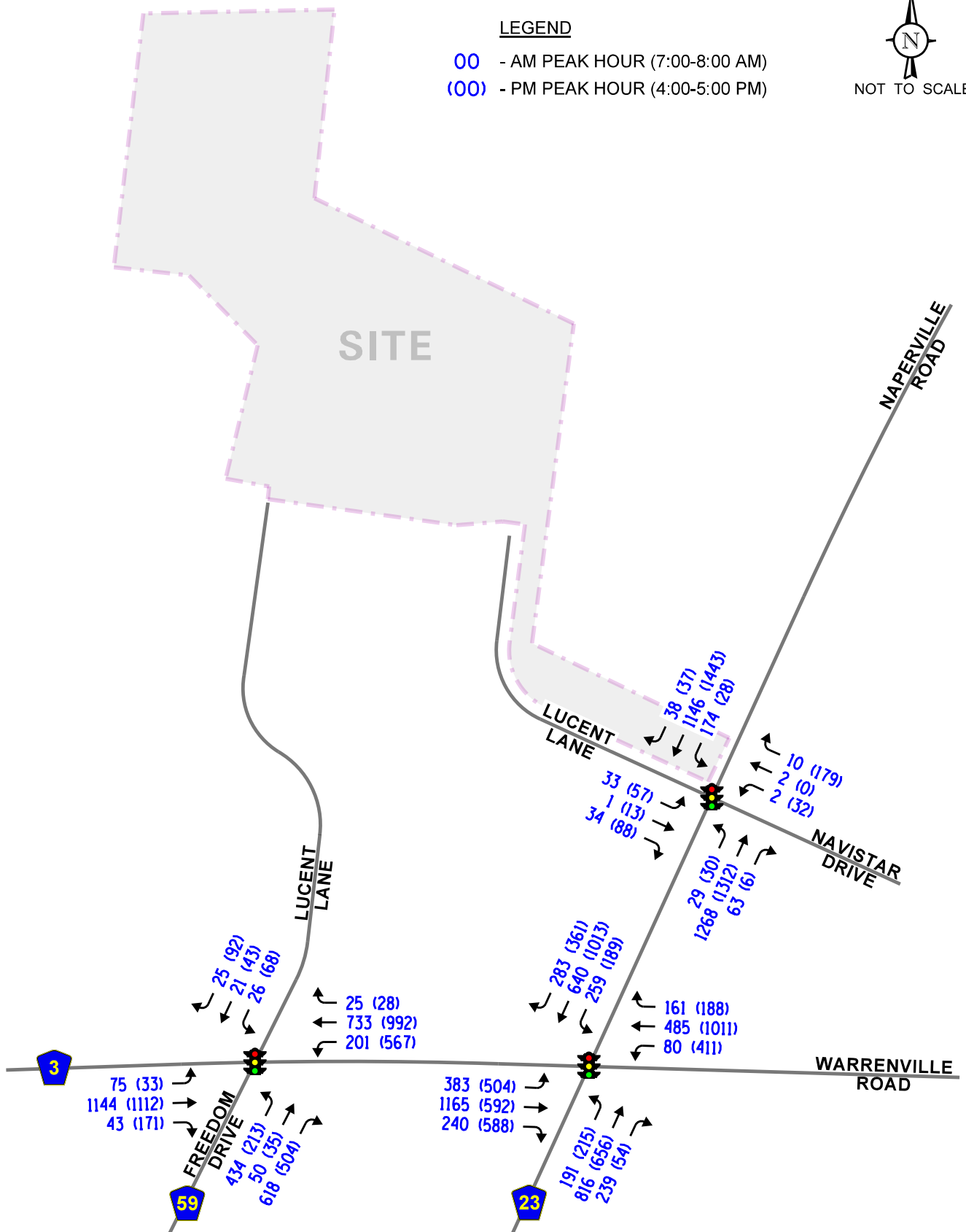
Figure: 6

LEGEND

- 00 - AM PEAK HOUR (7:00-8:00 AM)
- (00) - PM PEAK HOUR (4:00-5:00 PM)



NOT TO SCALE



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Existing Plus Site-Generated Traffic Volumes



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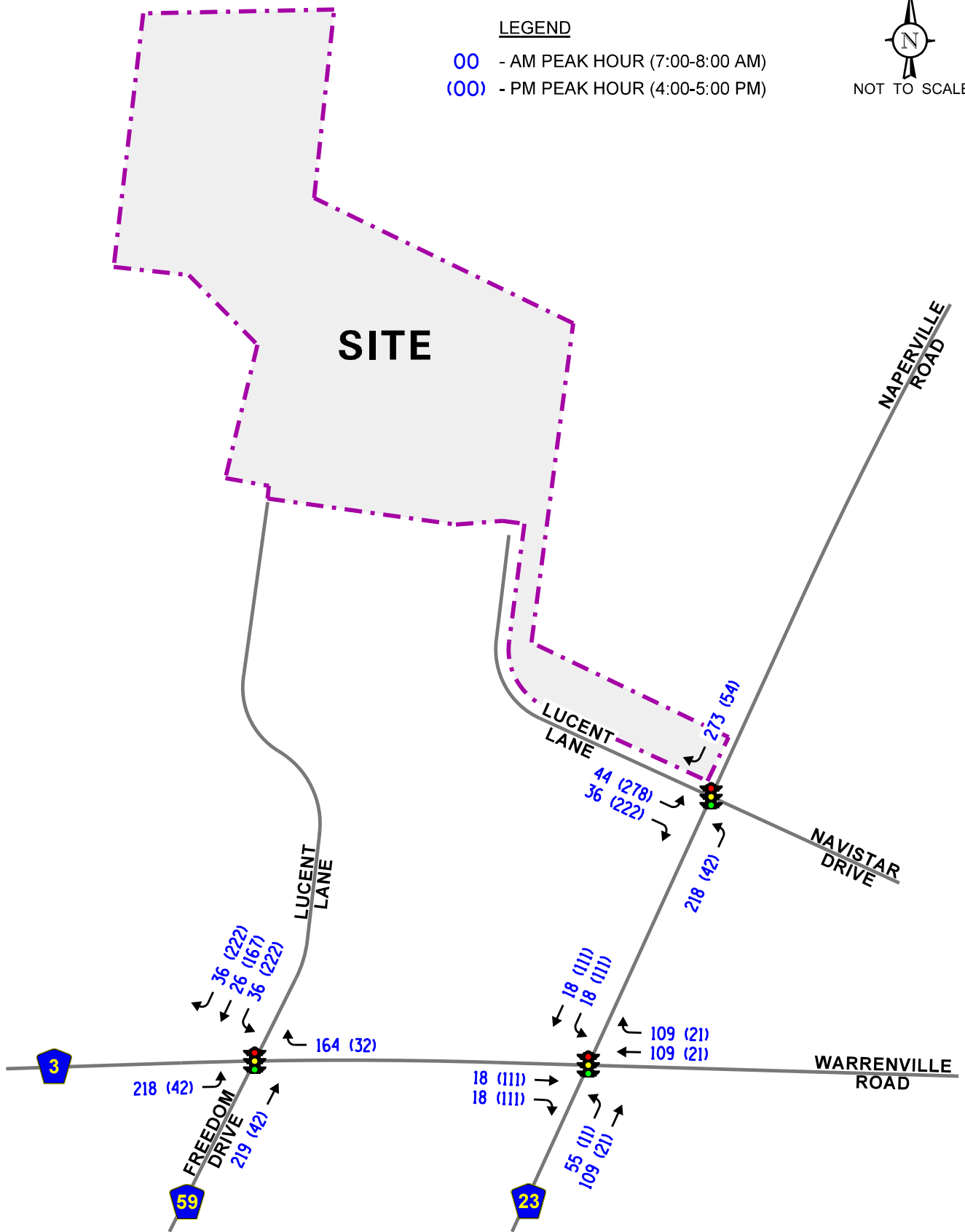
Figure: 7

LEGEND

- 00 - AM PEAK HOUR (7:00-8:00 AM)
- (00) - PM PEAK HOUR (4:00-5:00 PM)



NOT TO SCALE



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Vacant Office Traffic Assignment



Job No: 20-177

Figure: 8

Background (No-Build) Traffic Conditions

The existing traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any particular planned development). Based on ADT projections provided by the Chicago Metropolitan Agency for Planning (CMAP), the existing traffic volumes are projected to increase by a compound annual growth rate of 0.44 percent per year. As such, traffic volumes were increased by 3.20 percent to represent Year 2026 conditions. A copy of the CMAP projections letter is included in the Appendix. In addition, the traffic to be generated by the Nokia campus vacancies (Figure 7) was included in the no-build conditions. The Year 2026 no-build traffic volumes are illustrated in **Figure 9**.

Total Projected Traffic Volumes

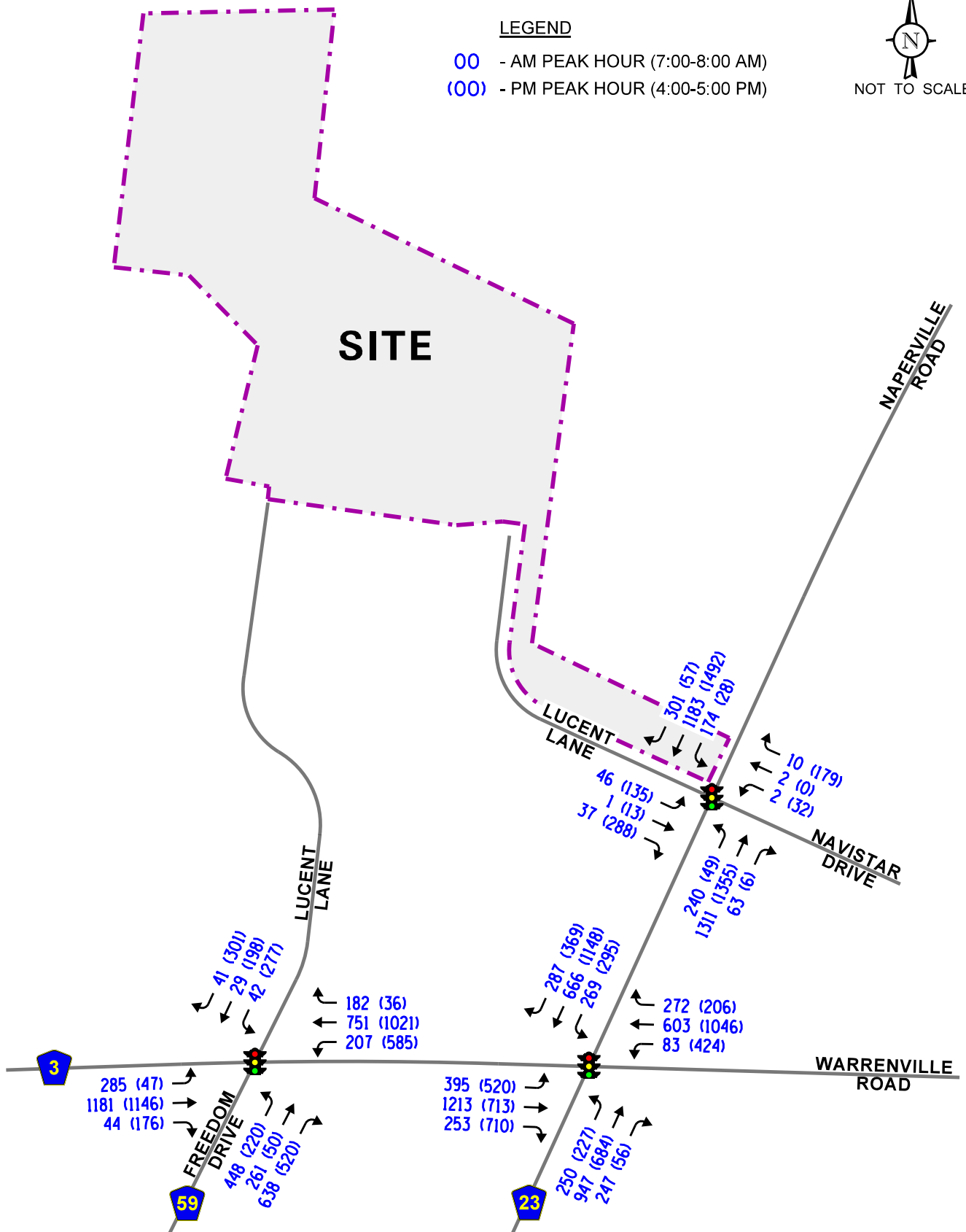
The development-generated traffic (Figure 6) was added to the Year 2026 no-build traffic volumes (Figure 8) to determine the Year 2026 total projected traffic volumes, as illustrated in **Figure 10**.



NOT TO SCALE

LEGEND

- 00 - AM PEAK HOUR (7:00-8:00 AM)
- (00) - PM PEAK HOUR (4:00-5:00 PM)



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Year 2026 No-Build Traffic Volumes
(Includes Nokia Campus Full Occupancy)



Job No: 20-177

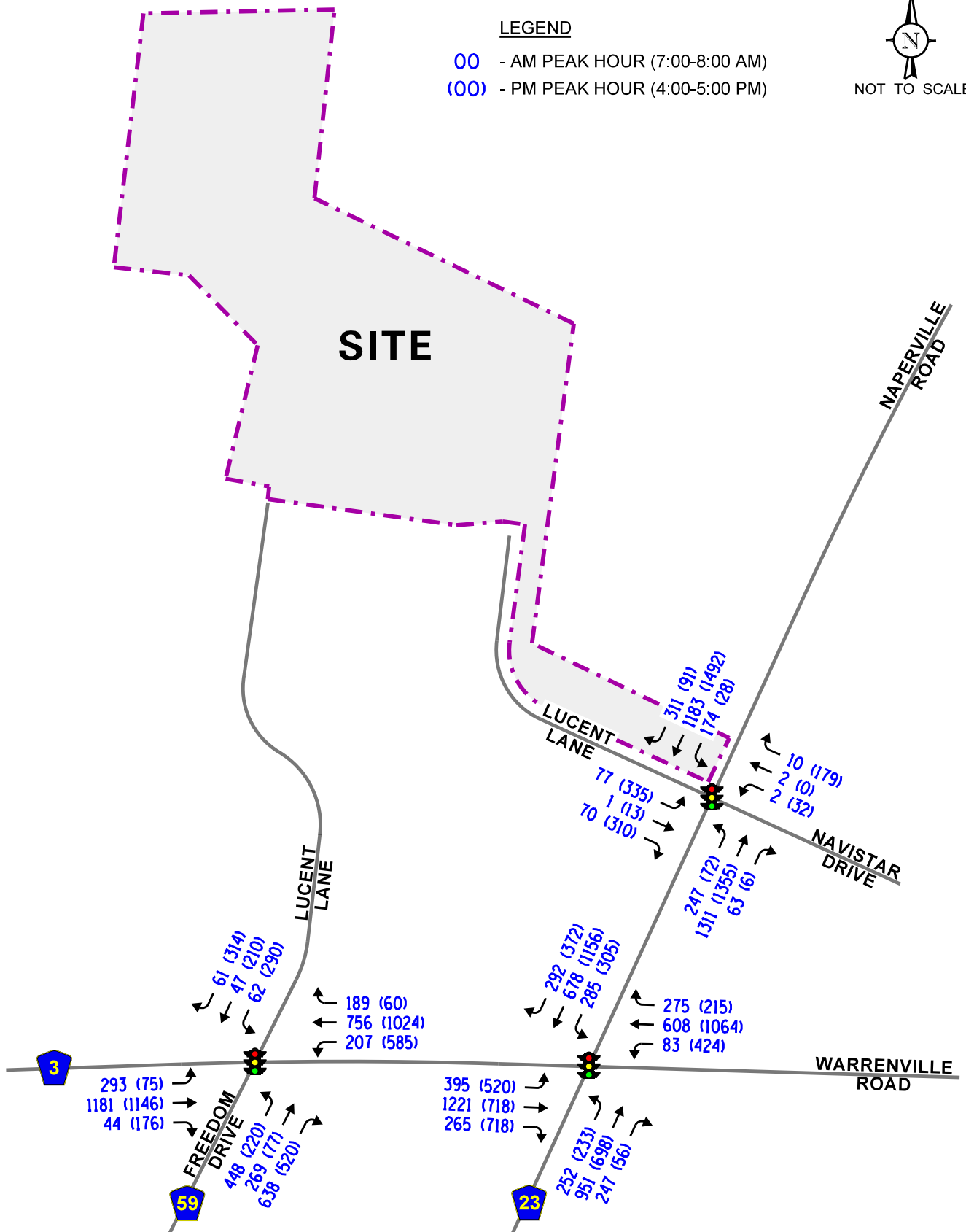
Figure: 9



NOT TO SCALE

LEGEND

- 00 - AM PEAK HOUR (7:00-8:00 AM)
- (00) - PM PEAK HOUR (4:00-5:00 PM)



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Year 2026 Total Projected Traffic Volumes



5. Traffic Analysis and Recommendations

The following provides an evaluation conducted for the weekday morning and evening peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access roadways are projected to operate and whether any roadway improvements or modifications are required.

Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning and evening peak hours for the existing (Year 2019), Existing Plus Site, Year 2026 no-build, and Year 2026 total projected traffic volumes.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM), 2010* and analyzed using the Synchro/SimTraffic 10 computer software. Synchro/SimTraffic 10 was utilized to represent the operations of the intersections.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the existing, existing plus site, Year 2026 no-build, and Year 2026 total projected conditions are presented in **Tables 6** through **9**. A discussion of the intersections follows. Summary sheets for the capacity analyses are included in the Appendix.

Table 6

CAPACITY ANALYSIS RESULTS – NAPERVILLE ROAD WITH WARRENVILLE ROAD – SIGNALIZED

	Peak Hour	Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Year 2019 Existing Conditions	Weekday Morning	E 58.9	D 44.7	B 13.3	E 77.0	D 44.7	B 12.1	E 74.2	D 52.5	C 23.5	E 72.9	D 39.4	B 12.4	D 44.1
		D – 43.7			D – 41.1			D – 50.2			D – 40.0			
Year 2019 Existing Plus Site Conditions	Weekday Morning	F 108.8	C 29.6	C 32.2	E 70.6	D 53.9	B 12.8	E 72.9	D 50.6	A 5.0	E 69.2	D 48.5	C 29.8	D 51.8
		D – 54.4			D – 53.6			D – 53.0			D – 46.5			
Year 2019 Existing Plus Site Conditions	Weekday Morning	E 57.1	D 46.3	B 12.3	E 77.0	D 44.5	B 12.1	E 74.4	D 53.7	C 23.8	E 72.5	D 38.1	B 11.2	D 44.2
		D – 44.0			D – 40.9			D – 51.1			D – 39.2			
Year 2019 Existing Plus Site Conditions	Weekday Evening	F 111.3	C 28.5	C 30.8	E 70.6	D 54.0	B 13.4	E 73.0	D 51.7	A 5.0	E 68.3	D 47.6	C 28.7	D 51.7
		D – 54.1			D – 53.5			D – 53.9			D 45.8			
Year 2026 No-Build Conditions	Weekday Morning	E 55.9	D 45.7	B 10.8	E 77.5	D 46.2	B 19.4	E 77.1	E 65.6	C 25.2	E 77.3	D 38.4	B 13.6	D 47.0
		D – 43.9			D – 41.3			E – 60.7			D – 41.2			
Year 2026 No-Build Conditions	Weekday Evening	F 106.2	D 36.0	C 30.5	E 70.4	D 54.0	B 13.6	E 65.4	E 59.3	A 5.8	E 68.6	E 67.0	C 27.6	E 55.4
		D – 52.8			D – 53.2			E – 57.6			E – 59.3			
Year 2026 Projected Conditions	Weekday Morning	E 55.8	D 45.3	B 10.7	E 77.5	D 46.0	B 19.2	E 77.2	E 69.1	C 25.5	E 78.1	D 38.2	B 13.6	D 47.5
		D – 42.6			D – 41.1			E – 63.1			D – 41.5			
Year 2026 Projected Conditions	Weekday Evening	F 104.7	D 36.1	C 29.1	E 70.4	D 54.1	B 14.0	E 64.9	E 60.7	A 5.8	E 70.5	E 77.1	C 27.6	E 57.2
		D – 51.8			D – 53.1			E – 58.6			E – 66.0			
Letter denotes Level of Service Delay is measured in seconds.		L – Left Turns T – Through			R – Right Turns									

Table 7

CAPACITY ANALYSIS RESULTS – WARRENVILLE ROAD WITH FREEDOM DRIVE/LUCENT LANE – SIGNALIZED

	Peak Hour	Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Year 2019 Existing Conditions	Weekday Morning	E 73.0	C 33.0	A 1.6	E 67.1	C 24.6	A 1.7	D 48.6	D 40.7	D 39.4	E 71.3	E 61.7	A 0.2	D 36.9
		C – 34.1			C – 33.2			D – 43.1			D – 43.9			
Year 2019 Existing Plus Site Conditions	Weekday Morning	E 73.2	D 36.0	A 2.0	E 68.6	C 27.6	A 3.4	E 57.9	D 41.2	D 39.5	E 76.0	R 56.9	A 0.4	D 40.0
		D – 37.1			D – 35.6			D – 46.8			D – 44.2			
Year 2019 Existing Plus Site Conditions	Weekday Evening	E 69.8	D 38.6	A 6.7	D 52.6	C 29.7	A 0.0	F 82.2	D 54.5	C 34.5	F 85.8	E 55.2	A 9.3	D 39.1
		C – 34.5			D – 38.0			D – 48.7			D – 43.5			
Year 2019 Existing Plus Site Conditions	Weekday Evening	E 71.7	D 40.2	A 6.7	D 53.8	C 31.9	A 2.3	E 74.2	E 55.1	C 34.7	F 92.9	E 55.7	B 12.3	D 40.3
		D – 36.7			D – 39.2			D – 46.8			D – 48.6			
Year 2026 No-Build Conditions	Weekday Morning	E 73.1	D 41.1	A 2.2	E 68.2	D 38.4	B 14.6	E 61.9	D 44.2	D 37.8	F 80.7	D 54.0	A 0.3	D 44.7
		D – 46.0			D – 40.0			D – 47.1			D – 44.4			
Year 2026 No-Build Conditions	Weekday Evening	E 71.5	D 42.7	B 10.0	D 54.9	C 32.9	A 3.5	F 118.4	E 55.6	C 34.4	F 669.6	E 59.6	D 46.4	F 82.6
		D – 39.5			D – 40.1			E – 59.2			F – 272.5			
Year 2026 Projected Conditions	Weekday Morning	E 73.0	D 42.0	A 2.2	E 68.7	D 39.3	B 14.8	E 60.8	D 44.2	D 37.8	F 89.3	D 54.6	A 3.2	D 45.1
		D – 46.8			D – 40.5			D – 46.7			D – 48.8			
Year 2026 Projected Conditions	Weekday Evening	E 73.6	D 42.7	B 10.9	D 54.9	C 33.5	A 4.9	F 118.4	E 56.2	C 34.4	F 715.3	E 60.0	D 47.4	F 86.5
		D – 40.4			D – 40.0			E – 59.1			F – 288.4			

Letter denotes Level of Service
 Delay is measured in seconds.
 L – Left Turns
 T – Through
 R – Right Turns

Table 8

CAPACITY ANALYSIS RESULTS – WARRENVILLE ROAD WITH FREEDOM DRIVE/LUCENT LANE – SIGNALIZED

	Peak Hour	Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Year 2026 No-Build Conditions ¹	Weekday Morning	E 73.1	D 39.8	A 2.2	E 67.6	D 37.2	B 14.2	E 61.9	D 44.0	D 37.7	E 72.1	E 55.2	A 0.3	D 43.9
		D – 45.0			D – 39.1			D – 46.9			D – 41.5			
	Weekday Evening	E 71.5	D 42.7	B 10.0	D 54.9	C 32.9	A 3.5	F 118.4	E 55.6	C 34.4	F 184.7	E 59.6	D 46.4	D 53.2
		D – 39.5			D – 40.1			E – 59.2			F – 99.2			
Year 2026 Projected Conditions ¹	Weekday Morning	E 73.0	D 40.7	A 2.2	E 67.7	D 38.2	B 14.5	E 60.9	D 44.1	D 37.6	E 72.9	E 55.5	A 3.2	D 44.3
		D – 45.8			D – 39.6			D – 46.6			D – 43.1			
	Weekday Evening	E 73.6	D 42.7	B 10.9	D 54.9	C 33.5	A 4.9	F 118.4	E 56.2	C 34.4	F 203.3	E 60.0	D 47.4	D 54.9
		D – 40.4			D – 40.0			E – 59.1			F – 106.1			
Letter denotes Level of Service L – Left Turns R – Right Turns														
Delay is measured in seconds. T – Through														
1 – Assumes the restriping of the southbound approach to provide dual left-turn lanes, two through lanes and an exclusive right-turn lane														

Table 9

CAPACITY ANALYSIS RESULTS – NAPERVILLE ROAD WITH LUCENT LANE/NAVISTAR DRIVE – SIGNALIZED

	Peak Hour	Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Year 2019 Existing Conditions	Weekday Morning	E 63.5	E 59.0		E 63.5	E 70.0	A 0.8	A 2.0	A 6.8	A 2.2	A 4.9	A 2.7	A 0.0	A 4.9
		E – 61.3			B – 18.4			A – 6.6			A – 2.9			
Year 2019 Existing Plus Site Conditions	Weekday Morning	E 70.2	C 29.5		E 61.5	E 70.0	A 0.8	A 2.8	B 10.0	A 3.3	A 6.7	A 4.6	A 0.1	A 8.3
		D – 49.2			B – 18.1			A – 9.6			A – 4.8			
Year 2019 Existing Plus Site Conditions	Weekday Evening	E 57.3	C 31.7		E 75.8	--	D 37.3	B 11.8	C 32.5	A 0.0	A 4.6	B 10.2	A 0.1	C 22.8
		D – 40.9			D – 43.2			C – 31.9			A – 9.8			
Year 2026 No-Build Conditions	Weekday Morning	E 73.8	C 28.6		E 61.5	E 70.0	A 0.7	B 19.8	A 9.6	A 2.6	A 7.5	A 7.5	A 3.8	B 10.0
		D – 53.2			B – 18.0			B – 10.9			A – 6.8			
Year 2026 No-Build Conditions	Weekday Evening	F 96.0	E 55.2		E 77.0	--	F 128.0	B 17.0	D 45.6	A 0.0	A 7.2	B 16.9	A 1.2	D 42.0
		E – 76.1			F – 120.2			D – 44.4			B – 16.1			
Year 2026 Projected Conditions	Weekday Morning	F 85.5	C 25.2		E 61.0	E 69.0	A 0.7	C 21.7	B 10.7	A 2.6	A 8.4	A 8.6	A 4.1	B 11.9
		E – 56.5			B – 17.8			B – 12.1			A – 7.7			
Year 2026 Projected Conditions	Weekday Evening	F 94.8	E 60.4		E 77.0	--	F 151.7	B 19.3	D 46.1	A 0.0	A 7.5	B 18.9	A 3.6	D 44.3
		E – 77.9			F – 140.2			D – 44.6			B – 17.8			

Letter denotes Level of Service
Delay is measured in seconds.

L – Left Turns
T – Through

R – Right Turns

Discussion and Recommendations

The following summarizes how the intersections are projected to operate and identifies any roadway and traffic control improvements necessary to accommodate the development traffic. As discussed in this section, the results of the capacity analyses under Year 2026 total projected conditions reflect primarily the assumed increase in background traffic volumes due to the regional growth factors and the traffic to be generated by the substantial vacancies within the Nokia campus discussed in the previous chapter.

Naperville Road with Warrenville Road

The results of the capacity analysis indicate that overall this intersection currently operates at Level of Service (LOS) D during the weekday morning and evening peak hours. It is important to note that while the left-turn movements operate at a LOS E or F, this is due to the long cycle length and the fact that these movements can only proceed through the intersection on a protected phase (green arrow) only. Inspection of the simulation runs indicate that these movements, although operating at LOS E or F, are able to process the traffic volumes efficiently.

Under existing traffic plus site conditions, the intersection will continue operating at the same LOS with virtually no changes in the delay experienced at this intersection. Therefore, this indicates that the proposed Naper Commons development will have a minimal impact on traffic conditions at this intersection.

Under Year 2026 no-build conditions, assuming the background growth and the traffic to be generated by the Nokia campus vacancies, this intersection overall is projected to operate at LOS D during the weekday morning peak hour and LOS E during the weekday evening peak hour with increases in overall delay of approximately three seconds and four seconds, respectively.

Under Year 2026 total projected traffic conditions, this intersection overall is projected to continue operating at LOS D during the weekday morning peak hour and LOS E during the weekday evening peak hour with increases in delay of approximately 0.5 seconds and two seconds, respectively, over Year 2026 no-build conditions. As previously indicated, this indicates that the proposed development will have a minimal impact on traffic condition at this intersection. In fact inspection of the traffic volumes indicates that the proposed development will amount to approximately one percent of the traffic at this intersection. As such, no geometric or traffic control improvements will be necessary in conjunction with the proposed development.

Warrenville Road with Freedom Drive/Lucent Lane

The results of the capacity analysis indicate that overall this intersection currently operates at LOS D during the weekday morning and evening peak hours. It is important to note that while the left-turn movements operate at a LOS E or F, it is due particularly to the long cycle length and the fact that these movements can only proceed through the intersection on a protected phase (green arrow) only. Inspection of the simulation runs indicate that these movements are contained within the provided storage length and are processed efficiently.

Under existing traffic plus site conditions, the intersection will continue operating at the same overall LOS with increases in delay of approximately three seconds during the morning peak hour and one second during the evening peak hour. Therefore, this indicates that the proposed Naper Commons development will have a minimal impact on traffic conditions at this intersection.

Under Year 2026 no-build conditions, assuming the background growth and the traffic to be generated by the Nokia campus vacancies, this intersection overall is projected to operate at LOS D during the weekday morning peak hour and LOS F during the weekday evening peak hour. It is important to note that the LOS F to be experienced during the weekday evening peak hour is mainly the result of the projected southbound left-turn movement (over 270 vehicles) in a single left-turn lane and on a protected phase (green arrow) only. If the Nokia campus is fully occupied, this poor LOS can be mitigated by restriping the southbound approach to provide dual left-turn lanes, two through lanes and an exclusive right-turn lane. Based on the results of the capacity analyses, the overall LOS during the evening peak hour would improve to LOS D.

Under Year 2026 total projected conditions, the overall delay will increase by approximately 0.5 seconds and four seconds during the weekday morning and evening peak hours, respectively. This indicates that the proposed development will have a minimal impact on traffic conditions at this intersection. As such, no geometric or traffic control improvements will be necessary at this intersection in conjunction with the proposed development.

Naperville Road with Lucent Lane/Navistar Drive

The results of the capacity analysis indicate that overall this intersection currently operates at LOS A during the weekday morning peak hour and LOS C during the weekday evening peak hour.

Under existing traffic plus site conditions, the intersection will continue operating at the same overall LOS with increases in delay of approximately three seconds during the morning peak hour and two seconds during the evening peak hour. Therefore, this indicates that the proposed Naper Commons development will have a minimal impact on traffic conditions at this intersection.

Under Year 2026 no-build conditions, assuming the background growth and the traffic to be generated by the Nokia campus vacancies, this intersection overall is projected to operate at LOS B during the weekday morning peak hour and LOS D during the weekday evening peak hour.

Under Year 2026 total projected conditions, the overall delay will increase by approximately two seconds during the weekday morning and evening peak hours. This indicates that the proposed development will have a minimal impact on traffic conditions at this intersection. As such, no geometric or traffic control improvements will be necessary at this intersection in conjunction with the proposed development.

7. Conclusion

Based on the preceding analyses and recommendations, the following conclusions have been made:

- The traffic that will be generated by the proposed development can be accommodated by the area roadway system.
- The projected traffic volumes will amount to approximately one percent of the traffic at the intersection of Naperville Road with Warrenville Road and will have a negligible impact on traffic conditions in the area.
- The existing access system serving the Nokia campus and the proposed development will ensure that efficient and flexible access is provided.
- If the Nokia campus becomes fully occupied, consideration should be given to restriping the southbound approach of Lucent Lane at its intersection with Warrenville Road to provide dual left-turn lanes, two through lanes, and an exclusive right-turn lane. This can be accommodated by the existing pavement width.

Appendix

Traffic Count Summary Sheets

Site Plan

ITE Trip Generation Rates and Equations

CMAP 2050 Projections Letter

Level of Service Criteria

Capacity Analysis Summary Sheets

Traffic Count Summary Sheets

DuPage County Division of Transportation
421 N. County Farm Road
Wheaton, Illinois, 60187

File Name: Warrenville and Freedom
Study Date: 09/10/19

Freedom Southbound				
Time	Right	Thru	Left	Appr. Total
7:00	1	0	2	3
7:15	2	0	1	3
7:30	1	2	1	4
7:45	1	1	2	4
Total	5	3	6	14

Warrenville Westbound				
Time	Right	Thru	Left	Appr. Total
7:00	5	153	51	209
7:15	3	169	47	219
7:30	3	187	59	249
7:45	7	236	44	287
Total	18	745	201	964

Freedom Northbound				
Time	Right	Thru	Left	Appr. Total
7:00	142	7	77	226
7:15	127	9	100	236
7:30	182	12	113	307
7:45	167	14	144	325
Total	618	42	434	1094

Warrenville Eastbound				
Time	Right	Thru	Left	Appr. Total
7:00	9	228	18	255
7:15	10	276	14	300
7:30	11	343	17	371
7:45	13	312	18	343
Total	43	1159	67	1269

Freedom Southbound				
Time	Right	Thru	Left	Appr. Total
16:00	29	4	15	48
16:15	13	7	10	30
16:30	16	7	14	37
16:45	21	13	16	50
Total	79	31	55	165

Warrenville Westbound				
Time	Right	Thru	Left	Appr. Total
16:00	1	241	144	386
16:15	1	237	146	384
16:30	0	286	149	435
16:45	2	296	128	426
Total	4	1060	567	1631

Freedom Northbound				
Time	Right	Thru	Left	Appr. Total
16:00	109	4	29	142
16:15	144	1	44	189
16:30	121	2	84	207
16:45	130	1	56	187
Total	504	8	213	725

Warrenville Eastbound				
Time	Right	Thru	Left	Appr. Total
16:00	45	314	1	360
16:15	38	261	2	301
16:30	49	327	2	378
16:45	39	319	0	358
Total	171	1221	5	1397

DuPage County Division of Transportation
421 N. County Farm Road
Wheaton, Illinois, 60187

File Name: Warrenville and Naperville
Study Date: 8/29/19

Naperville Southbound				
Time	Right	Thru	Left	Appr. Total
7:00	59	100	39	198
7:15	67	162	58	287
7:30	59	183	74	316
7:45	93	183	72	348
Total	278	628	243	1149

Warrenville Westbound				
Time	Right	Thru	Left	Appr. Total
7:00	33	120	20	173
7:15	41	116	14	171
7:30	33	113	24	170
7:45	51	131	22	204
Total	158	480	80	718

Naperville Northbound				
Time	Right	Thru	Left	Appr. Total
7:00	45	155	56	256
7:15	61	208	49	318
7:30	66	233	42	341
7:45	67	216	42	325
Total	239	812	189	1240

Warrenville Eastbound				
Time	Right	Thru	Left	Appr. Total
7:00	55	206	89	350
7:15	56	289	88	433
7:30	65	311	104	480
7:45	52	351	102	505
Total	228	1157	383	1768

Naperville Southbound				
Time	Right	Thru	Left	Appr. Total
16:00	76	237	35	348
16:15	83	228	43	354
16:30	80	243	50	373
16:45	119	297	50	466
Total	358	1005	178	1541

Warrenville Westbound				
Time	Right	Thru	Left	Appr. Total
16:00	47	213	95	355
16:15	42	241	87	370
16:30	47	269	117	433
16:45	43	270	112	425
Total	179	993	411	1583

Naperville Northbound				
Time	Right	Thru	Left	Appr. Total
16:00	13	150	49	212
16:15	9	155	50	214
16:30	13	143	59	215
16:45	19	194	51	264
Total	54	642	209	905

Warrenville Eastbound				
Time	Right	Thru	Left	Appr. Total
16:00	130	108	129	367
16:15	144	154	115	413
16:30	162	133	140	435
16:45	144	192	120	456
Total	580	587	504	1671

DuPage County Division of Transportation
 421 N. County Farm Road
 Wheaton, IL. 60187

File Name: Naperville Road and Lucent
 Study Date: 7/8/18

Time	Naperville Southbound				Lucent Westbound				Naperville Northbound				Lucent Eastbound		
	Right	Thru	Left	Appr. Total	Right	Thru	Left	Appr. Total	Right	Thru	Left	Appr. Total	Right	Thru	Left
7:00	5	199	41	245	4	0	0	4	17	246	7	270	0	0	0
7:15	8	262	28	298	4	1	1	6	10	323	5	338	0	0	0
7:30	9	312	49	370	1	0	0	1	13	342	3	358	1	1	1
7:45	6	345	56	407	1	1	1	3	23	285	7	315	0	0	1
Total	28	1118	174	1320	10	2	2	14	63	1196	22	1281	1	1	2

Time	Naperville Southbound				Lucent Westbound				Naperville Northbound				Lucent Eastbound		
	Right	Thru	Left	Appr. Total	Right	Thru	Left	Appr. Total	Right	Thru	Left	Appr. Total	Right	Thru	Left
16:00	1	304	4	309	46	0	5	51	2	270	1	273	12	0	13
16:15	2	302	7	311	26	0	15	41	1	312	1	314	19	2	4
16:30	0	345	6	351	60	0	7	67	0	375	3	378	19	0	6
16:45	0	341	11	352	47	0	5	52	3	351	2	356	16	11	14
Total	3	1292	28	1323	179	0	32	211	6	1308	7	1321	66	13	37

t ind
Appr. Total
0
0
3
1
4

t ind
Appr. Total
25
25
25
41
116

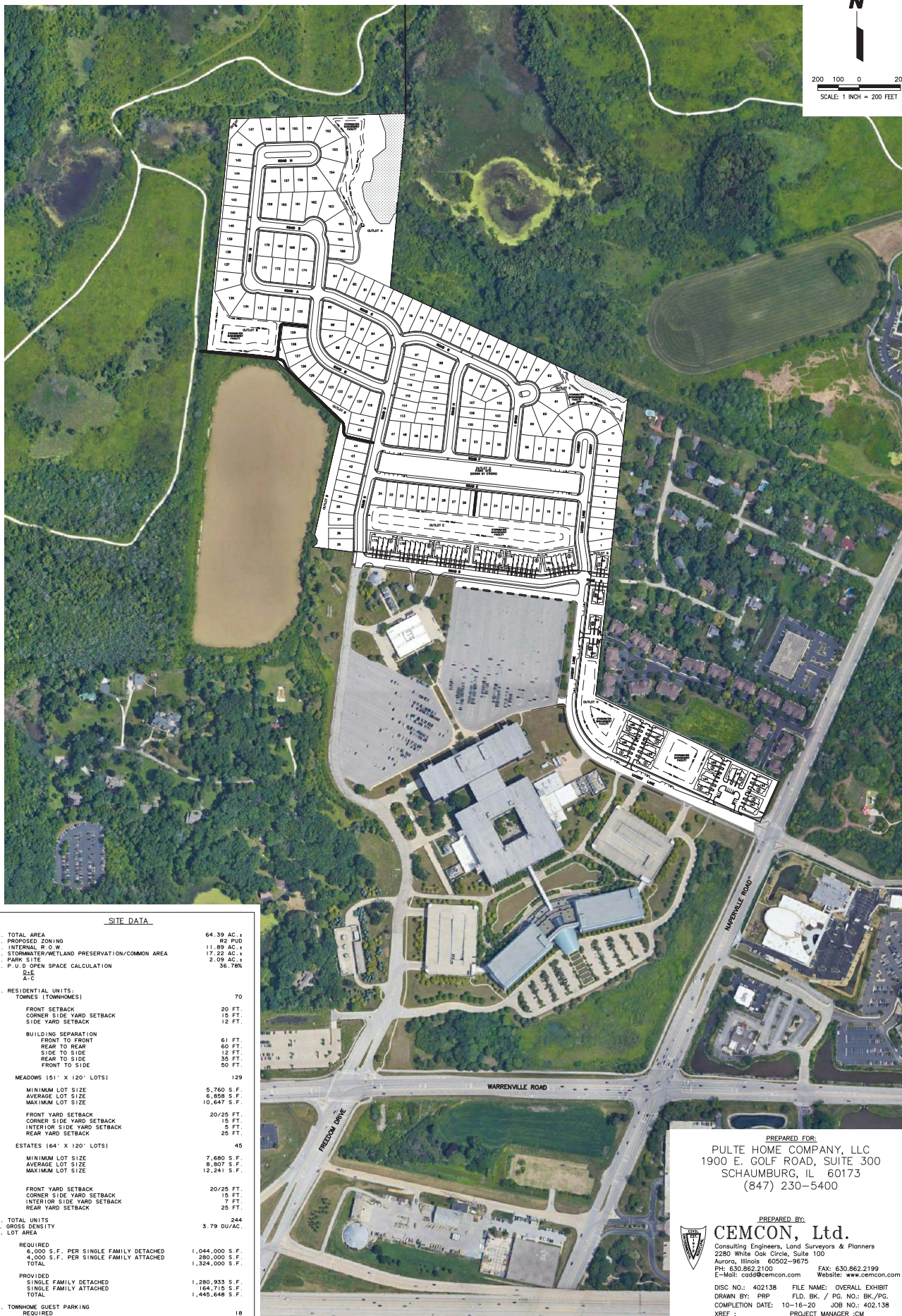
Site Plan

OVERALL SITE PLAN FOR NAPERVILLE COMMONS

NAPERVILLE, IL



200 100 0 200
SCALE: 1 INCH = 200 FEET



SITE DATA

A. TOTAL AREA	64.39 AC. ±
B. PROPOSED ZONING	R2 PUD
C. INTERNAL P.D.W.	11.89 AC. ±
D. C. STORMWATER WETLAND PRESERVATION/Common Area	17.22 AC. ±
E. PARK SITE	2.09 AC. ±
F. P.U.D OPEN SPACE CALCULATION	36.76%
G. RESIDENTIAL UNITS:	
TOWNHOMES (TOWNHOMES)	70
FRONT SETBACK	20 FT.
CORNER SIDE YARD SETBACK	15 FT.
SIDE YARD SETBACK	12 FT.
BUILDING SEPARATION	
FRONT TO FRONT	61 FT.
REAR TO REAR	60 FT.
SIDE TO SIDE	12 FT.
REAR TO SIDE	35 FT.
FRONT TO SIDE	50 FT.
MEADOWS (51' X 120' LOTS)	129
MINIMUM LOT SIZE	5,760 S.F.
AVERAGE LOT SIZE	6,898 S.F.
MAXIMUM LOT SIZE	10,647 S.F.
FRONT YARD SETBACK	20/25 FT.
CORNER SIDE YARD SETBACK	15 FT.
INTERIOR SIDE YARD SETBACK	5 FT.
REAR YARD SETBACK	25 FT.
ESTATES (64' X 120' LOTS)	45
MINIMUM LOT SIZE	7,680 S.F.
AVERAGE LOT SIZE	8,807 S.F.
MAXIMUM LOT SIZE	12,241 S.F.
FRONT YARD SETBACK	20/25 FT.
CORNER SIDE YARD SETBACK	15 FT.
INTERIOR SIDE YARD SETBACK	7 FT.
REAR YARD SETBACK	25 FT.
H. TOTAL UNITS	244
I. GROSS DENSITY	3.79 DU/AC.
J. LOT AREA	
REQUIRED	
8,000 S.F. PER SINGLE FAMILY DETACHED	1,044,000 S.F.
4,000 S.F. PER SINGLE FAMILY ATTACHED	280,000 S.F.
TOTAL	1,324,000 S.F.
PROVIDED	
SINGLE FAMILY DETACHED	1,280,933 S.F.
SINGLE FAMILY ATTACHED	164,715 S.F.
TOTAL	1,445,648 S.F.
K. TOWNHOME GUEST PARKING	
REQUIRED	18
PROVIDED	21

DRAWING FILE: P:\NAPERVILLE\NAPERVILLE COMMONS\OVERALL EXHIBIT.DWG

PREPARED FOR:
PULTE HOME COMPANY, LLC
1900 E. GOLF ROAD, SUITE 300
CHAUMBURG, IL 60173
(847) 230-5400



PREPARED BY: CEMCON, Ltd.

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2280 White Oak Circle, Suite 100
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DISC NO.: 402138 FILE NAME: OVERALL EXHIBIT
DRAWN BY: PRP FILE NO.: BK./PG.
COMPLETION DATE: 10-16-20 JOB NO.: 402138
XREF: PROJECT MANAGER :CM

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ITE Trip Generation Rates and Equations

Land Use: 210

Single-Family Detached Housing

Description

Single-family detached housing includes all single-family detached homes on individual lots. A typical site surveyed is a suburban subdivision.

Additional Data

The number of vehicles and residents had a high correlation with average weekday vehicle trip ends. The use of these variables was limited, however, because the number of vehicles and residents was often difficult to obtain or predict. The number of dwelling units was generally used as the independent variable of choice because it was usually readily available, easy to project, and had a high correlation with average weekday vehicle trip ends.

This land use included data from a wide variety of units with different sizes, price ranges, locations, and ages. Consequently, there was a wide variation in trips generated within this category. Other factors, such as geographic location and type of adjacent and nearby development, may also have had an effect on the site trip generation.

Single-family detached units had the highest trip generation rate per dwelling unit of all residential uses because they were the largest units in size and had more residents and more vehicles per unit than other residential land uses; they were generally located farther away from shopping centers, employment areas, and other trip attractors than other residential land uses; and they generally had fewer alternative modes of transportation available because they were typically not as concentrated as other residential land uses.

Time-of-day distribution data for this land use are presented in Appendix A. For the six general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 7:15 and 8:15 a.m. and 4:00 and 5:00 p.m., respectively. For the two sites with Saturday data, the overall highest vehicle volume was counted between 3:00 and 4:00 p.m. For the one site with Sunday data, the overall highest vehicle volume was counted between 10:15 and 11:15 a.m.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in California, Connecticut, Delaware, Illinois, Indiana, Maryland, Minnesota, Montana, New Jersey, North Carolina, Ohio, Oregon, Pennsylvania, South Carolina, South Dakota, Tennessee, Vermont, and Virginia.

Source Numbers

100, 105, 114, 126, 157, 167, 177, 197, 207, 211, 217, 267, 275, 293, 300, 319, 320, 356, 357, 367, 384, 387, 407, 435, 522, 550, 552, 579, 598, 601, 603, 614, 637, 711, 716, 720, 728, 735, 868, 903, 925, 936

Single-Family Detached Housing (210)

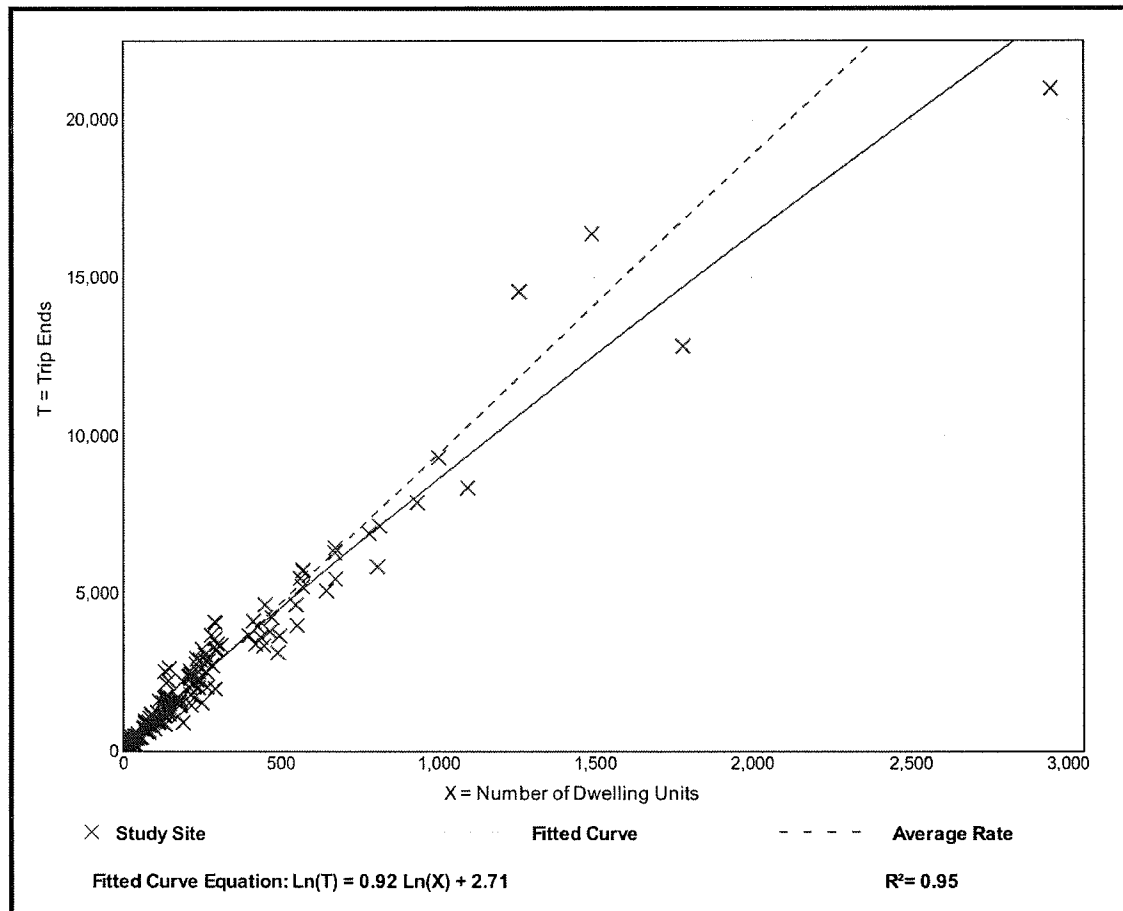
Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

Setting/Location: General Urban/Suburban
 Number of Studies: 159
 Avg. Num. of Dwelling Units: 264
 Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
9.44	4.81 - 19.39	2.10

Data Plot and Equation



Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 173

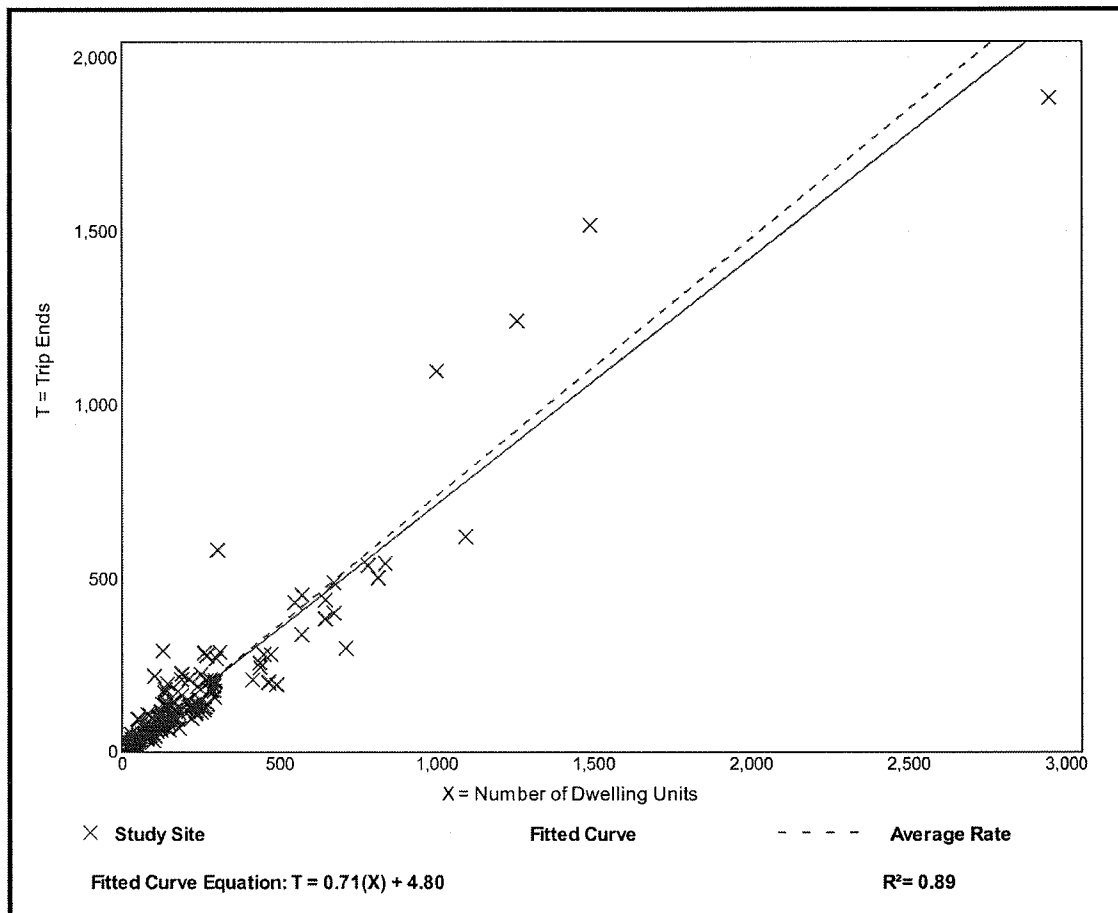
Avg. Num. of Dwelling Units: 219

Directional Distribution: 25% entering, 75% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.74	0.33 - 2.27	0.27

Data Plot and Equation



Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.

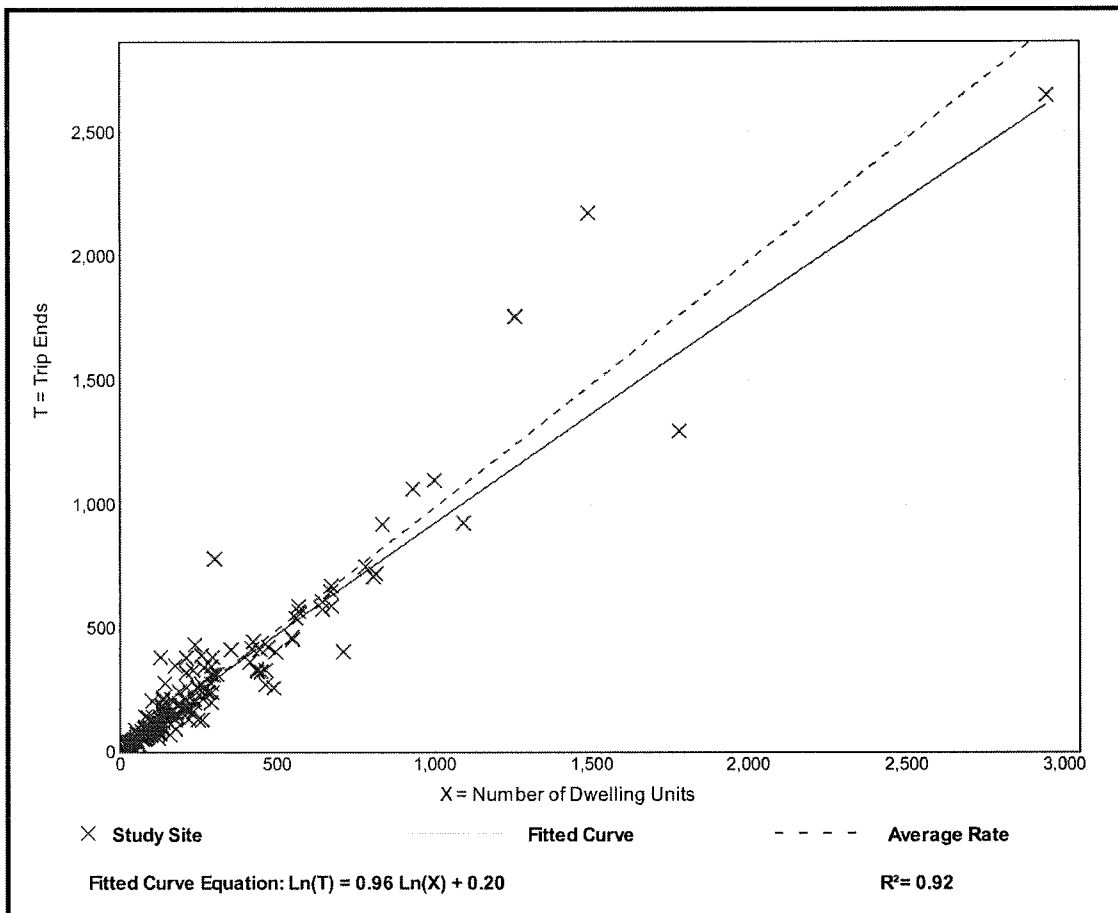
Setting/Location: General Urban/Suburban

Number of Studies: 190
 Avg. Num. of Dwelling Units: 242
 Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.99	0.44 - 2.98	0.31

Data Plot and Equation



Land Use: 220

Multifamily Housing (Low-Rise)

Description

Low-rise multifamily housing includes apartments, townhouses, and condominiums located within the same building with at least three other dwelling units and that have one or two levels (floors). Multifamily housing (mid-rise) (Land Use 221), multifamily housing (high-rise) (Land Use 222), and off-campus student apartment (Land Use 225) are related land uses.

Additional Data

In prior editions of *Trip Generation Manual*, the low-rise multifamily housing sites were further divided into rental and condominium categories. An investigation of vehicle trip data found no clear differences in trip making patterns between the rental and condominium sites within the ITE database. As more data are compiled for future editions, this land use classification can be reinvestigated.

For the three sites for which both the number of residents and the number of occupied dwelling units were available, there were an average of 2.72 residents per occupied dwelling unit.

For the two sites for which the numbers of both total dwelling units and occupied dwelling units were available, an average of 96.2 percent of the total dwelling units were occupied.

This land use included data from a wide variety of units with different sizes, price ranges, locations, and ages. Consequently, there was a wide variation in trips generated within this category. Other factors, such as geographic location and type of adjacent and nearby development, may also have had an effect on the site trip generation.

Time-of-day distribution data for this land use are presented in Appendix A. For the 10 general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 7:15 and 8:15 a.m. and 4:45 and 5:45 p.m., respectively. For the one site with Saturday data, the overall highest vehicle volume was counted between 9:45 and 10:45 a.m. For the one site with Sunday data, the overall highest vehicle volume was counted between 11:45 a.m. and 12:45 p.m.

For the one dense multi-use urban site with 24-hour count data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 7:00 and 8:00 a.m. and 6:15 and 7:15 p.m., respectively.

For the three sites for which data were provided for both occupied dwelling units and residents, there was an average of 2.72 residents per occupied dwelling unit.

The average numbers of person trips per vehicle trip at the five general urban/suburban sites at which both person trip and vehicle trip data were collected were as follows:

- 1.13 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 7 and 9 a.m.
- 1.21 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 4 and 6 p.m.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in British Columbia (CAN), California, District of Columbia, Florida, Georgia, Illinois, Indiana, Maine, Maryland, Minnesota, New Jersey, New York, Ontario, Oregon, Pennsylvania, South Dakota, Tennessee, Texas, Utah, Virginia, and Washington.

It is expected that the number of bedrooms and number of residents are likely correlated to the number of trips generated by a residential site. Many of the studies included in this land use did not indicate the total number of bedrooms. To assist in the future analysis of this land use, it is important that this information be collected and included in trip generation data submissions.

Source Numbers

168, 187, 188, 204, 211, 300, 305, 306, 319, 320, 321, 357, 390, 412, 418, 525, 530, 571, 579, 583, 864, 868, 869, 870, 896, 903, 918, 946, 947, 948, 951

Multifamily Housing (Low-Rise) (220)

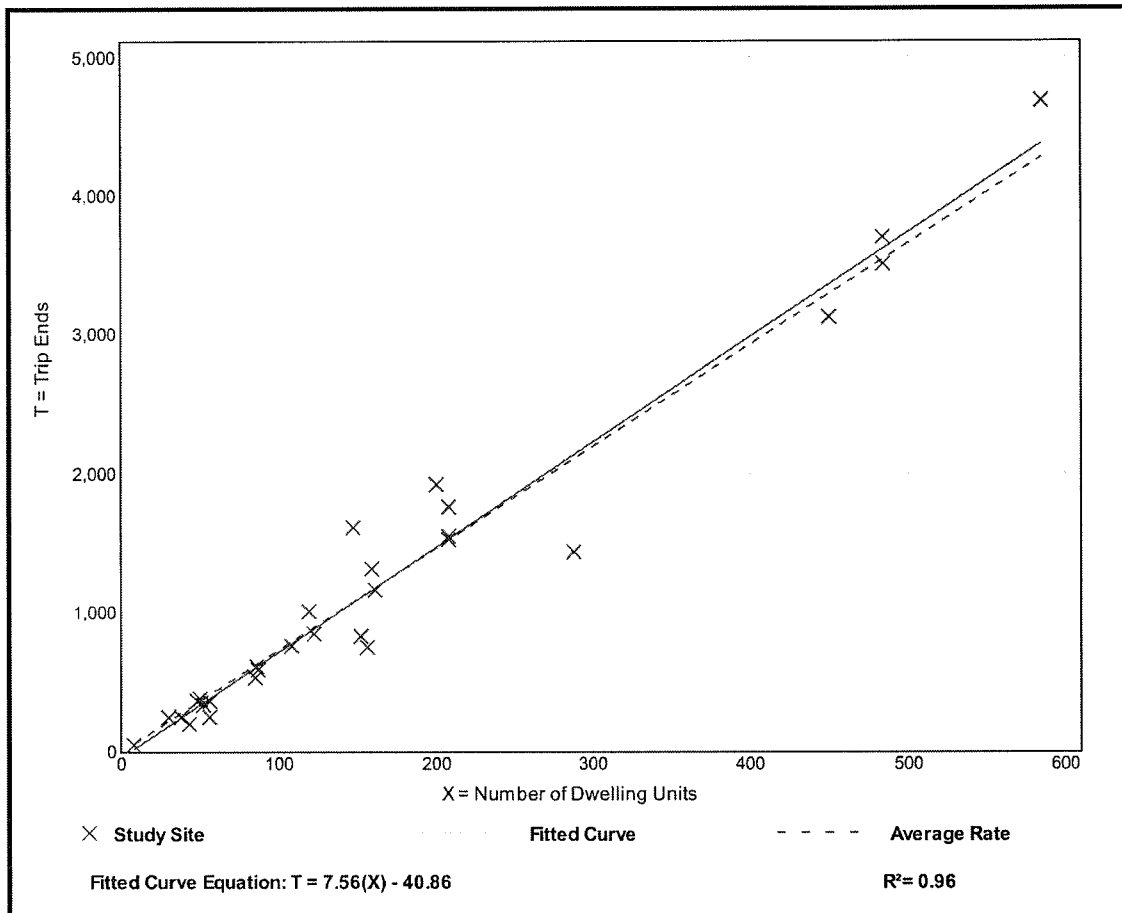
Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 29
Avg. Num. of Dwelling Units: 168
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
7.32	4.45 - 10.97	1.31

Data Plot and Equation



Multifamily Housing (Low-Rise)

(220)

Vehicle Trip Ends vs: Dwelling Units
 On a: Weekday,
 Peak Hour of Adjacent Street Traffic,
 One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 42

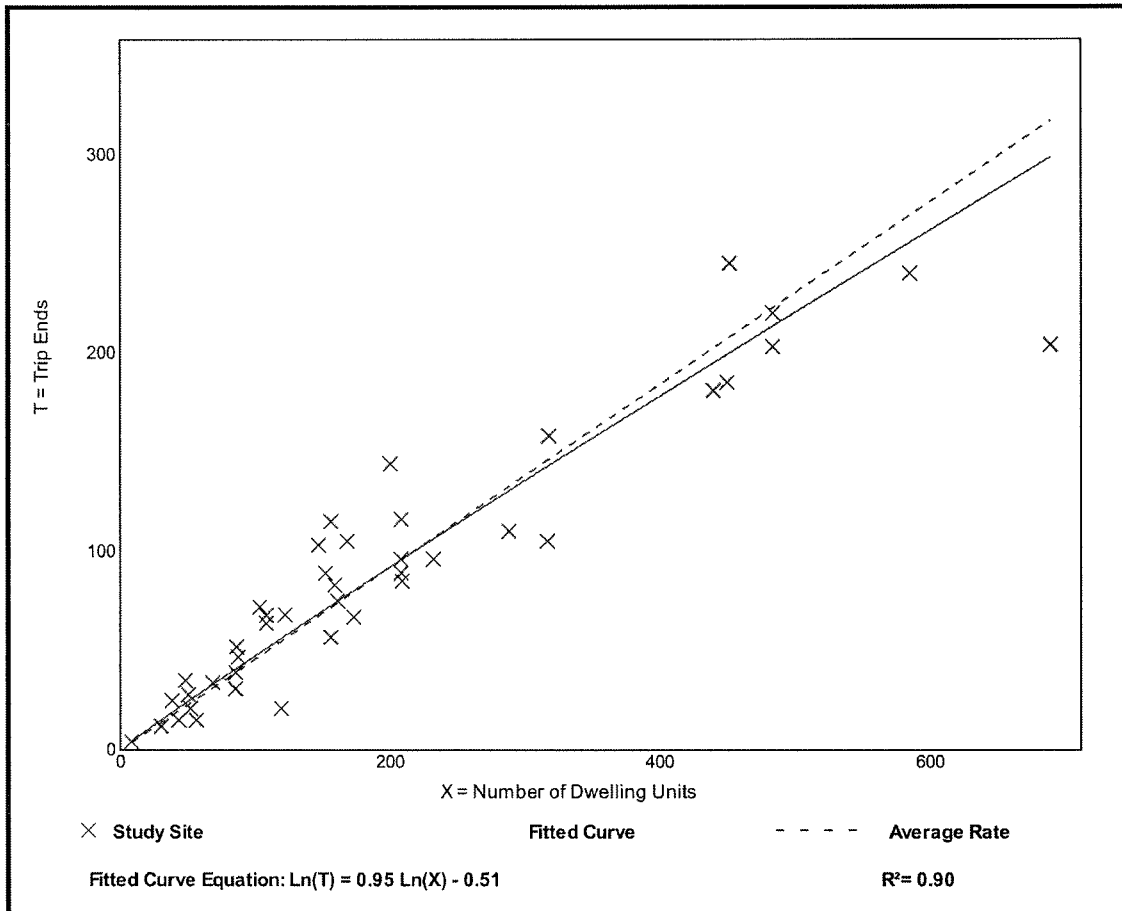
Avg. Num. of Dwelling Units: 199

Directional Distribution: 23% entering, 77% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.46	0.18 - 0.74	0.12

Data Plot and Equation



Multifamily Housing (Low-Rise) (220)

Vehicle Trip Ends vs: Dwelling Units
 On a: Weekday,
 Peak Hour of Adjacent Street Traffic,
 One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 50

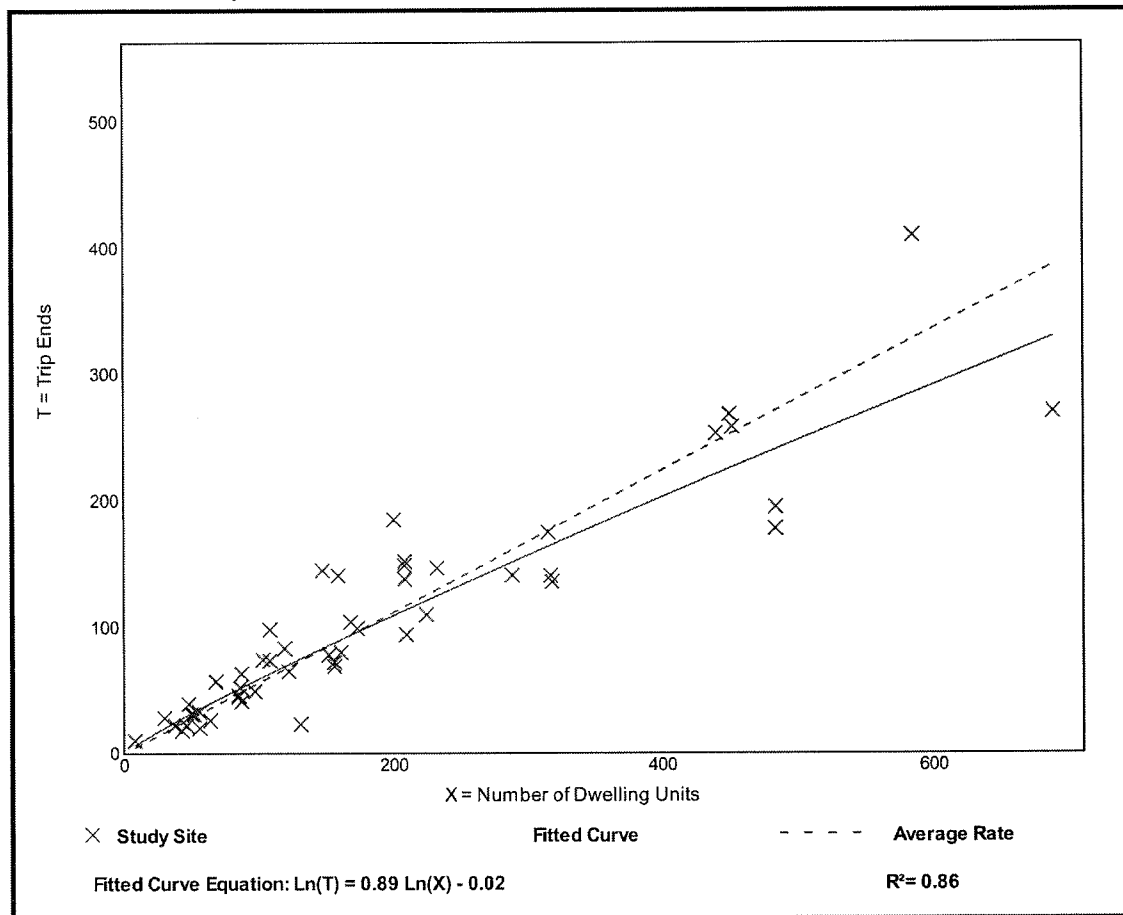
Avg. Num. of Dwelling Units: 187

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.56	0.18 - 1.25	0.16

Data Plot and Equation



Land Use: 710

General Office Building

Description

A general office building houses multiple tenants; it is a location where affairs of businesses, commercial or industrial organizations, or professional persons or firms are conducted. An office building or buildings may contain a mixture of tenants including professional services, insurance companies, investment brokers, and tenant services, such as a bank or savings and loan institution, a restaurant, or cafeteria and service retail facilities. A general office building with a gross floor area of 5,000 square feet or less is classified as a small office building (Land Use 712). Corporate headquarters building (Land Use 714), single tenant office building (Land Use 715), office park (Land Use 750), research and development center (Land Use 760), and business park (Land Use 770) are additional related uses.

If information is known about individual buildings, it is suggested that the general office building category be used rather than office parks when estimating trip generation for one or more office buildings in a single development. The office park category is more general and should be used when a breakdown of individual or different uses is not known. If the general office building category is used and if additional buildings, such as banks, restaurants, or retail stores are included in the development, the development should be treated as a multiuse project. On the other hand, if the office park category is used, internal trips are already reflected in the data and do not need to be considered.

When the buildings are interrelated (defined by shared parking facilities or the ability to easily walk between buildings) or house one tenant, it is suggested that the total area or employment of all the buildings be used for calculating the trip generation. When the individual buildings are isolated and not related to one another, it is suggested that trip generation be calculated for each building separately and then summed.

Additional Data

The average building occupancy varied considerably within the studies for which occupancy data were provided. The reported occupied gross floor area was 88 for general urban/suburban sites and 96 percent for the center city core and dense multi-use urban sites.

Time-of-day distribution data for this land use for a weekday, Saturday, and Sunday are presented in Appendix A. For the 16 general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 7:30 and 8:30 a.m. and 4:30 and 5:30 p.m., respectively.

For the three general urban/suburban sites with person trip data, the overall highest volumes during the AM and PM on a weekday were counted between 8:45 and 9:45 a.m. and 12:45 and 1:45 p.m., respectively. For the three dense multi-use urban sites with person trip data, the overall highest volumes during the AM and PM on a weekday were counted between 8:30 and 9:30 a.m. and 4:45 and 5:45 p.m., respectively. For the four center city core sites with person trip data, the overall highest volumes during the AM and PM on a weekday were counted between 9:00 and 10:00 a.m. and 12:45 and 1:45 p.m., respectively.

The average numbers of person trips per vehicle trip at the eight center city core sites at which both person trip and vehicle trip data were collected were as follows:

- 2.76 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 7 and 9 a.m.
- 2.90 during Weekday, AM Peak Hour of Generator
- 2.91 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 4 and 6 p.m.
- 3.02 during Weekday, PM Peak Hour of Generator

The average numbers of person trips per vehicle trip at the 18 dense multi-use urban sites at which both person trip and vehicle trip data were collected were as follows:

- 1.47 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 7 and 9 a.m.
- 1.47 during Weekday, AM Peak Hour of Generator
- 1.46 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 4 and 6 p.m.
- 1.53 during Weekday, PM Peak Hour of Generator

The average numbers of person trips per vehicle trip at the 23 general urban/suburban sites at which both person trip and vehicle trip data were collected were as follows:

- 1.30 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 7 and 9 a.m.
- 1.34 during Weekday, AM Peak Hour of Generator
- 1.32 during Weekday, Peak Hour of Adjacent Street Traffic, one hour between 4 and 6 p.m.
- 1.41 during Weekday, PM Peak Hour of Generator

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), California, Colorado, Connecticut, Georgia, Illinois, Indiana, Kansas, Kentucky, Maine, Maryland, Michigan, Minnesota, Missouri, Montana, New Hampshire, New Jersey, New York, Pennsylvania, Texas, Utah, Virginia, and Washington.

Source Numbers

161, 175, 183, 184, 185, 207, 212, 217, 247, 253, 257, 260, 262, 273, 279, 297, 298, 300, 301, 302, 303, 304, 321, 322, 323, 324, 327, 404, 407, 408, 418, 419, 423, 562, 734, 850, 859, 862, 867, 869, 883, 884, 890, 891, 904, 940, 944, 946, 964, 965, 972

General Office Building (710)

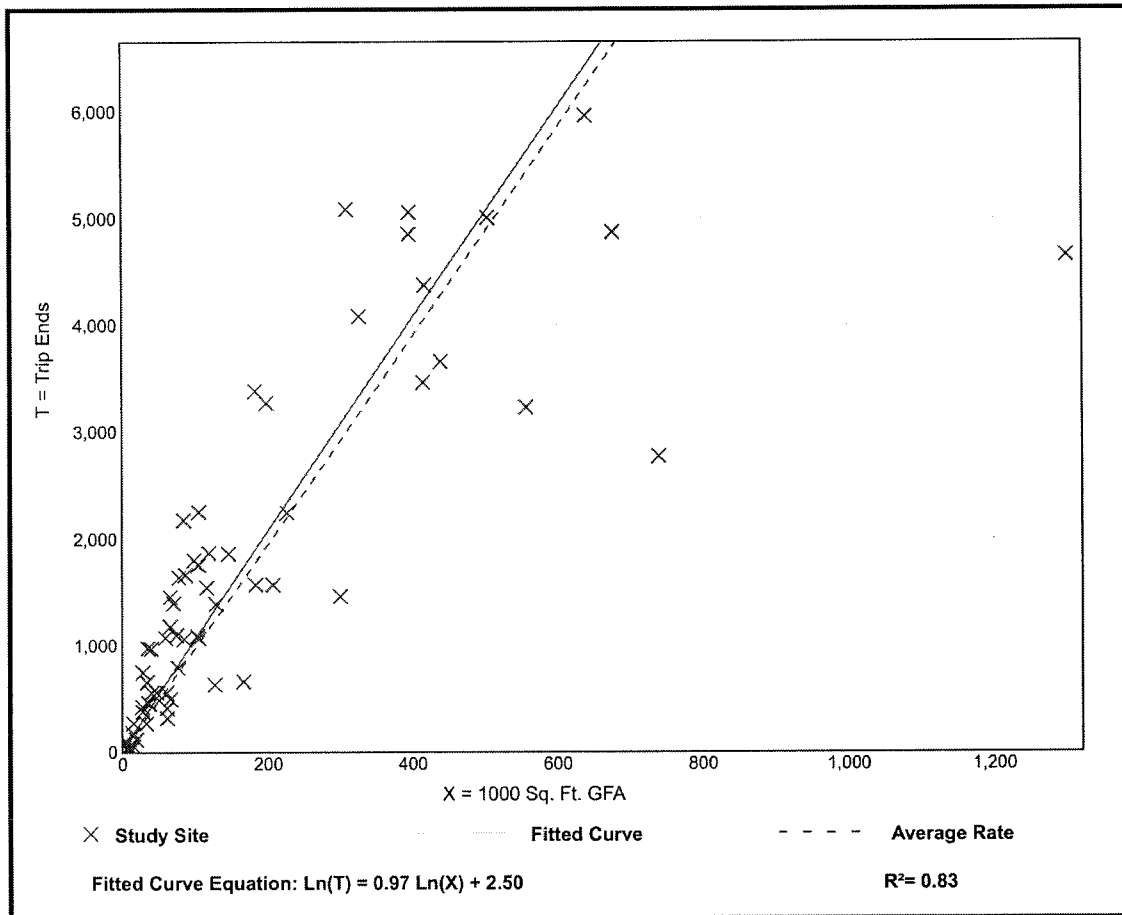
Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 66
1000 Sq. Ft. GFA: 171
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
9.74	2.71 - 27.56	5.15

Data Plot and Equation



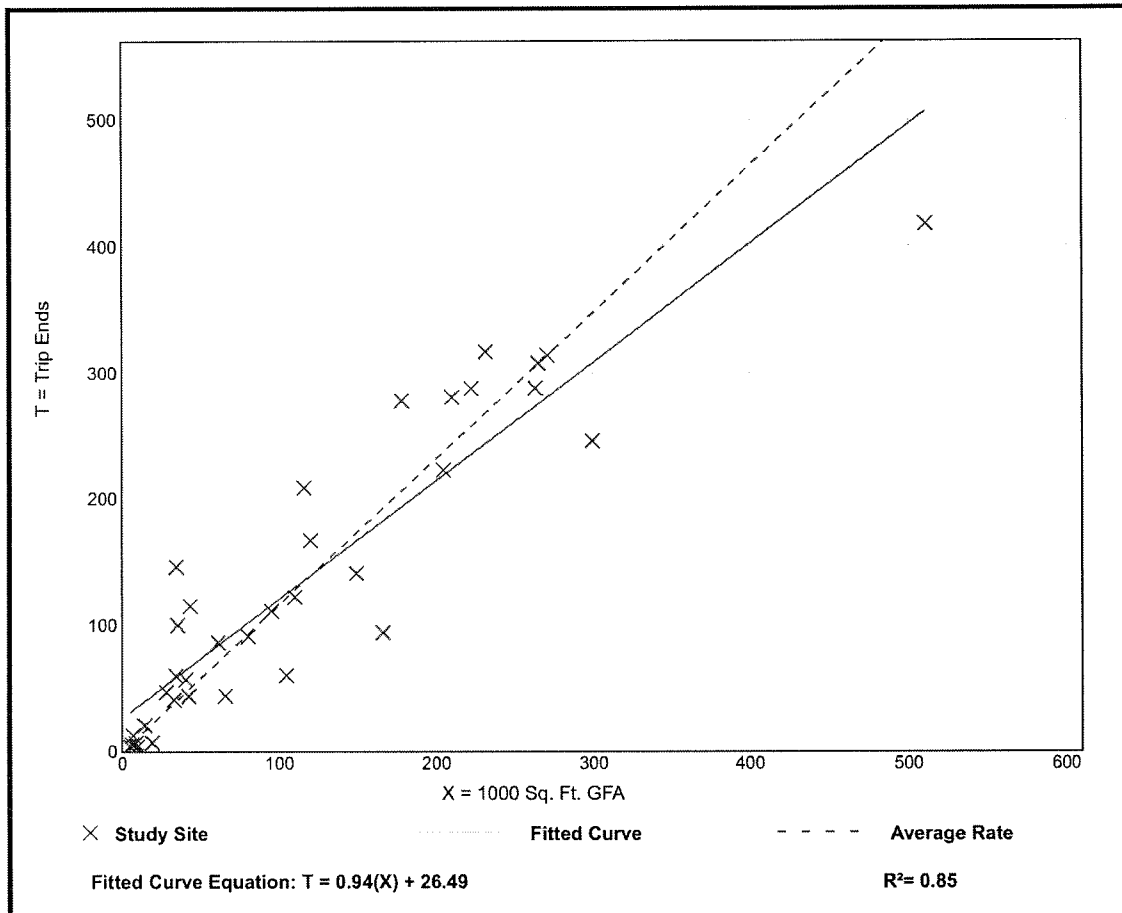
General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
 Peak Hour of Adjacent Street Traffic,
 One Hour Between 7 and 9 a.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 35
 1000 Sq. Ft. GFA: 117
 Directional Distribution: 86% entering, 14% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.16	0.37 - 4.23	0.47

Data Plot and Equation



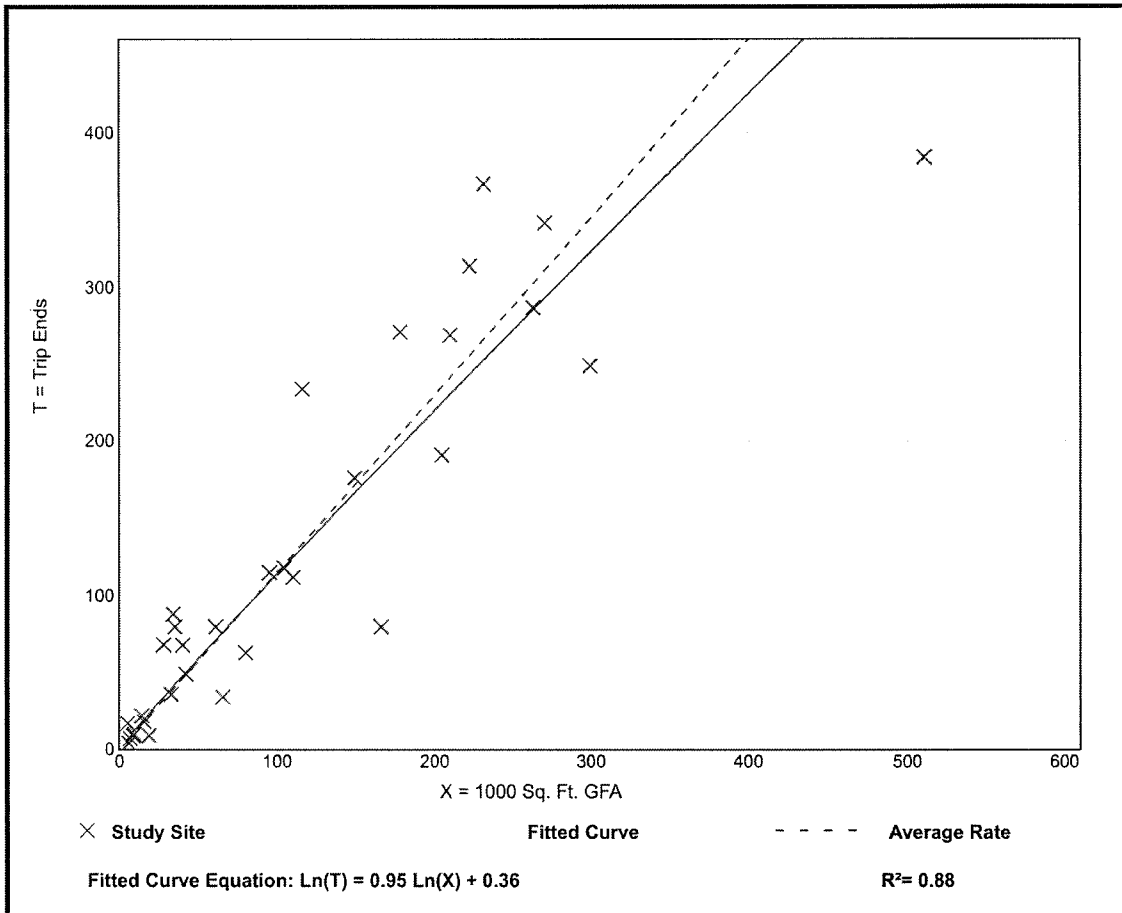
General Office Building (710)

Vehicle Trip Ends vs: 1000 Sq. Ft. GFA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 4 and 6 p.m.
Setting/Location: General Urban/Suburban
 Number of Studies: 32
 1000 Sq. Ft. GFA: 114
 Directional Distribution: 16% entering, 84% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GFA

Average Rate	Range of Rates	Standard Deviation
1.15	0.47 - 3.23	0.42

Data Plot and Equation



CMAP 2040 Projections Letter



Chicago Metropolitan Agency for Planning

233 South Wacker Drive
Suite 800
Chicago, Illinois 60606

312 454 0400
www.cmap.illinois.gov

August 19, 2020

Javier Millan
Principal
Kenig, Lindgren, O'Hara and Aboona, Inc.
9575 West Higgins Road
Suite 400
Rosemont, IL 60018

Subject: Naperville Road @ Warrenville Road
IDOT

Dear Mr. Millan:

In response to a request made on your behalf and dated August 18, 2020, we have developed year 2050 average daily traffic (ADT) projections for the subject location.

ROAD SEGMENT	Current Volumes	Year 2050 ADT
Naperville Rd north of Warrenville Rd	28,000	30,600
Naperville Rd south of Warrenville Rd	33,100	34,800
Warrenville Rd east of Naperville Rd	16,600	21,000
Warrenville Rd west of Naperville Rd	20,000	25,300

Traffic projections are developed using existing ADT data provided in the request letter and the results from the March 2020 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2050 socioeconomic projections and assumes the implementation of the ON TO 2050 Comprehensive Regional Plan for the Northeastern Illinois area. The provision of this data in support of your request does not constitute a CMAP endorsement of the proposed development or any subsequent developments.

If you have any questions, please call me at (312) 386-8806.

Sincerely,

Jose Rodriguez, PTP, AICP
Senior Planner, Research & Analysis

cc: Quigley (IDOT)
2020_TrafficForecast\Naperville\du-31-20\du-31-20.docx

Level of Service Criteria

LEVEL OF SERVICE CRITERIA

Signalized Intersections		
Level of Service	Interpretation	Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping.	≤10
B	Good progression, with more vehicles stopping than for Level of Service A.	>10 - 20
C	Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.	>20 - 35
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.	>35 - 55
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.	>55 - 80
F	The volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.	>80.0
Unsignalized Intersections		
Level of Service	Average Total Delay (SEC/VEH)	
A	0 - 10	
B	> 10 - 15	
C	> 15 - 25	
D	> 25 - 35	
E	> 35 - 50	
F	> 50	

Source: *Highway Capacity Manual*, 2010.

Capacity Analysis Summary Sheets

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	67	1144	43	201	728	18	434	42	618	6	3	5
Future Volume (vph)	67	1144	43	201	728	18	434	42	618	6	3	5
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			55			87						87
Link Speed (mph)		45			45			35				20
Link Distance (ft)		609			1506			1718				409
Travel Time (s)		9.2			22.8			33.5				13.9
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	71	1204	45	212	766	19	457	44	651	6	3	5
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	24.0	55.0	50.0	24.0	55.0	15.0	50.0	56.0		15.0	21.0	24.0
Total Split (%)	16.0%	36.7%	33.3%	16.0%	36.7%	10.0%	33.3%	37.3%		10.0%	14.0%	16.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	8.5	74.3	120.5	15.4	81.2	93.3	40.1	37.2	57.1	6.1	8.4	13.7
Actuated g/C Ratio	0.06	0.50	0.80	0.10	0.54	0.62	0.27	0.25	0.38	0.04	0.06	0.09
v/c Ratio	0.37	0.65	0.04	0.60	0.38	0.02	0.50	0.05	0.61	0.08	0.01	0.02
Control Delay	73.0	33.0	1.6	67.1	24.6	1.7	48.6	40.7	39.4	71.3	62.0	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.0	33.0	1.6	67.1	24.6	1.7	48.6	40.7	39.4	71.3	62.0	0.2
LOS	E	C	A	E	C	A	D	D	D	E	E	A
Approach Delay		34.1			33.2			43.1			43.9	
Approach LOS		C			C			D			D	
Queue Length 50th (ft)	35	486	0	108	337	0	177	17	287	6	1	0
Queue Length 95th (ft)	61	658	12	153	390	m5	273	32	316	22	6	0
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings

3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020



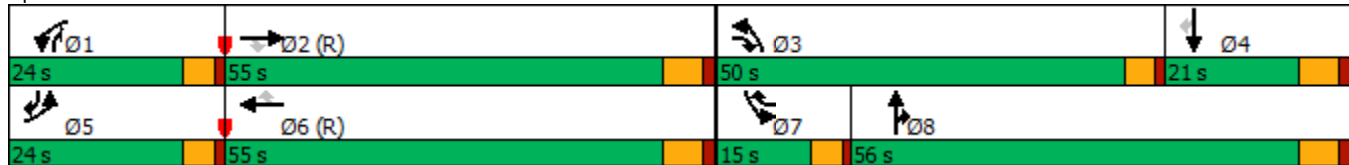
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	446	1845	1362	449	2016	1061	1098	1241	1139	123	396	333
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.65	0.03	0.47	0.38	0.02	0.42	0.04	0.57	0.05	0.01	0.02

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	148 (99%), Referenced to phase 2:EBT and 6:WBT, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.65
Intersection Signal Delay:	36.9
Intersection LOS:	D
Intersection Capacity Utilization	69.6%
ICU Level of Service	C
Analysis Period (min)	15































m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 	 	 	 	 	 	 	 			
Traffic Volume (vph)	383	1157	228	80	480	158	189	812	239	243	628	278
Future Volume (vph)	383	1157	228	80	480	158	189	812	239	243	628	278
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			55			87			87			113
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	403	1218	240	84	505	166	199	855	252	256	661	293
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	36.0	74.0	21.0	13.0	51.0	21.0	21.0	42.0	13.0	21.0	42.0	36.0
Total Split (%)	24.0%	49.3%	14.0%	8.7%	34.0%	14.0%	14.0%	28.0%	8.7%	14.0%	28.0%	24.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.2	59.0	78.9	8.0	43.9	65.6	13.9	46.2	60.2	15.8	48.1	77.2
Actuated g/C Ratio	0.15	0.39	0.53	0.05	0.29	0.44	0.09	0.31	0.40	0.11	0.32	0.51
v/c Ratio	0.76	0.83	0.28	0.46	0.46	0.22	0.63	0.75	0.37	0.71	0.39	0.34
Control Delay	58.9	44.7	13.3	77.0	44.7	12.1	74.2	52.5	23.5	72.9	39.4	12.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	58.9	44.7	13.3	77.0	44.7	12.1	74.2	52.5	23.5	72.9	39.4	12.4
LOS	E	D	B	E	D	B	E	D	C	E	D	B
Approach Delay		43.7			41.1			50.2			40.0	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	203	572	124	41	211	44	98	403	111	126	186	99
Queue Length 95th (ft)	220	617	132	71	263	90	139	#571	207	161	251	199
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

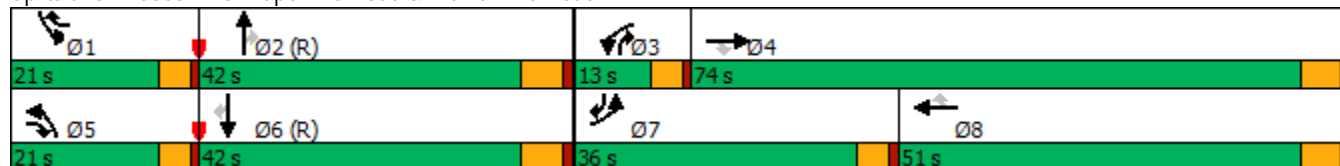


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	720	1688	887	194	1128	754	381	1147	692	389	1715	951
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.56	0.72	0.27	0.43	0.45	0.22	0.52	0.75	0.36	0.66	0.39	0.31

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 136 (91%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 44.1
 Intersection LOS: D
 Intersection Capacity Utilization 80.3%
 ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	2	1	1	2	2	10	22	1268	63	174	1146	28
Future Volume (vph)	2	1	1	2	2	10	22	1268	63	174	1146	28
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.925				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1723	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted							0.228			0.175		
Satd. Flow (perm)	1863	1723	0	1863	1961	1583	425	3725	1583	326	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				51			80			80
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	2	2	0	2	2	11	23	1335	66	183	1206	29
Turn Type	pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	13.5	24.0		13.5	24.0	15.0	15.0	97.5	97.5	15.0	97.5	97.5
Total Split (%)	9.0%	16.0%		9.0%	16.0%	10.0%	10.0%	65.0%	65.0%	10.0%	65.0%	65.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	7.4	5.7		7.4	5.8	11.2	134.4	126.7	126.7	141.1	136.0	136.0
Actuated g/C Ratio	0.05	0.04		0.05	0.04	0.07	0.90	0.84	0.84	0.94	0.91	0.91
v/c Ratio	0.02	0.03		0.02	0.03	0.07	0.05	0.42	0.05	0.47	0.36	0.02
Control Delay	63.5	59.0		63.5	70.0	0.8	2.0	6.8	2.2	4.9	2.7	0.0
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.5	59.0		63.5	70.0	0.8	2.0	6.8	2.2	4.9	2.7	0.0
LOS	E	E		E	E	A	A	A	A	A	A	A
Approach Delay		61.3			18.4			6.6			2.9	
Approach LOS		E			B			A			A	
Queue Length 50th (ft)	2	1		2	2	0	0	222	1	0	62	0
Queue Length 95th (ft)	11	11		11	12	0	m8	330	m13	44	251	0
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	133	207		133	235	195	493	3146	1349	420	3378	1443
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.01		0.02	0.01	0.06	0.05	0.42	0.05	0.44	0.36	0.02

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	100 (67%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.47
Intersection Signal Delay:	4.9
Intersection LOS:	A
Intersection Capacity Utilization	60.4%
ICU Level of Service	B
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

Ø1	Ø2 (R)	Ø3	Ø4
15 s	97.5 s	13.5 s	24 s
Ø5	Ø6 (R)	Ø7	Ø8
15 s	97.5 s	13.5 s	24 s

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	5	1112	171	567	989	4	213	8	504	55	31	79
Future Volume (vph)	5	1112	171	567	989	4	213	8	504	55	31	79
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			129			55						87
Link Speed (mph)		45			45			35				20
Link Distance (ft)		609			1506			1718				409
Travel Time (s)		9.2			22.8			33.5				13.9
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	5	1171	180	597	1041	4	224	8	531	58	33	83
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	15.0	63.0	15.0	43.5	91.5	15.0	15.0	28.5		15.0	28.5	15.0
Total Split (%)	10.0%	42.0%	10.0%	29.0%	61.0%	10.0%	10.0%	19.0%		10.0%	19.0%	10.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	5.8	66.5	85.7	32.5	93.2	108.5	13.2	22.9	59.9	9.2	19.1	28.6
Actuated g/C Ratio	0.04	0.44	0.57	0.22	0.62	0.72	0.09	0.15	0.40	0.06	0.13	0.19
v/c Ratio	0.04	0.71	0.19	0.80	0.45	0.00	0.74	0.01	0.48	0.54	0.07	0.22
Control Delay	69.8	38.6	6.7	52.6	29.7	0.0	82.2	54.5	34.5	85.8	55.2	9.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	69.8	38.6	6.7	52.6	29.7	0.0	82.2	54.5	34.5	85.8	55.2	9.3
LOS	E	D	A	D	C	A	F	D	C	F	E	A
Approach Delay		34.5			38.0			48.7				43.5
Approach LOS		C			D			D				D
Queue Length 50th (ft)	2	505	24	289	416	0	114	3	217	56	14	0
Queue Length 95th (ft)	10	636	70	353	444	m0	#199	12	257	106	32	42
Internal Link Dist (ft)		529			1426			1638				329

Lanes, Volumes, Timings

3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	240	1651	959	892	2315	1172	301	568	1211	123	558	418
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.02	0.71	0.19	0.67	0.45	0.00	0.74	0.01	0.44	0.47	0.06	0.20

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 50 (33%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 39.1 Intersection LOS: D
 Intersection Capacity Utilization 71.9% ICU Level of Service C
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road

Ø1	Ø2 (R)	Ø3	Ø4
43.5 s	63 s	15 s	28.5 s
Ø5	Ø6 (R)	Ø7	Ø8
15 s	91.5 s	15 s	28.5 s

Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	504	587	580	411	993	179	209	642	54	178	1005	358
Future Volume (vph)	504	587	580	411	993	179	209	642	54	178	1005	358
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			87			87			57			120
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	531	618	611	433	1045	188	220	676	57	187	1058	377
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	28.0	51.0	23.0	39.0	62.0	17.0	23.0	43.0	39.0	17.0	37.0	28.0
Total Split (%)	18.7%	34.0%	15.3%	26.0%	41.3%	11.3%	15.3%	28.7%	26.0%	11.3%	24.7%	18.7%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.5	48.9	70.1	24.2	49.6	68.1	15.2	43.4	73.6	12.5	40.8	70.3
Actuated g/C Ratio	0.16	0.33	0.47	0.16	0.33	0.45	0.10	0.29	0.49	0.08	0.27	0.47
v/c Ratio	0.99	0.51	0.78	0.78	0.85	0.25	0.63	0.63	0.07	0.65	0.73	0.47
Control Delay	108.8	29.6	32.2	70.6	53.9	12.8	72.9	50.6	5.0	69.2	48.5	29.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	108.8	29.6	32.2	70.6	53.9	12.8	72.9	50.6	5.0	69.2	48.5	29.8
LOS	F	C	C	E	D	B	E	D	A	E	D	C
Approach Delay		54.4			53.6			53.0			46.5	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	284	182	267	212	500	56	107	307	0	92	351	237
Queue Length 95th (ft)	#401	210	462	262	552	101	151	396	25	135	#473	396
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings
 6: Naperville Road & Warrenville Road

09/09/2020

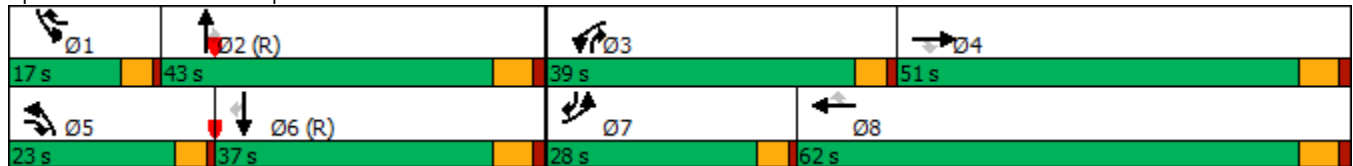


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	537	1213	819	789	1390	772	423	1077	910	301	1454	805
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.99	0.51	0.75	0.55	0.75	0.24	0.52	0.63	0.06	0.62	0.73	0.47

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 5 (3%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 80
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.99
 Intersection Signal Delay: 51.8
 Intersection LOS: D
 Intersection Capacity Utilization 82.4%
 ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	37	13	66	32	0	179	7	1312	6	28	1443	3
Future Volume (vph)	37	13	66	32	0	179	7	1312	6	28	1443	3
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.875				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1630	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted	0.784						0.140			0.135		
Satd. Flow (perm)	1460	1630	0	1863	1961	1583	261	3725	1583	251	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		69				147			80			80
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	39	83	0	34	0	188	7	1381	6	29	1519	3
Turn Type	pm+pt	NA		pm+pt		pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	16.5	24.0		16.5	24.0	13.5	13.5	96.0	96.0	13.5	96.0	96.0
Total Split (%)	11.0%	16.0%		11.0%	16.0%	9.0%	9.0%	64.0%	64.0%	9.0%	64.0%	64.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	20.2	7.8		8.7		12.7	113.1	105.4	105.4	121.8	117.8	117.8
Actuated g/C Ratio	0.13	0.05		0.06		0.08	0.75	0.70	0.70	0.81	0.79	0.79
v/c Ratio	0.16	0.56		0.33		0.70	0.03	0.53	0.01	0.09	0.52	0.00
Control Delay	55.2	33.9		75.8		31.8	11.3	31.1	0.0	4.5	7.9	0.0
Queue Delay	0.0	0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.2	33.9		75.8		31.8	11.3	31.1	0.0	4.5	7.9	0.0
LOS	E	C		E		C	B	C	A	A	A	A
Approach Delay		40.7			38.5			30.9			7.8	
Approach LOS		D			D			C			A	
Queue Length 50th (ft)	34	13		33		39	3	664	0	5	235	0
Queue Length 95th (ft)	67	68		70		122	m5	m780	m0	15	471	0
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	250	256		148		282	299	2618	1136	323	2925	1260
Starvation Cap Reductn	0	0		0		0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0		0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0		0	0	0	0	0	0	0
Reduced v/c Ratio	0.16	0.32		0.23		0.67	0.02	0.53	0.01	0.09	0.52	0.00

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	81 (54%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.70
Intersection Signal Delay:	20.9
Intersection LOS:	C
Intersection Capacity Utilization	61.4%
ICU Level of Service	B
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

Ø1	Ø2 (R)	Ø3	Ø4
13.5 s	96 s	16.5 s	24 s
Ø5	Ø6 (R)	Ø7	Ø8
13.5 s	96 s	16.5 s	24 s

Lanes, Volumes, Timings 3: Freedom Drive/Lucent Lane & Warrenville Road

09/14/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	75	1144	43	201	733	25	434	50	618	26	21	25
Future Volume (vph)	75	1144	43	201	733	25	434	50	618	26	21	25
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			55			87						87
Link Speed (mph)		45			45			35				20
Link Distance (ft)		609			1506			1718				409
Travel Time (s)		9.2			22.8			33.5				13.9
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	79	1204	45	212	772	26	457	53	651	27	22	26
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	24.0	55.0	50.0	24.0	55.0	15.0	50.0	56.0		15.0	21.0	24.0
Total Split (%)	16.0%	36.7%	33.3%	16.0%	36.7%	10.0%	33.3%	37.3%		10.0%	14.0%	16.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	8.8	70.6	109.1	15.4	77.2	91.0	32.5	37.2	57.1	7.8	15.1	25.3
Actuated g/C Ratio	0.06	0.47	0.73	0.10	0.51	0.61	0.22	0.25	0.38	0.05	0.10	0.17
v/c Ratio	0.39	0.69	0.04	0.60	0.40	0.03	0.62	0.06	0.61	0.30	0.06	0.08
Control Delay	73.2	36.0	2.0	68.6	27.6	3.4	57.9	41.2	39.5	76.0	56.9	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.2	36.0	2.0	68.6	27.6	3.4	57.9	41.2	39.5	76.0	56.9	0.4
LOS	E	D	A	E	C	A	E	D	D	E	E	A
Approach Delay		37.1			35.6			46.8			44.2	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	39	496	0	108	340	1	223	21	288	26	10	0
Queue Length 95th (ft)	67	679	13	153	394	m10	273	37	318	60	24	0
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings

3: Freedom Drive/Lucent Lane & Warrenville Road

09/14/2020

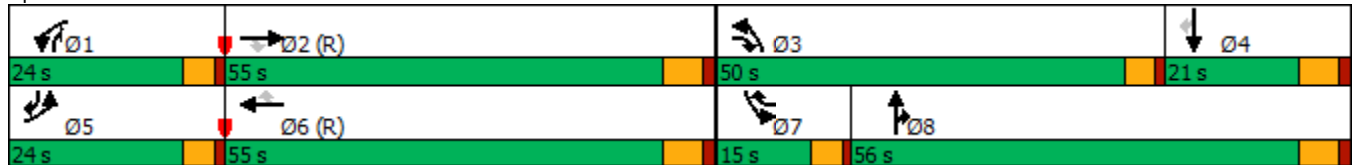


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	446	1753	1302	449	1917	1021	1049	1241	1138	123	468	445
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.69	0.03	0.47	0.40	0.03	0.44	0.04	0.57	0.22	0.05	0.06

Intersection Summary

























Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 148 (99%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.69
 Intersection Signal Delay: 40.0 Intersection LOS: D
 Intersection Capacity Utilization 69.6% ICU Level of Service C
 Analysis Period (min) 15
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/14/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	383	1165	240	80	485	161	191	816	239	259	640	283
Future Volume (vph)	383	1165	240	80	485	161	191	816	239	259	640	283
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			55			87			87			110
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	403	1226	253	84	511	169	201	859	252	273	674	298
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	36.0	74.0	21.0	13.0	51.0	21.0	21.0	42.0	13.0	21.0	42.0	36.0
Total Split (%)	24.0%	49.3%	14.0%	8.7%	34.0%	14.0%	14.0%	28.0%	8.7%	14.0%	28.0%	24.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.2	59.3	79.3	8.0	44.2	66.4	13.9	45.4	59.4	16.2	47.7	76.9
Actuated g/C Ratio	0.15	0.40	0.53	0.05	0.29	0.44	0.09	0.30	0.40	0.11	0.32	0.51
v/c Ratio	0.76	0.83	0.29	0.46	0.47	0.23	0.63	0.76	0.37	0.74	0.40	0.34
Control Delay	57.1	46.3	12.3	77.0	44.5	12.1	74.4	53.7	23.8	72.5	38.1	11.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	57.1	46.3	12.3	77.0	44.5	12.1	74.4	53.7	23.8	72.5	38.1	11.2
LOS	E	D	B	E	D	B	E	D	C	E	D	B
Approach Delay		44.0			40.9			51.1			39.2	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	198	587	132	41	213	45	99	409	113	135	190	107
Queue Length 95th (ft)	218	640	135	71	266	93	140	#575	207	163	257	202
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings
 6: Naperville Road & Warrenville Road

09/14/2020

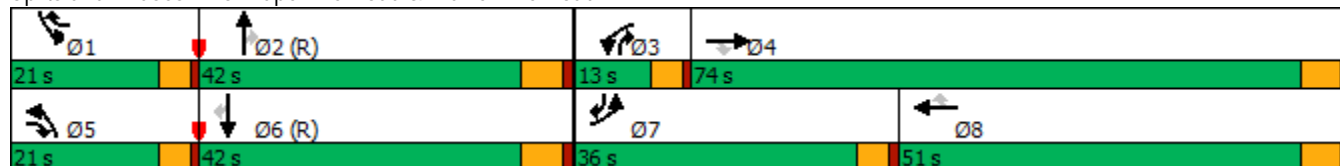


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	720	1688	889	194	1132	759	380	1128	684	394	1702	946
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.56	0.73	0.28	0.43	0.45	0.22	0.53	0.76	0.37	0.69	0.40	0.32

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 136 (91%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.83
 Intersection Signal Delay: 44.2 Intersection LOS: D
 Intersection Capacity Utilization 81.1% ICU Level of Service D
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/14/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	33	1	34	2	2	10	29	1268	63	174	1146	38
Future Volume (vph)	33	1	34	2	2	10	29	1268	63	174	1146	38
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.854				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1591	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted	0.755						0.221			0.166		
Satd. Flow (perm)	1406	1591	0	1863	1961	1583	412	3725	1583	309	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		36				51			80			80
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	35	37	0	2	2	11	31	1335	66	183	1206	40
Turn Type	pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	13.5	24.0		13.5	24.0	15.0	15.0	97.5	97.5	15.0	97.5	97.5
Total Split (%)	9.0%	16.0%		9.0%	16.0%	10.0%	10.0%	65.0%	65.0%	10.0%	65.0%	65.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	10.4	6.9		7.4	5.8	11.4	126.3	118.4	118.4	132.9	126.5	126.5
Actuated g/C Ratio	0.07	0.05		0.05	0.04	0.08	0.84	0.79	0.79	0.89	0.84	0.84
v/c Ratio	0.30	0.35		0.02	0.03	0.07	0.08	0.45	0.05	0.51	0.38	0.03
Control Delay	70.2	29.5		61.5	70.0	0.8	2.8	10.0	3.3	6.7	4.6	0.1
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	70.2	29.5		61.5	70.0	0.8	2.8	10.0	3.3	6.7	4.6	0.1
LOS	E	C		E	E	A	A	B	A	A	A	A
Approach Delay		49.2			18.1			9.6			4.8	
Approach LOS		D			B			A			A	
Queue Length 50th (ft)	34	1		2	2	0	2	253	7	15	143	0
Queue Length 95th (ft)	66	39		10	12	0	m12	332	m12	51	277	1
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/14/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	133	222		133	235	195	459	2940	1266	388	3142	1348
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.26	0.17		0.02	0.01	0.06	0.07	0.45	0.05	0.47	0.38	0.03

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	100 (67%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.51
Intersection Signal Delay:	8.3
Intersection LOS:	A
Intersection Capacity Utilization	64.8%
ICU Level of Service	C
Analysis Period (min)	15

























m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

Ø1	Ø2 (R)	Ø3	Ø4
15 s	97.5 s	13.5 s	24 s
Ø5	Ø6 (R)	Ø7	Ø8
15 s	97.5 s	13.5 s	24 s

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

09/14/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	33	1112	171	567	992	28	213	35	504	68	43	92
Future Volume (vph)	33	1112	171	567	992	28	213	35	504	68	43	92
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			129			55						87
Link Speed (mph)		45			45			35			20	
Link Distance (ft)		609			1506			1718			409	
Travel Time (s)		9.2			22.8			33.5			13.9	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	35	1171	180	597	1044	29	224	37	531	72	45	97
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	15.0	63.0	15.0	43.5	91.5	15.0	15.0	28.5		15.0	28.5	15.0
Total Split (%)	10.0%	42.0%	10.0%	29.0%	61.0%	10.0%	10.0%	19.0%		10.0%	19.0%	10.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	7.0	64.2	85.7	32.5	89.8	105.4	15.5	22.6	59.6	9.6	19.1	29.8
Actuated g/C Ratio	0.05	0.43	0.57	0.22	0.60	0.70	0.10	0.15	0.40	0.06	0.13	0.20
v/c Ratio	0.22	0.73	0.19	0.80	0.47	0.03	0.63	0.07	0.48	0.64	0.09	0.25
Control Delay	71.7	40.2	6.7	53.8	31.9	2.3	74.2	55.1	34.7	92.9	55.7	12.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.7	40.2	6.7	53.8	31.9	2.3	74.2	55.1	34.7	92.9	55.7	12.3
LOS	E	D	A	D	C	A	E	E	C	F	E	B
Approach Delay		36.7			39.2			46.8			48.6	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	17	505	24	289	421	1	114	16	217	70	20	8
Queue Length 95th (ft)	37	636	70	355	457	m3	#199	34	257	#130	40	55
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings

3: Freedom Drive/Lucent Lane & Warrenville Road

09/14/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	240	1594	959	892	2229	1137	353	563	1207	123	558	418
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.73	0.19	0.67	0.47	0.03	0.63	0.07	0.44	0.59	0.08	0.23

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 50 (33%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.80
 Intersection Signal Delay: 40.3 Intersection LOS: D
 Intersection Capacity Utilization 71.9% ICU Level of Service C
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road

Ø1	Ø2 (R)	Ø3	Ø4
43.5 s	63 s	15 s	28.5 s
Ø5	Ø6 (R)	Ø7	Ø8
15 s	91.5 s	15 s	28.5 s

Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/14/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	504	592	588	411	1011	188	215	656	54	189	1013	361
Future Volume (vph)	504	592	588	411	1011	188	215	656	54	189	1013	361
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			87			87			57			120
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	531	623	619	433	1064	198	226	691	57	199	1066	380
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	28.0	51.0	23.0	39.0	62.0	17.0	23.0	43.0	39.0	17.0	37.0	28.0
Total Split (%)	18.7%	34.0%	15.3%	26.0%	41.3%	11.3%	15.3%	28.7%	26.0%	11.3%	24.7%	18.7%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.5	49.4	70.8	24.2	50.1	68.8	15.4	42.7	72.9	12.7	40.0	69.5
Actuated g/C Ratio	0.16	0.33	0.47	0.16	0.33	0.46	0.10	0.28	0.49	0.08	0.27	0.46
v/c Ratio	0.99	0.51	0.78	0.78	0.86	0.26	0.64	0.65	0.07	0.68	0.75	0.48
Control Delay	111.3	28.5	30.8	70.6	54.0	13.4	73.0	51.7	5.0	68.3	47.6	28.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	111.3	28.5	30.8	70.6	54.0	13.4	73.0	51.7	5.0	68.3	47.6	28.7
LOS	F	C	C	E	D	B	E	D	A	E	D	C
Approach Delay		54.1			53.5			53.9			45.8	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	284	185	276	212	508	61	110	319	0	98	358	251
Queue Length 95th (ft)	#402	215	476	262	565	109	154	406	25	140	#480	397
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings

6: Naperville Road & Warrenville Road

09/14/2020

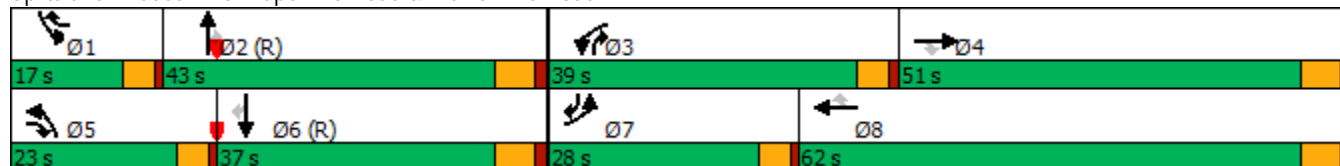


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	537	1226	824	789	1390	778	423	1059	903	302	1428	798
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.99	0.51	0.75	0.55	0.77	0.25	0.53	0.65	0.06	0.66	0.75	0.48

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 5 (3%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.99
 Intersection Signal Delay: 51.7 Intersection LOS: D
 Intersection Capacity Utilization 83.2% ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/14/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	57	13	88	32	0	179	30	1312	6	28	1443	37
Future Volume (vph)	57	13	88	32	0	179	30	1312	6	28	1443	37
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.870				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1621	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted	0.784						0.132			0.134		
Satd. Flow (perm)	1460	1621	0	1863	1961	1583	246	3725	1583	250	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		93				133			80			80
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	60	107	0	34	0	188	32	1381	6	29	1519	39
Turn Type	pm+pt	NA		pm+pt		pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	16.5	24.0		16.5	24.0	13.5	13.5	96.0	96.0	13.5	96.0	96.0
Total Split (%)	11.0%	16.0%		11.0%	16.0%	9.0%	9.0%	64.0%	64.0%	9.0%	64.0%	64.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	20.5	8.1		8.7		13.3	112.6	104.5	104.5	121.5	113.2	113.2
Actuated g/C Ratio	0.14	0.05		0.06		0.09	0.75	0.70	0.70	0.81	0.75	0.75
v/c Ratio	0.25	0.61		0.33		0.72	0.13	0.53	0.01	0.09	0.54	0.03
Control Delay	57.3	31.7		75.8		37.3	11.8	32.5	0.0	4.6	10.2	0.1
Queue Delay	0.0	0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	57.3	31.7		75.8		37.3	11.8	32.5	0.0	4.6	10.2	0.1
LOS	E	C		E		D	B	C	A	A	B	A
Approach Delay		40.9			43.2			31.9			9.8	
Approach LOS		D			D			C			A	
Queue Length 50th (ft)	53	13		33		53	12	680	0	5	329	0
Queue Length 95th (ft)	93	75		70		139	m23	m779	m0	15	493	0
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/14/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	253	276		148		273	286	2594	1126	326	2812	1214
Starvation Cap Reductn	0	0		0		0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0		0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0		0	0	0	0	0	0	0
Reduced v/c Ratio	0.24	0.39		0.23		0.69	0.11	0.53	0.01	0.09	0.54	0.03

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	81 (54%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	80
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.72
Intersection Signal Delay:	22.8
Intersection LOS:	C
Intersection Capacity Utilization	61.4%
ICU Level of Service	B
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

13.5 s	96 s	16.5 s	24 s
13.5 s	96 s	16.5 s	24 s

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	285	1181	44	207	751	182	448	261	638	42	29	41
Future Volume (vph)	285	1181	44	207	751	182	448	261	638	42	29	41
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			55			192						87
Link Speed (mph)		45			45			35			20	
Link Distance (ft)		609			1506			1718			409	
Travel Time (s)		9.2			22.8			33.5			13.9	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	300	1243	46	218	791	192	472	275	672	44	31	43
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	24.0	55.0	50.0	24.0	55.0	15.0	50.0	56.0		15.0	21.0	24.0
Total Split (%)	16.0%	36.7%	33.3%	16.0%	36.7%	10.0%	33.3%	37.3%		10.0%	14.0%	16.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	18.3	65.4	101.6	15.7	62.9	77.5	30.2	39.2	59.4	8.7	20.0	42.0
Actuated g/C Ratio	0.12	0.44	0.68	0.10	0.42	0.52	0.20	0.26	0.40	0.06	0.13	0.28
v/c Ratio	0.72	0.77	0.04	0.61	0.51	0.21	0.68	0.28	0.61	0.43	0.06	0.09
Control Delay	73.1	41.1	2.2	68.2	38.4	14.6	61.9	44.2	37.8	80.7	54.0	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.1	41.1	2.2	68.2	38.4	14.6	61.9	44.2	37.8	80.7	54.0	0.3
LOS	E	D	A	E	D	B	E	D	D	F	D	A
Approach Delay		46.0			40.0			47.1			44.4	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	147	541	0	112	369	59	230	113	294	42	13	0
Queue Length 95th (ft)	193	#778	14	152	468	135	280	146	318	85	30	0
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

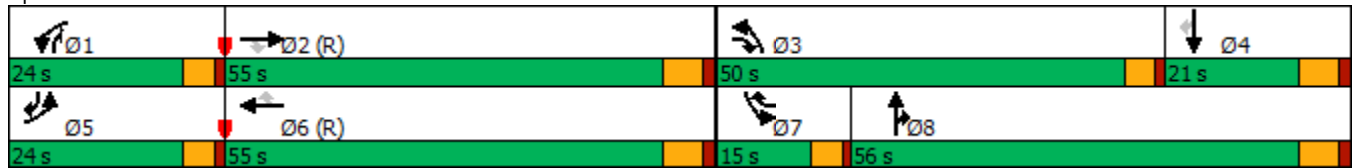


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	465	1624	1246	451	1561	927	1041	1241	1170	123	542	525
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.65	0.77	0.04	0.48	0.51	0.21	0.45	0.22	0.57	0.36	0.06	0.08

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 148 (99%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.77
 Intersection Signal Delay: 44.7
 Intersection LOS: D
 Intersection Capacity Utilization 71.4%
 ICU Level of Service C
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	285	1181	44	207	751	182	448	261	638	42	29	41
Future Volume (vph)	285	1181	44	207	751	182	448	261	638	42	29	41
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	2		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	0.97	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			55			192						87
Link Speed (mph)		45			45			35			20	
Link Distance (ft)		609			1506			1718			409	
Travel Time (s)		9.2			22.8			33.5			13.9	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	300	1243	46	218	791	192	472	275	672	44	31	43
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	24.0	55.0	50.0	24.0	55.0	15.0	50.0	56.0		15.0	21.0	24.0
Total Split (%)	16.0%	36.7%	33.3%	16.0%	36.7%	10.0%	33.3%	37.3%		10.0%	14.0%	16.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	18.3	66.6	102.8	15.7	64.0	77.3	30.2	39.4	59.6	7.3	18.8	40.8
Actuated g/C Ratio	0.12	0.44	0.69	0.10	0.43	0.52	0.20	0.26	0.40	0.05	0.13	0.27
v/c Ratio	0.72	0.75	0.04	0.61	0.50	0.21	0.68	0.28	0.61	0.26	0.07	0.09
Control Delay	73.1	39.8	2.2	67.6	37.2	14.2	61.9	44.0	37.7	72.1	55.2	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.1	39.8	2.2	67.6	37.2	14.2	61.9	44.0	37.7	72.1	55.2	0.3
LOS	E	D	A	E	D	B	E	D	D	E	E	A
Approach Delay		45.0			39.1			46.9			41.5	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	147	530	0	111	361	59	230	113	293	21	14	0
Queue Length 95th (ft)	193	#764	14	154	464	130	280	145	317	43	30	0
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020

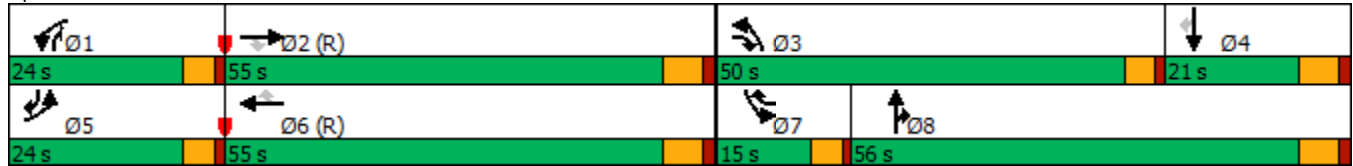


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	465	1653	1257	451	1589	938	1041	1241	1172	240	513	513
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.65	0.75	0.04	0.48	0.50	0.20	0.45	0.22	0.57	0.18	0.06	0.08

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	148 (99%), Referenced to phase 2:EBT and 6:WBT, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.75
Intersection Signal Delay:	43.9
Intersection LOS:	D
Intersection Capacity Utilization	71.2%
ICU Level of Service	C
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	395	1213	253	83	603	272	250	947	247	269	666	287
Future Volume (vph)	395	1213	253	83	603	272	250	947	247	269	666	287
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			55			87			87			59
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	416	1277	266	87	635	286	263	997	260	283	701	302
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	36.0	74.0	21.0	13.0	51.0	21.0	21.0	42.0	13.0	21.0	42.0	36.0
Total Split (%)	24.0%	49.3%	14.0%	8.7%	34.0%	14.0%	14.0%	28.0%	8.7%	14.0%	28.0%	24.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.7	60.9	82.7	8.0	45.2	67.7	15.8	43.5	57.6	16.6	44.3	74.0
Actuated g/C Ratio	0.16	0.41	0.55	0.05	0.30	0.45	0.11	0.29	0.38	0.11	0.30	0.49
v/c Ratio	0.77	0.85	0.30	0.47	0.57	0.38	0.73	0.92	0.39	0.75	0.44	0.37
Control Delay	55.9	45.7	10.8	77.5	46.2	19.4	77.1	65.6	25.2	77.3	38.4	13.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.9	45.7	10.8	77.5	46.2	19.4	77.1	65.6	25.2	77.3	38.4	13.6
LOS	E	D	B	E	D	B	E	E	C	E	D	B
Approach Delay		43.1			41.3			60.7			41.2	
Approach LOS		D			D			E			D	
Queue Length 50th (ft)	191	612	140	43	272	120	129	511	123	139	209	148
Queue Length 95th (ft)	219	665	m134	73	335	195	180	#724	216	#175	256	215
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

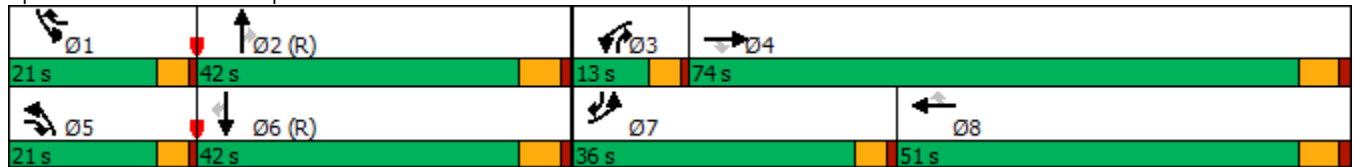


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	720	1688	908	194	1144	770	387	1080	665	396	1580	889
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.58	0.76	0.29	0.45	0.56	0.37	0.68	0.92	0.39	0.71	0.44	0.34

Intersection Summary
























Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 136 (91%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.92
 Intersection Signal Delay: 47.0 Intersection LOS: D
 Intersection Capacity Utilization 86.1% ICU Level of Service E
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	46	1	37	2	2	10	240	1311	63	174	1183	301
Future Volume (vph)	46	1	37	2	2	10	240	1311	63	174	1183	301
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.854				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1591	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted	0.755						0.190			0.162		
Satd. Flow (perm)	1406	1591	0	1863	1961	1583	354	3725	1583	302	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		39				51			80			155
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	48	40	0	2	2	11	253	1380	66	183	1245	317
Turn Type	pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	13.5	24.0		13.5	24.0	15.0	15.0	97.5	97.5	15.0	97.5	97.5
Total Split (%)	9.0%	16.0%		9.0%	16.0%	10.0%	10.0%	65.0%	65.0%	10.0%	65.0%	65.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	10.7	7.2		7.4	5.8	12.0	130.0	117.5	117.5	128.1	116.6	116.6
Actuated g/C Ratio	0.07	0.05		0.05	0.04	0.08	0.87	0.78	0.78	0.85	0.78	0.78
v/c Ratio	0.40	0.35		0.02	0.03	0.06	0.62	0.47	0.05	0.52	0.43	0.25
Control Delay	73.8	28.6		61.5	70.0	0.7	19.8	9.6	2.6	7.5	7.5	3.8
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.8	28.6		61.5	70.0	0.7	19.8	9.6	2.6	7.5	7.5	3.8
LOS	E	C		E	E	A	B	A	A	A	A	A
Approach Delay		53.2			18.0			10.9			6.8	
Approach LOS		D			B			B			A	
Queue Length 50th (ft)	46	1		2	2	0	108	256	5	16	183	34
Queue Length 95th (ft)	84	41		10	12	0	m183	m340	m7	51	375	106
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	133	225		133	235	198	426	2917	1257	379	2894	1264
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.36	0.18		0.02	0.01	0.06	0.59	0.47	0.05	0.48	0.43	0.25

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	100 (67%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.62
Intersection Signal Delay:	10.0
Intersection LOS:	A
Intersection Capacity Utilization	66.9%
ICU Level of Service	C
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

Ø1	Ø2 (R)	Ø3	Ø4
15 s	97.5 s	13.5 s	24 s
Ø5	Ø6 (R)	Ø7	Ø8
15 s	97.5 s	13.5 s	24 s

Lanes, Volumes, Timings
3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	1146	176	585	1021	36	220	50	520	277	198	301
Future Volume (vph)	47	1146	176	585	1021	36	220	50	520	277	198	301
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			97			55						87
Link Speed (mph)		45			45			35				20
Link Distance (ft)		609			1506			1718				409
Travel Time (s)		9.2			22.8			33.5				13.9
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	49	1206	185	616	1075	38	232	53	547	292	208	317
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	15.0	63.0	15.0	43.5	91.5	15.0	15.0	28.5		15.0	28.5	15.0
Total Split (%)	10.0%	42.0%	10.0%	29.0%	61.0%	10.0%	10.0%	19.0%		10.0%	19.0%	10.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	7.8	62.6	79.1	33.4	88.2	104.7	10.5	22.5	60.4	10.5	22.5	36.3
Actuated g/C Ratio	0.05	0.42	0.53	0.22	0.59	0.70	0.07	0.15	0.40	0.07	0.15	0.24
v/c Ratio	0.27	0.78	0.21	0.81	0.49	0.03	0.97	0.09	0.49	2.37	0.37	0.71
Control Delay	71.5	42.7	10.0	54.9	32.9	3.5	118.4	55.6	34.4	669.6	59.6	46.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.5	42.7	10.0	54.9	32.9	3.5	118.4	55.6	34.4	669.6	59.6	46.4
LOS	E	D	B	D	C	A	F	E	C	F	E	D
Approach Delay		39.5			40.1			59.2				272.5
Approach LOS		D			D			E				F
Queue Length 50th (ft)	24	532	42	305	410	1	119	23	223	-467	97	211
Queue Length 95th (ft)	46	663	95	365	487	m5	#208	46	267	#659	140	318
Internal Link Dist (ft)		529			1426			1638				329

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	240	1553	880	892	2189	1121	240	558	1226	123	558	475
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.78	0.21	0.69	0.49	0.03	0.97	0.09	0.45	2.37	0.37	0.67

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 50 (33%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 120
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 2.37
 Intersection Signal Delay: 82.6 Intersection LOS: F
 Intersection Capacity Utilization 83.8% ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	1146	176	585	1021	36	220	50	520	277	198	301
Future Volume (vph)	47	1146	176	585	1021	36	220	50	520	277	198	301
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	2		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	0.97	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			97			55						87
Link Speed (mph)		45			45			35				20
Link Distance (ft)		609			1506			1718				409
Travel Time (s)		9.2			22.8			33.5				13.9
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	49	1206	185	616	1075	38	232	53	547	292	208	317
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	15.0	63.0	15.0	43.5	91.5	15.0	15.0	28.5		15.0	28.5	15.0
Total Split (%)	10.0%	42.0%	10.0%	29.0%	61.0%	10.0%	10.0%	19.0%		10.0%	19.0%	10.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	7.8	62.6	79.1	33.4	88.2	104.7	10.5	22.5	60.4	10.5	22.5	36.3
Actuated g/C Ratio	0.05	0.42	0.53	0.22	0.59	0.70	0.07	0.15	0.40	0.07	0.15	0.24
v/c Ratio	0.27	0.78	0.21	0.81	0.49	0.03	0.97	0.09	0.49	1.22	0.37	0.71
Control Delay	71.5	42.7	10.0	54.9	32.9	3.5	118.4	55.6	34.4	184.7	59.6	46.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.5	42.7	10.0	54.9	32.9	3.5	118.4	55.6	34.4	184.7	59.6	46.4
LOS	E	D	B	D	C	A	F	E	C	F	E	D
Approach Delay		39.5			40.1			59.2				99.2
Approach LOS		D			D			E				F
Queue Length 50th (ft)	24	532	42	305	410	1	119	23	223	-179	97	211
Queue Length 95th (ft)	46	663	95	365	487	m5	#208	46	267	#278	140	318
Internal Link Dist (ft)		529			1426			1638				329

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	240	1553	880	892	2189	1121	240	558	1226	240	558	475
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.78	0.21	0.69	0.49	0.03	0.97	0.09	0.45	1.22	0.37	0.67

Intersection Summary
































Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 50 (33%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 100
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.22
 Intersection Signal Delay: 53.2 Intersection LOS: D
 Intersection Capacity Utilization 76.4% ICU Level of Service D
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 	 	 	 	 	 	 	 			
Traffic Volume (vph)	520	713	710	424	1046	206	227	684	56	295	1148	369
Future Volume (vph)	520	713	710	424	1046	206	227	684	56	295	1148	369
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			87			87			55			120
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	547	751	747	446	1101	217	239	720	59	311	1208	388
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	28.0	51.0	23.0	39.0	62.0	17.0	23.0	43.0	39.0	17.0	37.0	28.0
Total Split (%)	18.7%	34.0%	15.3%	26.0%	41.3%	11.3%	15.3%	28.7%	26.0%	11.3%	24.7%	18.7%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.5	50.0	75.9	24.7	51.2	74.2	19.9	37.3	68.1	16.9	34.4	63.9
Actuated g/C Ratio	0.16	0.33	0.51	0.16	0.34	0.49	0.13	0.25	0.45	0.11	0.23	0.43
v/c Ratio	1.02	0.61	0.89	0.79	0.87	0.26	0.53	0.78	0.08	0.80	0.98	0.52
Control Delay	106.2	36.0	30.5	70.4	54.0	13.6	65.4	59.3	5.8	68.6	67.0	27.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	106.2	36.0	30.5	70.4	54.0	13.6	65.4	59.3	5.8	68.6	67.0	27.6
LOS	F	D	C	E	D	B	E	E	A	E	E	C
Approach Delay		52.8			53.2			57.6			59.3	
Approach LOS		D			D			E			E	
Queue Length 50th (ft)	~300	269	420	218	523	69	112	348	2	153	~489	257
Queue Length 95th (ft)	m#354	m282	m351	269	591	124	162	425	27	m#267	#589	m387
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

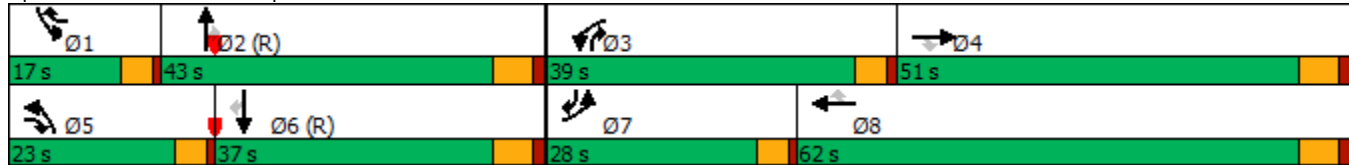


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	537	1241	843	789	1390	827	455	927	847	387	1227	743
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.02	0.61	0.89	0.57	0.79	0.26	0.53	0.78	0.07	0.80	0.98	0.52

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 5 (3%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 120
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.02
 Intersection Signal Delay: 55.4 Intersection LOS: E
 Intersection Capacity Utilization 89.6% ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	315	13	288	32	0	179	49	1355	6	28	1492	57
Future Volume (vph)	315	13	288	32	0	179	49	1355	6	28	1492	57
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.857				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1596	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted	0.950			0.000			0.109			0.115		
Satd. Flow (perm)	1770	1596	0	0	1961	1583	203	3725	1583	214	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		190				61			80			80
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	332	317	0	34	0	188	52	1426	6	29	1571	60
Turn Type	pm+pt	NA		pm+pt		pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	16.5	24.0		16.5	24.0	13.5	13.5	96.0	96.0	13.5	96.0	96.0
Total Split (%)	11.0%	16.0%		11.0%	16.0%	9.0%	9.0%	64.0%	64.0%	9.0%	64.0%	64.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	29.5	17.0		8.3		11.4	103.8	95.1	95.1	111.7	101.7	101.7
Actuated g/C Ratio	0.20	0.11		0.06		0.08	0.69	0.63	0.63	0.74	0.68	0.68
v/c Ratio	0.96	0.91		0.35		1.07	0.25	0.60	0.01	0.10	0.62	0.05
Control Delay	96.0	55.2		77.0		128.0	17.0	45.6	0.0	7.2	16.9	1.2
Queue Delay	0.0	0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	96.0	55.2		77.0		128.0	17.0	45.6	0.0	7.2	16.9	1.2
LOS	F	E		E		F	B	D	A	A	B	A
Approach Delay		76.1			120.2			44.4			16.1	
Approach LOS		E			F			D			B	
Queue Length 50th (ft)	312	127		33		~138	26	753	0	7	483	0
Queue Length 95th (ft)	#426	#294		70		#328	m33	m795	m0	18	601	11
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings

9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	347	369		147		176	243	2362	1032	277	2526	1099
Starvation Cap Reductn	0	0		0		0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0		0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0		0	0	0	0	0	0	0
Reduced v/c Ratio	0.96	0.86		0.23		1.07	0.21	0.60	0.01	0.10	0.62	0.05

Intersection Summary

























Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 81 (54%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.07
 Intersection Signal Delay: 42.0 Intersection LOS: D
 Intersection Capacity Utilization 76.7% ICU Level of Service D
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

Ø1	Ø2 (R)	Ø3	Ø4
13.5 s	96 s	16.5 s	24 s
Ø5	Ø6 (R)	Ø7	Ø8
13.5 s	96 s	16.5 s	24 s

Lanes, Volumes, Timings
3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	293	1181	44	207	756	189	448	269	638	62	47	61
Future Volume (vph)	293	1181	44	207	756	189	448	269	638	62	47	61
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			55			199						87
Link Speed (mph)		45			45			35			20	
Link Distance (ft)		609			1506			1718			409	
Travel Time (s)		9.2			22.8			33.5			13.9	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	308	1243	46	218	796	199	472	283	672	65	49	64
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	24.0	55.0	50.0	24.0	55.0	15.0	50.0	56.0		15.0	21.0	24.0
Total Split (%)	16.0%	36.7%	33.3%	16.0%	36.7%	10.0%	33.3%	37.3%		10.0%	14.0%	16.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	18.6	64.6	101.5	15.7	61.7	77.1	31.0	39.3	59.5	9.4	20.1	42.4
Actuated g/C Ratio	0.12	0.43	0.68	0.10	0.41	0.51	0.21	0.26	0.40	0.06	0.13	0.28
v/c Ratio	0.72	0.78	0.04	0.61	0.52	0.22	0.67	0.29	0.61	0.59	0.10	0.13
Control Delay	73.0	42.0	2.2	68.7	39.3	14.8	60.8	44.2	37.8	89.3	54.6	3.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.0	42.0	2.2	68.7	39.3	14.8	60.8	44.2	37.8	89.3	54.6	3.2
LOS	E	D	A	E	D	B	E	D	D	F	D	A
Approach Delay		46.8			40.5			46.7			48.8	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	152	549	0	112	377	62	230	116	294	63	22	0
Queue Length 95th (ft)	197	#778	14	152	471	137	280	150	318	116	42	18
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

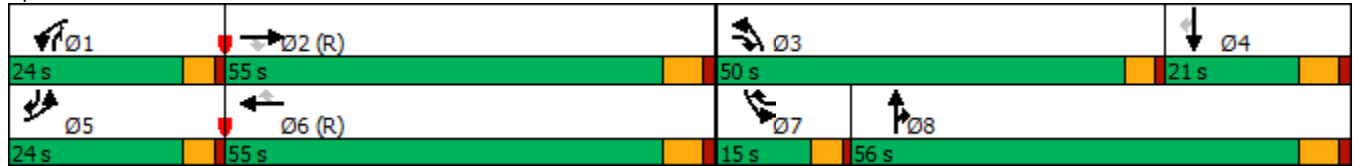


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	468	1603	1237	451	1531	920	1041	1241	1171	123	544	527
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.66	0.78	0.04	0.48	0.52	0.22	0.45	0.23	0.57	0.53	0.09	0.12

Intersection Summary

























Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 148 (99%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.78
 Intersection Signal Delay: 45.1
 Intersection LOS: D
 Intersection Capacity Utilization 71.4%
 ICU Level of Service C
 Analysis Period (min) 15
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	293	1181	44	207	756	189	448	269	638	62	47	61
Future Volume (vph)	293	1181	44	207	756	189	448	269	638	62	47	61
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	2		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	0.97	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			55			199						87
Link Speed (mph)		45			45			35			20	
Link Distance (ft)		609			1506			1718			409	
Travel Time (s)		9.2			22.8			33.5			13.9	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	308	1243	46	218	796	199	472	283	672	65	49	64
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	24.0	55.0	50.0	24.0	55.0	15.0	50.0	56.0		15.0	21.0	24.0
Total Split (%)	16.0%	36.7%	33.3%	16.0%	36.7%	10.0%	33.3%	37.3%		10.0%	14.0%	16.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	18.6	65.7	102.5	15.7	62.8	77.0	30.8	39.4	59.6	8.2	19.1	41.4
Actuated g/C Ratio	0.12	0.44	0.68	0.10	0.42	0.51	0.21	0.26	0.40	0.05	0.13	0.28
v/c Ratio	0.72	0.76	0.04	0.61	0.51	0.22	0.67	0.29	0.61	0.35	0.10	0.13
Control Delay	73.0	40.7	2.2	67.7	38.2	14.5	60.9	44.1	37.6	72.9	55.5	3.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.0	40.7	2.2	67.7	38.2	14.5	60.9	44.1	37.6	72.9	55.5	3.2
LOS	E	D	A	E	D	B	E	D	D	E	E	A
Approach Delay		45.8			39.6			46.6			43.1	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	152	535	0	111	366	62	230	116	294	32	22	0
Queue Length 95th (ft)	197	#778	14	152	471	137	280	150	318	57	42	18
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings

3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020

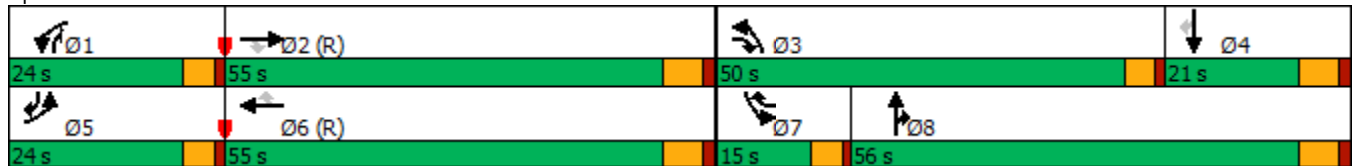


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	468	1631	1248	451	1559	930	1041	1241	1173	240	519	517
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.66	0.76	0.04	0.48	0.51	0.21	0.45	0.23	0.57	0.27	0.09	0.12

Intersection Summary
































Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	148 (99%), Referenced to phase 2:EBT and 6:WBT, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.76
Intersection Signal Delay:	44.3
Intersection LOS:	D
Intersection Capacity Utilization	71.4%
ICU Level of Service	C
Analysis Period (min)	15
# 95th percentile volume exceeds capacity, queue may be longer.	
Queue shown is maximum after two cycles.	

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 	 	 	 	 	 	 	 			
Traffic Volume (vph)	395	1221	265	83	608	275	252	951	247	285	678	292
Future Volume (vph)	395	1221	265	83	608	275	252	951	247	285	678	292
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			55			87			87			57
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	416	1285	279	87	640	289	265	1001	260	300	714	307
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	36.0	74.0	21.0	13.0	51.0	21.0	21.0	42.0	13.0	21.0	42.0	36.0
Total Split (%)	24.0%	49.3%	14.0%	8.7%	34.0%	14.0%	14.0%	28.0%	8.7%	14.0%	28.0%	24.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.7	61.2	83.0	8.0	45.5	68.5	15.9	42.7	56.8	17.0	43.9	73.7
Actuated g/C Ratio	0.16	0.41	0.55	0.05	0.30	0.46	0.11	0.28	0.38	0.11	0.29	0.49
v/c Ratio	0.77	0.85	0.31	0.47	0.57	0.38	0.73	0.94	0.40	0.77	0.46	0.38
Control Delay	55.8	45.3	10.7	77.5	46.0	19.2	77.2	69.1	25.5	78.1	38.2	13.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.8	45.3	10.7	77.5	46.0	19.2	77.2	69.1	25.5	78.1	38.2	13.6
LOS	E	D	B	E	D	B	E	E	C	E	D	B
Approach Delay		42.6			41.1			63.1			41.5	
Approach LOS		D			D			E			D	
Queue Length 50th (ft)	187	623	144	43	274	120	130	~523	124	147	213	153
Queue Length 95th (ft)	220	666	m141	73	339	198	181	#728	216	#209	245	192
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings
 6: Naperville Road & Warrenville Road

09/09/2020

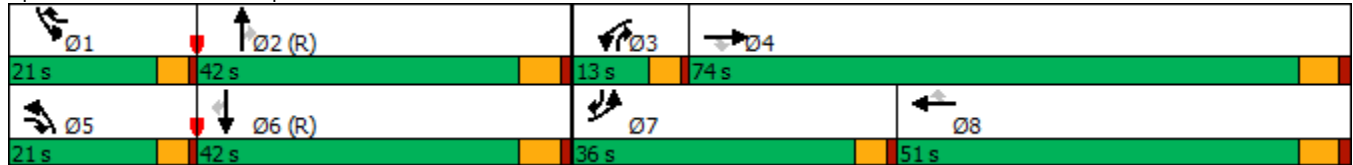


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	720	1688	912	194	1148	775	388	1061	657	401	1567	885
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.58	0.76	0.31	0.45	0.56	0.37	0.68	0.94	0.40	0.75	0.46	0.35

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 136 (91%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 0.94
 Intersection Signal Delay: 47.5 Intersection LOS: D
 Intersection Capacity Utilization 86.8% ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	77	1	70	2	2	10	247	1311	63	174	1183	311
Future Volume (vph)	77	1	70	2	2	10	247	1311	63	174	1183	311
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.852				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1587	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted	0.727						0.184			0.160		
Satd. Flow (perm)	1354	1587	0	1863	1961	1583	343	3725	1583	298	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		74				51			80			160
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	81	75	0	2	2	11	260	1380	66	183	1245	327
Turn Type	pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	13.5	24.0		13.5	24.0	15.0	15.0	97.5	97.5	15.0	97.5	97.5
Total Split (%)	9.0%	16.0%		9.0%	16.0%	10.0%	10.0%	65.0%	65.0%	10.0%	65.0%	65.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	11.7	7.8		7.6	6.0	12.4	127.8	114.4	114.4	124.6	112.8	112.8
Actuated g/C Ratio	0.08	0.05		0.05	0.04	0.08	0.85	0.76	0.76	0.83	0.75	0.75
v/c Ratio	0.62	0.49		0.02	0.03	0.06	0.65	0.49	0.05	0.53	0.44	0.27
Control Delay	85.5	25.2		61.0	69.0	0.7	21.7	10.7	2.6	8.4	8.6	4.1
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	85.5	25.2		61.0	69.0	0.7	21.7	10.7	2.6	8.4	8.6	4.1
LOS	F	C		E	E	A	C	B	A	A	A	A
Approach Delay		56.5			17.8			12.1			7.7	
Approach LOS		E			B			B			A	
Queue Length 50th (ft)	79	1		2	2	0	124	274	4	16	186	35
Queue Length 95th (ft)	127	55		10	11	0	m189	m336	m7	54	393	116
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	135	255		136	235	200	413	2841	1226	369	2801	1230
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.60	0.29		0.01	0.01	0.06	0.63	0.49	0.05	0.50	0.44	0.27

Intersection Summary

Area Type:	Other
Cycle Length:	150
Actuated Cycle Length:	150
Offset:	100 (67%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
Natural Cycle:	90
Control Type:	Actuated-Coordinated
Maximum v/c Ratio:	0.65
Intersection Signal Delay:	11.9
Intersection LOS:	B
Intersection Capacity Utilization	69.0%
ICU Level of Service	C
Analysis Period (min)	15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

Ø1	Ø2 (R)	Ø3	Ø4
15 s	97.5 s	13.5 s	24 s
Ø5	Ø6 (R)	Ø7	Ø8
15 s	97.5 s	13.5 s	24 s

Lanes, Volumes, Timings
3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	75	1146	176	585	1024	60	220	77	520	290	210	314
Future Volume (vph)	75	1146	176	585	1024	60	220	77	520	290	210	314
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	1		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	1770	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			89			63						87
Link Speed (mph)		45			45			35				20
Link Distance (ft)		609			1506			1718				409
Travel Time (s)		9.2			22.8			33.5				13.9
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	79	1206	185	616	1078	63	232	81	547	305	221	331
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	15.0	63.0	15.0	43.5	91.5	15.0	15.0	28.5		15.0	28.5	15.0
Total Split (%)	10.0%	42.0%	10.0%	29.0%	61.0%	10.0%	10.0%	19.0%		10.0%	19.0%	10.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	8.7	62.6	79.1	33.4	87.3	103.8	10.5	22.5	60.4	10.5	22.5	37.2
Actuated g/C Ratio	0.06	0.42	0.53	0.22	0.58	0.69	0.07	0.15	0.40	0.07	0.15	0.25
v/c Ratio	0.40	0.78	0.21	0.81	0.50	0.06	0.97	0.15	0.49	2.48	0.40	0.72
Control Delay	73.6	42.7	10.9	54.9	33.5	4.9	118.4	56.2	34.4	715.3	60.0	47.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	73.6	42.7	10.9	54.9	33.5	4.9	118.4	56.2	34.4	715.3	60.0	47.4
LOS	E	D	B	D	C	A	F	E	C	F	E	D
Approach Delay		40.4			40.0			59.1			288.4	
Approach LOS		D			D			E			F	
Queue Length 50th (ft)	39	532	46	305	420	3	119	36	223	-493	104	224
Queue Length 95th (ft)	67	663	100	367	492	m15	#208	64	267	#688	148	339
Internal Link Dist (ft)		529			1426			1638			329	

Lanes, Volumes, Timings

3: Freedom Drive/Lucent Lane & Warrenville Road

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	240	1553	876	892	2167	1114	240	558	1226	123	558	475
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.78	0.21	0.69	0.50	0.06	0.97	0.15	0.45	2.48	0.40	0.70

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 50 (33%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 120
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 2.48
 Intersection Signal Delay: 86.5
 Intersection LOS: F
 Intersection Capacity Utilization 84.5%
 ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	1146	176	585	1021	36	220	50	520	277	198	301
Future Volume (vph)	47	1146	176	585	1021	36	220	50	520	277	198	301
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	260		210	485		325	365		355	170		215
Storage Lanes	2		1	2		1	2		1	2		1
Taper Length (ft)	300			300			300			80		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	0.88	0.97	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	2787	3433	3725	1583
Right Turn on Red			Yes			Yes			No			Yes
Satd. Flow (RTOR)			97			55						87
Link Speed (mph)		45			45			35				20
Link Distance (ft)		609			1506			1718				409
Travel Time (s)		9.2			22.8			33.5				13.9
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	49	1206	185	616	1075	38	232	53	547	292	208	317
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pt+ov	Prot	NA	pm+ov
Protected Phases	5	2	3	1	6	7	3	8	81	7	4	5
Permitted Phases			2			6						4
Detector Phase	5	2	3	1	6	7	3	8	81	7	4	5
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	21.0	9.5
Total Split (s)	15.0	63.0	15.0	43.5	91.5	15.0	15.0	28.5		15.0	28.5	15.0
Total Split (%)	10.0%	42.0%	10.0%	29.0%	61.0%	10.0%	10.0%	19.0%		10.0%	19.0%	10.0%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5		3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5		1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0		4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	C-Max	None	None	C-Max	None	None	None		None	None	None
Act Effect Green (s)	7.8	62.6	79.1	33.4	88.2	104.7	10.5	22.5	60.4	10.5	22.5	36.3
Actuated g/C Ratio	0.05	0.42	0.53	0.22	0.59	0.70	0.07	0.15	0.40	0.07	0.15	0.24
v/c Ratio	0.27	0.78	0.21	0.81	0.49	0.03	0.97	0.09	0.49	1.22	0.37	0.71
Control Delay	71.5	42.7	10.0	54.9	32.9	3.5	118.4	55.6	34.4	184.7	59.6	46.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	71.5	42.7	10.0	54.9	32.9	3.5	118.4	55.6	34.4	184.7	59.6	46.4
LOS	E	D	B	D	C	A	F	E	C	F	E	D
Approach Delay		39.5			40.1			59.2				99.2
Approach LOS		D			D			E				F
Queue Length 50th (ft)	24	532	42	305	410	1	119	23	223	-179	97	211
Queue Length 95th (ft)	46	663	95	365	487	m5	#208	46	267	#278	140	318
Internal Link Dist (ft)		529			1426			1638				329

Lanes, Volumes, Timings
 3: Freedom Drive/Lucent Lane & Warrenville Road

10/15/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	260		210	485		325	365		355	170		215
Base Capacity (vph)	240	1553	880	892	2189	1121	240	558	1226	240	558	475
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.20	0.78	0.21	0.69	0.49	0.03	0.97	0.09	0.45	1.22	0.37	0.67

Intersection Summary





































Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 50 (33%), Referenced to phase 2:EBT and 6:WBT, Start of Green
 Natural Cycle: 100
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.22
 Intersection Signal Delay: 53.2 Intersection LOS: D
 Intersection Capacity Utilization 76.4% ICU Level of Service D
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Freedom Drive/Lucent Lane & Warrenville Road



Lanes, Volumes, Timings
6: Naperville Road & Warrenville Road

09/09/2020

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	 	 	 	 	 	 	 	 	 	  	  	
Traffic Volume (vph)	520	718	718	424	1064	215	233	698	56	305	1156	372
Future Volume (vph)	520	718	718	424	1064	215	233	698	56	305	1156	372
Ideal Flow (vphpl)	1900	2000	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	425		470	270		305	370		245	300		320
Storage Lanes	2		1	2		1	1		1	2		1
Taper Length (ft)	300			250			110			155		
Lane Util. Factor	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.95	1.00	0.97	0.91	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	3433	3725	1583	3433	3725	1583	3433	3725	1583	3433	5353	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			87			87			55			120
Link Speed (mph)		45			45			40			40	
Link Distance (ft)		1506			1157			608			1352	
Travel Time (s)		22.8			17.5			10.4			23.0	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	547	756	756	446	1120	226	245	735	59	321	1217	392
Turn Type	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov	Prot	NA	pm+ov
Protected Phases	7	4	5	3	8	1	5	2	3	1	6	7
Permitted Phases			4			8			2			6
Detector Phase	7	4	5	3	8	1	5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5	9.5	24.0	9.5
Total Split (s)	28.0	51.0	23.0	39.0	62.0	17.0	23.0	43.0	39.0	17.0	37.0	28.0
Total Split (%)	18.7%	34.0%	15.3%	26.0%	41.3%	11.3%	15.3%	28.7%	26.0%	11.3%	24.7%	18.7%
Yellow Time (s)	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5	3.5	4.5	3.5
All-Red Time (s)	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0	1.0	1.5	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5	4.5	6.0	4.5
Lead/Lag	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None	None	None	None	None	None	C-Max	None	None	C-Max	None
Act Effect Green (s)	23.5	50.6	77.1	24.7	51.8	74.5	20.6	37.0	67.7	16.7	33.1	62.6
Actuated g/C Ratio	0.16	0.34	0.51	0.16	0.35	0.50	0.14	0.25	0.45	0.11	0.22	0.42
v/c Ratio	1.02	0.60	0.88	0.79	0.87	0.27	0.52	0.80	0.08	0.84	1.03	0.54
Control Delay	104.7	36.1	29.1	70.4	54.1	14.0	64.9	60.7	5.8	70.5	77.1	27.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	104.7	36.1	29.1	70.4	54.1	14.0	64.9	60.7	5.8	70.5	77.1	27.6
LOS	F	D	C	E	D	B	E	E	A	E	E	C
Approach Delay		51.8			53.1			58.6			66.0	
Approach LOS		D			D			E			E	
Queue Length 50th (ft)	~299	272	439	218	532	74	115	357	2	151	~495	263
Queue Length 95th (ft)	m#350	m282	m349	269	606	131	166	436	27	m#275	#596	m387
Internal Link Dist (ft)		1426			1077			528			1272	

Lanes, Volumes, Timings
 6: Naperville Road & Warrenville Road

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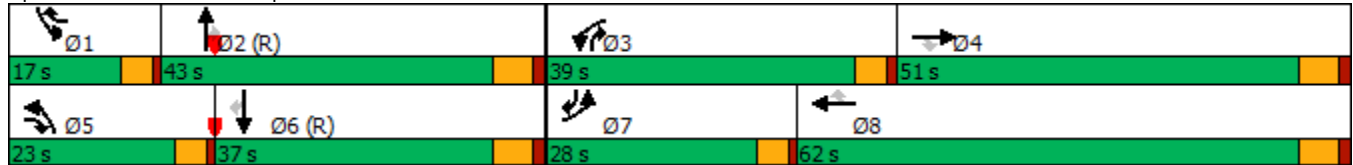


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	425		470	270		305	370		245	300		320
Base Capacity (vph)	537	1255	855	789	1390	830	470	918	844	382	1182	731
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	1.02	0.60	0.88	0.57	0.81	0.27	0.52	0.80	0.07	0.84	1.03	0.54

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 5 (3%), Referenced to phase 2:NBT and 6:SBT, Start of Green
 Natural Cycle: 100
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.03
 Intersection Signal Delay: 57.2 Intersection LOS: E
 Intersection Capacity Utilization 90.3% ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Naperville Road & Warrenville Road



Lanes, Volumes, Timings
 9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	335	13	310	32	0	179	72	1355	6	28	1492	91
Future Volume (vph)	335	13	310	32	0	179	72	1355	6	28	1492	91
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	170		0	140		140	137		125	200		105
Storage Lanes	1		0	1		1	1		1	1		1
Taper Length (ft)	80			155			145			110		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.856				0.850			0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1595	0	1770	1961	1583	1770	3725	1583	1770	3725	1583
Flt Permitted	0.950			0.000			0.099			0.114		
Satd. Flow (perm)	1770	1595	0	0	1961	1583	184	3725	1583	212	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		190				59			80			80
Link Speed (mph)		20			20			40				45
Link Distance (ft)		479			564			1352				507
Travel Time (s)		16.3			19.2			23.0				7.7
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	353	340	0	34	0	188	76	1426	6	29	1571	96
Turn Type	pm+pt	NA		pm+pt		pm+ov	pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8	1	5	2		1	6	
Permitted Phases	4			8		8	2		2	6		6
Detector Phase	7	4		3	8	1	5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0	24.0	9.5	24.0	24.0
Total Split (s)	16.5	24.0		16.5	24.0	13.5	13.5	96.0	96.0	13.5	96.0	96.0
Total Split (%)	11.0%	16.0%		11.0%	16.0%	9.0%	9.0%	64.0%	64.0%	9.0%	64.0%	64.0%
Yellow Time (s)	3.0	4.5		3.0	4.5	3.0	3.0	4.5	4.5	3.0	4.5	4.5
All-Red Time (s)	1.0	1.5		1.0	1.5	1.0	1.0	1.5	1.5	1.0	1.5	1.5
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0	6.0		4.0	6.0	4.0	4.0	6.0	6.0	4.0	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	C-Max
Act Effect Green (s)	31.2	18.8		8.3		10.5	103.7	94.3	94.3	109.2	97.4	97.4
Actuated g/C Ratio	0.21	0.13		0.06		0.07	0.69	0.63	0.63	0.73	0.65	0.65
v/c Ratio	0.96	0.93		0.35		1.14	0.37	0.61	0.01	0.11	0.65	0.09
Control Delay	94.8	60.4		77.0		151.7	19.3	46.1	0.0	7.5	18.9	3.6
Queue Delay	0.0	0.0		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	94.8	60.4		77.0		151.7	19.3	46.1	0.0	7.5	18.9	3.6
LOS	F	E		E		F	B	D	A	A	B	A
Approach Delay		77.9			140.2			44.6			17.8	
Approach LOS		E			F			D			B	
Queue Length 50th (ft)	329	151		33		~167	38	752	0	8	515	6
Queue Length 95th (ft)	#484	#348		70		#331	m48	m794	m0	18	612	31
Internal Link Dist (ft)		399			484			1272			427	

Lanes, Volumes, Timings

9: Naperville Road & Lucent Lane/Navistar Drive

09/09/2020



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)	170			140		140	137		125	200		105
Base Capacity (vph)	368	376		147		165	229	2342	1024	263	2419	1055
Starvation Cap Reductn	0	0		0		0	0	0	0	0	0	0
Spillback Cap Reductn	0	0		0		0	0	0	0	0	0	0
Storage Cap Reductn	0	0		0		0	0	0	0	0	0	0
Reduced v/c Ratio	0.96	0.90		0.23		1.14	0.33	0.61	0.01	0.11	0.65	0.09

Intersection Summary

Area Type: Other
 Cycle Length: 150
 Actuated Cycle Length: 150
 Offset: 81 (54%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green
 Natural Cycle: 90
 Control Type: Actuated-Coordinated
 Maximum v/c Ratio: 1.14
 Intersection Signal Delay: 44.3 Intersection LOS: D
 Intersection Capacity Utilization 83.3% ICU Level of Service E
 Analysis Period (min) 15
 ~ Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.
 # 95th percentile volume exceeds capacity, queue may be longer.
 Queue shown is maximum after two cycles.
 m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Naperville Road & Lucent Lane/Navistar Drive

13.5 s	96 s	16.5 s	24 s
13.5 s	96 s	16.5 s	24 s